



Alpine, an ITW Company 155 Harlem Ave North Building, 4th Floor Glenview, IL 60025 Phone: (800)755-6001 www.alpineitw.com

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COA#0-278, Yoonhwak Kim, FL PE #86367 Florida Certificate of Product Approval #FL 1999

| Site Information: | Page 1: |
|---|--------------------|
| Customer: Carpenter Contractors of America | Job Number: 402471 |
| Job Description: ,7F5 ,BRAA ,15;16;21;25 ,RB01 / BRADEN/A | |
| Address: | |

| Job Engineering Criteria: | | | |
|--|--------------------------------|--|--|
| Design Code: FBC 8th Ed. 2023 Res. | IntelliVIEW Version: 23.02.01A | | |
| | JRef #: 1YdO89750003 | | |
| Wind Standard: ASCE 7-22 Wind Speed (mph): 160 | Design Loading (psf): 45.00 | | |
| Building Type: Closed | | | |

This package contains general notes pages, 50 truss drawing(s) and 36 detail(s).

| Item | Drawing Number | Truss |
|------|-------------------|-------|
| 1 | 267.25.0813.23168 | A1 |
| 3 | 267.25.0813.23716 | A1B |
| 5 | 267.25.0813.22888 | A2T |
| 7 | 267.25.0813.22619 | A4T |
| 9 | 267.25.0813.24008 | A6T |
| 11 | 267.25.0920.32770 | A8GT |
| 13 | 267.25.0813.23873 | A10 |
| 15 | 267.25.0813.23372 | A12 |
| 17 | 267.25.0919.40050 | A14G |
| 19 | 267.25.0920.06300 | A16G |
| 21 | 267.25.0813.23074 | C1 |
| 23 | 267.25.0921.30007 | C3G |
| 25 | 267.25.0813.23481 | M2G |
| 27 | 267.25.0813.21993 | EJ7 |
| 29 | 267.25.0813.23983 | EJ4 |
| 31 | 267.25.0813.22415 | EJ3A |
| 33 | 267.25.0813.23027 | CJ5A |
| 35 | 267.25.0813.22024 | CJ3 |
| 37 | 267.25.0813.22086 | CJ3B |
| 39 | 267.25.0813.22400 | CJ1A |
| 41 | 267.25.0813.22401 | CJ1F |
| 43 | 267.25.0813.23184 | CJ1C |
| 45 | 267.25.0813.22604 | HJ7 |
| 47 | 267.25.0813.22823 | НЈЗА |
| 49 | 267.25.0813.22057 | НЈЗС |

| Item | Drawing Number | Truss |
|------|-------------------|-------|
| 2 | 267.25.0813.23686 | A1A |
| 4 | 267.25.0813.22776 | A1T |
| 6 | 267.25.0813.23278 | A3T |
| 8 | 267.25.0813.23371 | A5T |
| 10 | 267.25.0813.23403 | A7T |
| 12 | 267.25.0813.23794 | A9G |
| 14 | 267.25.0813.23512 | A11 |
| 16 | 267.25.0813.23732 | A13 |
| 18 | 267.25.0813.23810 | A15 |
| 20 | 267.25.0920.53987 | B1G |
| 22 | 267.25.0921.06127 | C2G |
| 24 | 267.25.0922.25063 | M1G |
| 26 | 267.25.0922.10650 | FG1 |
| 28 | 267.25.0813.24092 | EJ7T |
| 30 | 267.25.0813.22886 | EJ3 |
| 32 | 267.25.0813.22055 | CJ5 |
| 34 | 267.25.0813.22416 | CJ5B |
| 36 | 267.25.0813.22698 | CJ3A |
| 38 | 267.25.0813.22056 | CJ1 |
| 40 | 267.25.0813.22352 | CJ1E |
| 42 | 267.25.0813.22997 | CJ1H |
| 44 | 267.25.0813.23089 | CJ1D |
| 46 | 267.25.0813.23575 | HJ4 |
| 48 | 267.25.0813.22322 | НЈЗВ |
| 50 | 267.25.0813.22745 | нуз |





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| Site Information: | Page 2: |
|---|--------------------|
| Customer: Carpenter Contractors of America | Job Number: 402471 |
| Job Description: ,7F5 ,BRAA ,15;16;21;25 ,RB01 / BRADEN/A | |
| Address: | |

| Item | Drawing Number | Truss |
|------|-----------------|-------|
| 51 | 160TL | |
| 53 | A14015ENC160118 | |
| 55 | A14030ENC160118 | |
| 57 | A16015ENC160118 | |
| 59 | A16030ENC160118 | |
| 61 | A18015ENC160118 | |
| 63 | BRCLBSUB1014 | |
| 65 | CNSY42PL0118 | |
| 67 | GBLLETIN0118 | |
| 69 | LSCSYX2A1014 | |
| 71 | PBSCAB101014 | |
| 73 | PBSCAB220723 | |
| 75 | S16015ENC100815 | |
| 77 | S16030ENC100815 | |
| 79 | STRBRIBR1014 | |
| 81 | VAL180160118 | |
| 83 | VALTN160118 | |
| 85 | REPPLTNL1014 | |

| Item | Drawing Number | Truss |
|------|-----------------|-------|
| 52 | A14015ENC101014 | |
| 54 | A14030ENC101014 | |
| 56 | A16015ENC101014 | |
| 58 | A16030ENC101014 | |
| 60 | A18015ENC101014 | |
| 62 | BRCLBSUB0119 | |
| 64 | CNNAILSP1014 | |
| 66 | DEFLCAMB1014 | |
| 68 | GBLLETIN1014 | |
| 70 | PB180220723 | |
| 72 | PBSCAB160118 | |
| 74 | RIGINSRT1014 | |
| 76 | S16015ENC160118 | |
| 78 | S16030ENC160118 | |
| 80 | VAL160101014 | |
| 82 | VAL180220723 | |
| 84 | VALTN220723 | _ |
| 86 | S20030END100815 | |

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high-quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed, and detailed by the Building Designer.

Connector Plate Information:

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Bearing Information:

The bearing area factor, Cb, is considered for the allowable capacity of solid sawn wood bearings supporting trusses that are located a minimum of 3" from the end of the lumber piece.

General Notes (continued)

Coated Lumber:

Coated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Coated lumber has no adjustments to lumber properties. Coated lumber may be more brittle than uncoated lumber. Special handling care must be taken to prevent breakage during all handling activities. Refer to manufacturer literature, specifications, and code evaluation reports for restrictions, details, and requirements.

Fire Retardant Treated Lumber:

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

Key to Terms:

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

C = Coated lumber.

C-AT = AtTEK coated lumber.

C-FX = FX Lumber Guard coated lumber.

C -TE = TechWood 4400 coated lumber.

CL = Certified lumber.

Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-BF = Boraflame Fire Retardant Treated lumber

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-ON = OnWood Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

FRT-PR = ProWood Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for all load cases.

Max Web CSI= Maximum bending and axial Combined Stress Index for Webs for all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

General Notes (continued)

Key to Terms (continued):

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.

W = Width of non-hanger bearing, in inches.

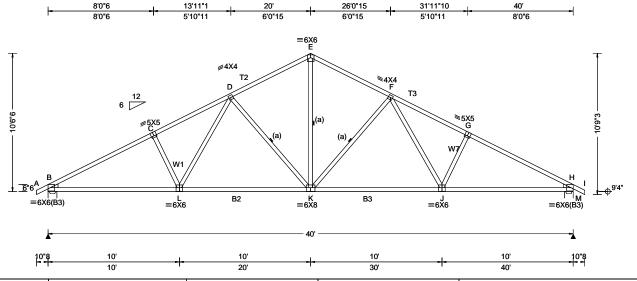
Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

References:

- 1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
- 2. ICC: International Code Council; www.iccsafe.org.
- 3. Alpine, a division of ITW Building Components Group Inc.: 155 Harlem Ave, North Building, 4th Floor, Glenview, IL 60025; www.alpineitw.com.
- 4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
- 5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www. sbcacomponents.com

SEQN: 837680/ COMN Ply: 1 Job Number: 402471 Cust: R 8975 JRef: 1YdO89750003 T2 / FROM: MTV Qty: 6 ,7F5 ,BRAA ,15;16;21;25 ,RB01 / BRADEN/A DrwNo: 267.25.0813.23168 Truss Label: A1 / WHK 09/24/2025



| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | |
|------------------------|--|------------------------------|---------------------------------|---|
| TCLL: 20.00 | Wind Std: ASCE 7-22 | Pg: NA Ct: NA CAT: NA | PP Deflection in loc L/defl L/# | |
| TCDL: 15.00 | Speed: 160 mph | Pf: NA Ce: NA | VERT(LL): 0.233 K 999 240 | |
| BCLL: 0.00 | Enclosure: Closed | Lu: NA Cs: NA | VERT(CL): 0.485 K 985 240 | |
| BCDL: 10.00 | Risk Category: II | Snow Duration: NA | HORZ(LL): 0.091 H | |
| Des Ld: 45.00 | EXP: C Kzt: NA | | HORZ(TL): 0.189 H | |
| NCBCLL: 10.00 | Mean Height: 15.00 ft TCDL: 4.2 psf | Building Code: | Creep Factor: 2.0 | |
| Soffit: 0.00 | BCDL: 5.0 psf | FBC 8th Ed. 2023 Res. | Max TC CSI: 0.888 | |
| Load Duration: 1.25 | MWFRS Parallel Dist: h to 2h | TPI Std: 2014 | Max BC CSI: 0.906 | |
| Spacing: 24.0 " | C&C Dist a: 4.00 ft | Rep Fac: Yes | Max Web CSI: 0.521 | |
| | Loc. from endwall: not in 13.00 ft | FT/RT:20(0)/10(0) | | |
| | GCpi: 0.18 | Plate Type(s): | | 1 |
| | Wind Duration: 1.60 | WAVE | VIEW Ver: 23.02.01A.1204.18 | |
| | | | | - |

Lumber

Top chord: 2x4 SP #1; T2,T3 2x4 SP #2 N; Bot chord: 2x4 SP #1; B2,B3 2x4 SP 2400f-2.0E; Webs: 2x4 SP #2 N; W1,W7 2x4 SP #3; Lt Wedge: 2x4 SP #3;Rt Wedge: 2x4 SP #3;

Bracing

(a) 1X4 #3SRB or better continuous lateral restraint to be equally spaced. Attach with (2) 8d Box or Gun nails(0.113"x2.5",min.). Restraint material to be supplied and attached at both ends to a suitable support by erection contractor.

(a) or scab reinforcement may be used in lieu of CLR restraint. substitute (1) scab for (1) CLR and (2) scabs for (2) CLR'S where shown. Scab reinforcement to be same size, species, grade, and 80% length of web member. Attach with 0.128x3" gun nails @ 6" oc.

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ /R /Rh /Rw /U /RL В 2109 /-/1333 /311 2109 /-/1333 /311 /-Wind reactions based on MWFRS Brg Wid = 8.0Min Reg = 2.5 (Truss) Brg Wid = 8.0 Min Req = 2.5 (Truss) Bearings B & H are a rigid surface. Bearings B & H require a seat plate. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

| B - C | 1402 - 3699 | E-F | 1144 | - 2474 |
|-------|-------------|-----|------|--------|
| C-D | 1435 - 3450 | F-G | | - 3450 |
| D - E | 1144 - 2474 | G-H | 1401 | - 3699 |

Maximum Bot Chord Forces Per Plv (lbs)

| Ciloius i | ens.comp. | Cilolus | i Ciio. | Comp. |
|-----------|---------------------------|------------|---------|-----------------|
| | 3174 - 1107 2663 - 837 | K-J J-H | | - 808 - 1085 |

Maximum Web Forces Per Ply (lbs)

| Webs | Tens.Comp. | Webs | Tens. (| Comp. |
|-------|-------------|-------|---------|-------|
| | 382 - 408 | K-F | 505 | - 829 |
| L - D | 711 - 263 | F-J | 711 | - 263 |
| D - K | 505 - 829 | J - G | 382 | - 408 |
| F - K | 1671 - 6/13 | | | |



COA#0-278, Yoonhwak Kim, FL PE #86367 Flood24Oe2fficate of Product Approval #FL 1999

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

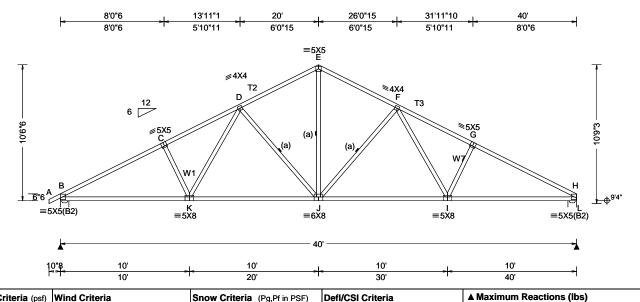
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installiers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have continuous lateral restraint (CLR), installed with diagonal bracing installed on the CLR per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TP1 1 Sec. 2.

For more information see these web sites: Alpine: alpineitw.com: TPI: binst.org: SBCA: sbcacomponents.com: ICC: iccsafe.org: AWC: awc.org



SEQN: 837536 / COMN Ply: 1 Job Number: 402471 Cust: R 8975 JRef: 1YdO89750003 T6 / FROM: MTV ,7F5 ,BRAA ,15;16;21;25 ,RB01 / BRADEN/A DrwNo: 267.25.0813.23686 Qty: 1 Truss Label: A1A KD / YK 09/24/2025



| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | DefI/CSI Criteria | ı |
|------------------------|--|------------------------------|---------------------------------|---|
| TCLL: 20.00 | Wind Std: ASCE 7-22 | Pg: NA Ct: NA CAT: NA | PP Deflection in loc L/defl L/# | ı |
| TCDL: 15.00 | Speed: 160 mph | Pf: NA Ce: NA | VERT(LL): 0.183 J 999 240 | ı |
| BCLL: 0.00 | Enclosure: Closed | Lu: NA Cs: NA | VERT(CL): 0.427 J 999 240 | |
| BCDL: 10.00 | Risk Category: II | Snow Duration: NA | HORZ(LL): 0.073 H | |
| Des Ld: 45.00 | EXP: C Kzt: NA | | HORZ(TL): 0.170 H | |
| NCBCLL: 10.00 | Mean Height: 15.00 ft TCDL: 4.2 psf | Building Code: | Creep Factor: 2.0 | |
| Soffit: 0.00 | BCDL: 5.0 psf | FBC 8th Ed. 2023 Res. | Max TC CSI: 0.831 | ı |
| Load Duration: 1.25 | MWFRS Parallel Dist: h to 2h | TPI Std: 2014 | Max BC CSI: 0.808 | ı |
| Spacing: 24.0 " | C&C Dist a: 4.00 ft | Rep Fac: Yes | Max Web CSI: 0.521 | |
| | Loc. from endwall: not in 13.00 ft | FT/RT:20(0)/10(0) | | |
| | GCpi: 0.18 | Plate Type(s): | | 1 |
| | Wind Duration: 1.60 | WAVE | VIEW Ver: 23.02.01A.1204.18 | l |
| | | | | _ |

Lumber

Top chord: 2x4 SP #1; T2,T3 2x4 SP #2 N; Bot chord: 2x4 SP #1; Webs: 2x4 SP #2 N; W1,W7 2x4 SP #3;

Bracing

(a) 1X4 #3SRB or better continuous lateral restraint to be equally spaced. Attach with (2) 8d Box or Gun nails(0.113"x2.5",min.). Restraint material to be supplied and attached at both ends to a suitable support by erection contractor.

(a) or scab reinforcement may be used in lieu of CLR restraint. substitute (1) scab for (1) CLR and (2) scabs for (2) CLR'S where shown. Scab reinforcement to be same size, species, grade, and 80% length of web member. Attach with 0.128x3" gun nails @ 6" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 1403 - 3321 E-F 1145 - 2171 C-D 1436 - 3071 F-G 1441 - 3076 Ď-E 1145 - 2171 - 3326 G-H 1408

Brg Wid = 8.0 Min Req = 2.2 (Truss)

/Rh

/-

Wind reactions based on MWFRS Brg Wid = 8.0

Bearings B & H are a rigid surface. Bearings B & H require a seat plate.

Non-Gravity

/RL

/-

/Rw /U

/1333 /311

/1258 /298

Min Reg = 2.3 (Truss)

Gravity

/R

Loc R+

1936 /-

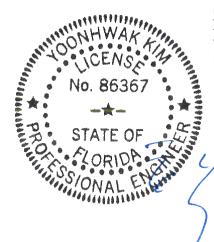
1870

В

Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

B - K 2841 - 1145 J - I 2360 - 848 K - J 2359 - 876 1 - H 2847 - 1130

Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. C - K .1 - F - 779 382 - 426 507 K - D 634 - 263 F - I 639 - 278 D-J 505 - 777 I-G 393 - 430 1398



COA#0-278, Yoonhwak Kim, FL PE #86367 Flood24/2026 icate of Product Approval #FL 1999

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

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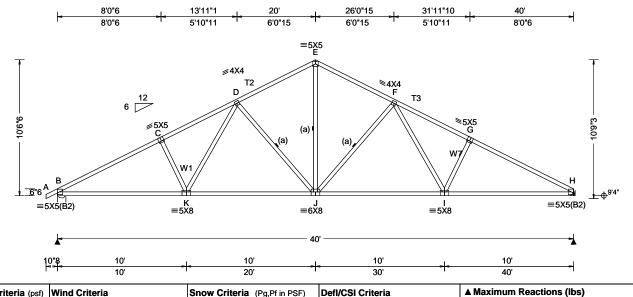
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have continuous lateral restraint (CLR), installed with diagonal bracing installed on the CLR per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec. 2.

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SEQN: 837539 / COMN Ply: 1 Job Number: 402471 Cust: R 8975 JRef: 1YdO89750003 T44 / FROM: MTV ,7F5 ,BRAA ,15;16;21;25 ,RB01 / BRADEN/A DrwNo: 267.25.0813.23716 Qty: 1 Truss Label: A1B KD / YK 09/24/2025



| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | DefI/CSI Criteria | |
|------------------------|---|------------------------------|---------------------------------|---|
| TCLL: 20.00 | Wind Std: ASCE 7-22 | Pg: NA Ct: NA CAT: NA | PP Deflection in loc L/defl L/# | |
| TCDL: 15.00 | Speed: 160 mph | Pf: NA Ce: NA | VERT(LL): 0.183 J 999 240 | |
| BCLL: 0.00 | Enclosure: Closed | Lu: NA Cs: NA | VERT(CL): 0.427 J 999 240 | ı |
| BCDL: 10.00 | Risk Category: II | Snow Duration: NA | HORZ(LL): 0.073 H | ı |
| Des Ld: 45.00 | EXP: C Kzt: NA Mean Height: 15.00 ft | | HORZ(TL): 0.170 H | |
| NCBCLL: 10.00 | TCDL: 4.2 psf | Building Code: | Creep Factor: 2.0 | |
| Soffit: 0.00 | BCDL: 5.0 psf | FBC 8th Ed. 2023 Res. | Max TC CSI: 0.831 | |
| Load Duration: 1.25 | MWFRS Parallel Dist: h to 2h | TPI Std: 2014 | Max BC CSI: 0.808 | |
| Spacing: 24.0 " | C&C Dist a: 4.00 ft | Rep Fac: Yes | Max Web CSI: 0.521 | |
| | Loc. from endwall: not in 13.00 ft | FT/RT:20(0)/10(0) | | |
| | GCpi: 0.18 | Plate Type(s): | | 4 |
| | Wind Duration: 1.60 | WAVE | VIEW Ver: 23.02.01A.1204.18 | |
| Lumbor | | | | - |

Top chord: 2x4 SP #1; T2,T3 2x4 SP #2 N; Bot chord: 2x4 SP #1; Webs: 2x4 SP #2 N; W1,W7 2x4 SP #3;

Bracing

(a) 1X4 #3SRB or better continuous lateral restraint to be equally spaced. Attach with (2) 8d Box or Gun nails(0.113"x2.5",min.). Restraint material to be supplied and attached at both ends to a suitable support by erection contractor.

(a) or scab reinforcement may be used in lieu of CLR restraint. substitute (1) scab for (1) CLR and (2) scabs for (2) CLR's where shown. Scab reinforcement to be same size, species, grade, and 80% length of web member. Attach with 0.128x3" gun nails @ 6" oc.

Hangers / Ties

(J) Hanger Support Required, by others

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

B - C 1403 - 3321 E-F 1145 - 2171 C-D 1436 - 3071 F-G 1441 - 3076 Ď-E 1145 - 2171 - 3326 G-H 1408

Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs)

/Rh

/-

Wind reactions based on MWFRS Brg Wid = 8.0

Non-Gravity

/RL

/-

Tens. Comp.

/Rw /U

/1333 /311

/1258 /298

Min Reg = 2.3 (Truss)

Chords

Min Req =

Gravity

/R

Loc R+

1936 /-

Brg Wid = -

Chords Tens.Comp.

Bearing B is a rigid surface. Bearing B requires a seat plate.

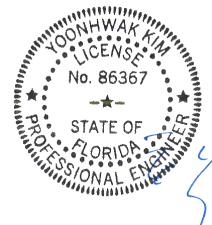
1870

В

Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

B - K 2841 - 1145 J - I 2360 - 848 K - J 2359 - 876 1 - H 2847 - 1130

Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. C - K .1 - F - 779 382 - 426 507 K - D 634 - 263 F - I 639 - 278 D-J 505 - 777 I-G 393 - 430 E - J 1398



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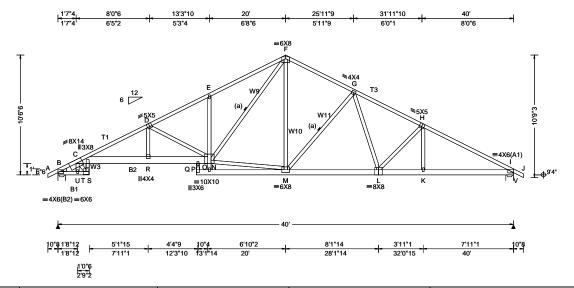
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SEQN: 103910/ COMN Ply: 1 Job Number: 402471 Cust: R 8975 JRef: 1YdO89750003 T49 / FROM: ARZ ,7F5 ,BRAA ,15;16;21;25 ,RB01 / BRADEN/A DrwNo: 267.25.0813.22776 Qty: 5 Truss Label: A1T / WHK 09/24/2025



| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | DefI/CSI Criteria | | |
|------------------------|---|------------------------------|---------------------------------|--|--|
| TCLL: 20.00 | Wind Std: ASCE 7-22 | Pg: NA Ct: NA CAT: NA | PP Deflection in loc L/defl L/# | | |
| TCDL: 15.00 | Speed: 160 mph | Pf: NA Ce: NA | VERT(LL): 0.273 E 999 240 | | |
| BCLL: 0.00 | Enclosure: Closed | Lu: NA Cs: NA | VERT(CL): 0.606 E 788 240 | | |
| BCDL: 10.00 | Risk Category: II | Snow Duration: NA | HORZ(LL): 0.135 I | | |
| Des Ld: 45.00 | EXP: C Kzt: NA Mean Height: 15.00 ft | | HORZ(TL): 0.299 I | | |
| NCBCLL: 10.00 | TCDL: 4.2 psf | Building Code: | Creep Factor: 2.0 | | |
| Soffit: 0.00 | BCDL: 5.0 psf | FBC 8th Ed. 2023 Res. | Max TC CSI: 0.747 | | |
| Load Duration: 1.25 | MWFRS Parallel Dist: h to 2h | TPI Std: 2014 | Max BC CSI: 0.712 | | |
| Spacing: 24.0 " | C&C Dist a: 4.00 ft | Rep Fac: Yes | Max Web CSI: 0.878 | | |
| - | Loc. from endwall: not in 13.00 ft | FT/RT:20(0)/10(0) | | | |
| | GCpi: 0.18 | Plate Type(s): | | | |
| | Wind Duration: 1.60 | WAVE | VIEW Ver: 23.02.01A.1204.18 | | |

Lumber

Top chord: 2x4 SP #1; T1 2x4 SP 2400f-2.0E; T3 2x4 SP #2 N; Bot chord: 2x6 SP SS; B1 2x4 SP #1; B2 2x8 SP 2400f-2.0E;

Webs: 2x4 SP #3; W3,W9,W10,W11 2x4 SP #2 N;

Lt Wedge: 2x4 SP #3;

Bracing

(a) 1X4 #3SRB or better continuous lateral restraint to be equally spaced. Attach with (2) 8d Box or Gun nails(0.113"x2.5",min.). Restraint material to be supplied and attached at both ends to a suitable support by erection contractor.

(a) or scab reinforcement may be used in lieu of CLR restraint. substitute (1) scab for (1) CLR and (2) scabs for (2) CLR'S where shown. Scab reinforcement to be same size, species, grade, and 80% length of web member. Attach with 0.128x3" gun nails @ 6" oc.

Plating Notes

All plates are 2X4 except as noted.

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design

Wind loading based on both gable and hip roof types.

Loc R+ /Rh /Rw /U /RL В 1983 /-/1332 /311 2023 /-/-/1335 /312 /-Wind reactions based on MWFRS Brg Wid = 8.0Min Reg = 2.3 (Truss) Brg Wid = 8.0 Min Req = 2.4 (Truss) Bearings B & I are a rigid surface. Bearings B & I require a seat plate.

Non-Gravity

▲ Maximum Reactions (lbs) Gravity

Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

| B - C | 1200 - 2782 | F-G | 1152 | - 2297 |
|-------|-------------|-------|------|--------|
| C - D | 1867 - 4737 | G-H | 1362 | - 3123 |
| D-E | 1502 - 3515 | H - I | 1400 | - 3509 |
| F-F | 1692 - 3516 | | | |

Maximum Bot Chord Forces Per Ply (lbs)

| Chords | Tens.Comp. | Chords | Tens. Comp. |
|--------|-------------|--------|-------------|
| B - U | 2330 - 1007 | P - N | 4118 - 1531 |
| C - T | 4097 - 1520 | M - L | 2523 - 806 |
| U - S | 2211 - 948 | L-K | 3022 - 1085 |
| T-R | 4165 - 1542 | K-I | 3023 - 1084 |
| R-P | 4148 - 1538 | | |

Maximum Web Forces Per Ply (lbs)

| vvebs | rens.comp. | webs | rens. v | Jonip. |
|-------|-------------|-------|---------|--------|
| C-U | 281 - 567 | N - F | 1796 | - 863 |
| C-S | 1347 - 3141 | N - M | 1932 | - 504 |
| T-S | 2228 - 952 | F-M | 606 | - 235 |
| R - D | 651 - 158 | M - G | 493 | - 860 |
| D - N | 575 - 1240 | G-L | 631 | - 196 |
| E - N | 396 - 439 | L-H | 347 | - 480 |
| | | | | |

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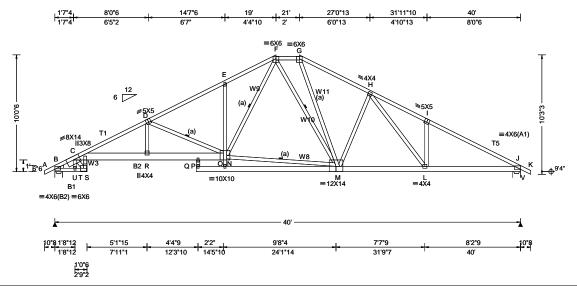
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SEQN: 103911/ MONO Ply: 1 Job Number: 402471 Cust: R 8975 JRef: 1YdO89750003 T4 / FROM: ARZ ,7F5 ,BRAA ,15;16;21;25 ,RB01 / BRADEN/A DrwNo: 267.25.0813.22888 Qty: 1 Truss Label: A2T / WHK 09/24/2025



| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | DefI/CSI Criteria |
|------------------------|--|------------------------------|---------------------------------|
| TCLL: 20.00 | Wind Std: ASCE 7-22 | Pg: NA Ct: NA CAT: NA | PP Deflection in loc L/defl L/# |
| TCDL: 15.00 | Speed: 160 mph | Pf: NA Ce: NA | VERT(LL): 0.258 E 999 240 |
| BCLL: 0.00 | Enclosure: Closed | Lu: NA Cs: NA | VERT(CL): 0.598 E 797 240 |
| BCDL: 10.00 | Risk Category: II | Snow Duration: NA | HORZ(LL): 0.134 J |
| Des Ld: 45.00 | EXP: C Kzt: NA | | HORZ(TL): 0.310 J |
| NCBCLL: 10.00 | Mean Height: 15.00 ft TCDL: 4.2 psf | Building Code: | Creep Factor: 2.0 |
| Soffit: 0.00 | BCDL: 5.0 psf | FBC 8th Ed. 2023 Res. | Max TC CSI: 0.758 |
| Load Duration: 1.25 | MWFRS Parallel Dist: h to 2h | TPI Std: 2014 | Max BC CSI: 0.684 |
| Spacing: 24.0 " | C&C Dist a: 4.00 ft | Rep Fac: Yes | Max Web CSI: 0.716 |
| | Loc. from endwall: not in 13.00 ft | | |
| | GCpi: 0.18 | Plate Type(s): | |
| | Wind Duration: 1.60 | WAVE | VIEW Ver: 23.02.01A.1204.18 |

Lumber

Top chord: 2x4 SP #2 N; T1 2x4 SP 2400f-2.0E; T5 2x4 SP #1: Bot chord: 2x6 SP #2 N; B1 2x4 SP #1; B2 2x8 SP 2400f-2.0E;

Webs: 2x4 SP #3; W3,W8,W9,W10,W11 2x4 SP #2 N;

Lt Wedge: 2x4 SP #3;

Bracing

(a) 1X4 #3SRB or better continuous lateral restraint to be equally spaced. Attach with (2) 8d Box or Gun nails(0.113"x2.5",min.). Restraint material to be supplied and attached at both ends to a suitable support by erection contractor.

(a) or scab reinforcement may be used in lieu of CLR restraint. substitute (1) scab for (1) CLR and (2) scabs for (2) CLR'S where shown. Scab reinforcement to be same size, species, grade, and 80% length of web member. Attach with 0.128x3" gun nails @ 6" oc.

Plating Notes

All plates are 2X4 except as noted.

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

| Pg: NA | Ct: NA | CAT: NA | PP Deflection | on in | loc L | _/defl | L/# | |
|------------|------------|---------|---------------|---------|-------|--------|-----|--|
| Pf: NA | | Ce: NA | VERT(LL): | 0.258 | 3 E | 999 | 240 | |
| Lu: NA | Cs: NA | | VERT(CL): | 0.598 | 3 E | 797 | 240 | |
| Snow Du | ration: NA | | HORZ(LL): | 0.134 | IJ | - | - | |
| | | | HORZ(TL): | 0.310 |) J | - | - | |
| Building (| Code: | | Creep Facto | or: 2.0 | | | | |
| FBC 8th I | Ed. 2023 I | Res. | Max TC CS | l: 0. | 758 | | | |
| TDI CAA. | 2044 | | May DO CC | ı. o | 604 | | | |

1934 /-/-/1336 /312 /-Wind reactions based on MWFRS Brg Wid = 8.0Min Reg = 2.3 (Truss) Brg Wid = 8.0 Min Req = 2.7 (Truss) Bearings B & J are a rigid surface. Bearings B & J require a seat plate. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 1227 - 2687 F-G 1251 - 1959 C-D 1969 - 4559 1377 - 2559 50 26

/Rh

Non-Gravity

/RL

/Rw /U

/1333 /311

▲ Maximum Reactions (lbs) Gravity

Loc R+

1931 /-

| | 1000 1000 | ~ :: | | |
|-----|-------------|-------|------|-------|
| D-E | 1511 - 3239 | H - I | 1622 | - 325 |
| E-F | 1693 - 3177 | I - J | 1456 | - 332 |
| | | | | |

Maximum Bot Chord Forces Per Ply (lbs)

| Tens.Comp. | Chords | Tens. Comp | ο. |
|-------------|--|--|--|
| 2243 - 1027 | R-P | 3996 - 163 | 36 |
| 3943 - 1615 | P - N | 3941 - 161 | 7 |
| 2129 - 967 | M - L | 2462 - 92 | 25 |
| 4011 - 1639 | L-J | 2855 - 113 | 34 |
| | 2243 - 1027 3943 - 1615 2129 - 967 | 2243 - 1027 R - P 3943 - 1615 P - N 2129 - 967 M - L | 2243 - 1027 R - P 3996 - 163 3943 - 1615 P - N 3941 - 161 2129 - 967 M - L 2462 - 92 |

Maximum Web Forces Per Ply (lbs)

| Webs | Tens.Comp. | Webs | Tens. (| Comp. |
|-------|-------------|-------|---------|-------|
| C - U | 284 - 544 | N - M | 1966 | - 648 |
| C-S | 1374 - 3025 | F-M | 314 | - 442 |
| T - S | 2145 - 970 | G - M | 762 | - 364 |
| R - D | 591 - 131 | M - H | 475 | - 652 |
| D - N | 662 - 1313 | H-L | 654 | - 357 |
| E - N | 374 - 393 | L-I | 372 | - 428 |
| N - F | 1559 - 740 | | | |

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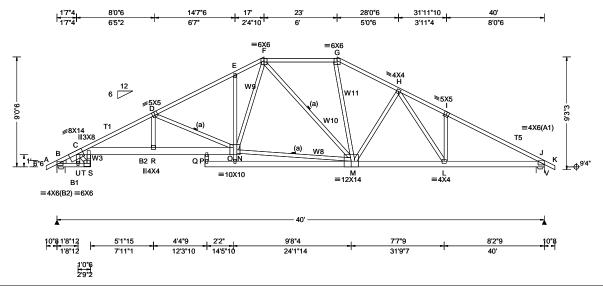
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SEQN: 103912/ MONO Ply: 1 Job Number: 402471 Cust: R 8975 JRef: 1YdO89750003 T3 / FROM: ARZ ,7F5 ,BRAA ,15;16;21;25 ,RB01 / BRADEN/A DrwNo: 267.25.0813.23278 Qty: 1 Truss Label: A3T / WHK 09/24/2025



| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | DefI/CSI Criteria |
|------------------------|--|------------------------------|---------------------------------|
| TCLL: 20.00 | Wind Std: ASCE 7-22 | Pg: NA Ct: NA CAT: NA | PP Deflection in loc L/defl L/# |
| TCDL: 15.00 | Speed: 160 mph | Pf: NA Ce: NA | VERT(LL): 0.251 E 999 240 |
| BCLL: 0.00 | Enclosure: Closed | Lu: NA Cs: NA | VERT(CL): 0.583 E 818 240 |
| BCDL: 10.00 | Risk Category: II | Snow Duration: NA | HORZ(LL): 0.135 J |
| Des Ld: 45.00 | EXP: C Kzt: NA | | HORZ(TL): 0.314 J |
| NCBCLL: 10.00 | Mean Height: 15.00 ft TCDL: 4.2 psf | Building Code: | Creep Factor: 2.0 |
| Soffit: 0.00 | BCDL: 5.0 psf | FBC 8th Ed. 2023 Res. | Max TC CSI: 0.759 |
| Load Duration: 1.25 | MWFRS Parallel Dist: h to 2h | TPI Std: 2014 | Max BC CSI: 0.682 |
| Spacing: 24.0 " | C&C Dist a: 4.00 ft | Rep Fac: Yes | Max Web CSI: 0.778 |
| - | Loc. from endwall: not in 13.00 ft | FT/RT:20(0)/10(0) | |
| | GCpi: 0.18 | Plate Type(s): | |
| | Wind Duration: 1.60 | WAVE | VIEW Ver: 23.02.01A.1204.18 |

Lumber

Top chord: 2x4 SP #2 N; T1 2x4 SP 2400f-2.0E; T5 2x4 SP #1: Bot chord: 2x6 SP #2 N; B1 2x4 SP #1; B2 2x8 SP 2400f-2.0E;

Webs: 2x4 SP #3; W3,W8,W9,W10,W11 2x4 SP #2 N;

Lt Wedge: 2x4 SP #3;

Bracing

(a) 1X4 #3SRB or better continuous lateral restraint to be equally spaced. Attach with (2) 8d Box or Gun nails(0.113"x2.5",min.). Restraint material to be supplied and attached at both ends to a suitable support by erection contractor.

(a) or scab reinforcement may be used in lieu of CLR restraint. substitute (1) scab for (1) CLR and (2) scabs for (2) CLR'S where shown. Scab reinforcement to be same size, species, grade, and 80% length of web member. Attach with 0.128x3" gun nails @ 6" oc.

Plating Notes

All plates are 2X4 except as noted.

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

| Pg: NA | Ct: NA | CAT: NA | PP Deflection | on in loc | : I /defl | I /# | | G | ravity | | No | on-Gra | vity |
|----------|---|---------|---------------|-----------|-----------|------|-----|---------|----------|--------------|--------------|--------|------|
| Pf: NA | • | Ce: NA | VERT(LL): | | | 240 | Loc | : R+ | / R- | / Rh | / Rw | / U | / RL |
| Lu: NA | Cs: NA | | VERT(CL): | 0.583 E | 818 | 240 | В | 1931 | /- | /- | /1330 | /319 | /382 |
| Snow Du | uration: NA | 1 | HORZ(LL): | 0.135 J | - | - | v | 1934 | /- | /- | /1334 | | /- |
| | | | HORZ(TL): | 0.314 J | - | - | Wir | nd read | tions b | ased on M | IWFRS | | |
| Building | Code: | | Creep Facto | or: 2.0 | | | В | Brg V | Vid = 8 | .0 Min R | eq = 2.3 | (Trus | s) |
| FBC 8th | Ed. 2023 | Res. | Max TC CS | l: 0.75 | 9 | | ٧ | • | | .0 Min R | • | (Trus | s) |
| TPI Std: | 2014 | | Max BC CS | i: 0.68 | 2 | | | | | re a rigid s | | | |
| Rep Fac | : Yes | | Max Web C | SI: 0.77 | 8 | | l | • | | equire a se | | | |
| ET/DT.2 | 0/0\/40/0\ | | | | | | Me | mbers | not list | ed have fo | rces less | than : | 3/5# |

B - C 1319 - 2683 F-G 1407 - 2132 C-D 2192 - 4559 G-H 1493 - 2555 D-E 1704 - 3240 H - I 1760 - 3248 E-F 1834 - 3116 1605 - 3328

Maximum Top Chord Forces Per Ply (lbs)

Non-Gravity

/382

Tens. Comp.

Chords

Maximum Bot Chord Forces Per Ply (lbs)

▲ Maximum Reactions (lbs) Gravity

Chords Tens.Comp.

| Chords | Tens.Comp. | Chords | Tens. Comp. | |
|--------|-------------|--------|-------------|--|
| B-U | 2238 - 1109 | R-P | 3998 - 1838 | |
| C - T | 3943 - 1814 | P - N | 3939 - 1815 | |
| U-S | 2125 - 1045 | M - L | 2522 - 1102 | |
| T - R | 4012 - 1841 | L - J | 2857 - 1276 | |

Maximum Web Forces Per Ply (lbs)

| Webs | Tens.Comp. | Webs | Tens. C | Comp. |
|-------|-------------|-------|---------|-------|
| C - U | 304 - 542 | N - M | 2268 | - 953 |
| C-S | 1484 - 3019 | F-M | 282 | - 482 |
| T - S | 2140 - 1048 | G - M | 674 | - 257 |
| R - D | 585 - 142 | M - H | 388 | - 559 |
| D - N | 698 - 1317 | H-L | 616 | - 331 |
| N-F | 1355 - 651 | L-I | 367 | - 418 |

COA#0-278, Yoonhwak Kim, FL PE #86367 Fload24Ce2fficate of Product Approval #FL 1999

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

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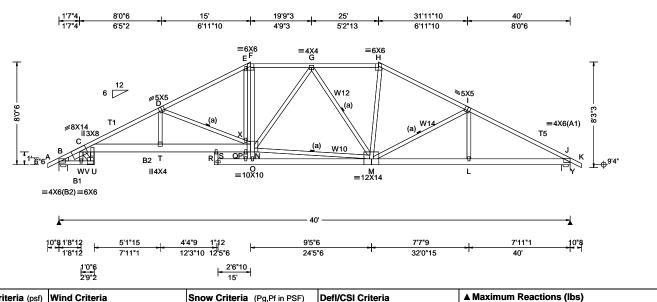
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have continuous lateral restraint (CLR), installed with diagonal bracing installed on the CLR per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec. 2.

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155 Harlem Ave North Building, 4th Floor Glenview, IL 60025

SEQN: 103913/ HIPS Ply: 1 Job Number: 402471 Cust: R 8975 JRef: 1YdO89750003 T50 / FROM: ARZ ,7F5 ,BRAA ,15;16;21;25 ,RB01 / BRADEN/A Qty: 1 DrwNo: 267.25.0813.22619 Truss Label: A4T / WHK 09/24/2025



| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | |
|------------------------|--|------------------------------|---------------------------------|---|
| TCLL: 20.00 | Wind Std: ASCE 7-22 | Pg: NA Ct: NA CAT: NA | PP Deflection in loc L/defl L/# | |
| TCDL: 15.00 | Speed: 160 mph | Pf: NA Ce: NA | VERT(LL): 0.247 N 999 240 | |
| BCLL: 0.00 | Enclosure: Closed | Lu: NA Cs: NA | VERT(CL): 0.573 N 833 240 | |
| BCDL: 10.00 | Risk Category: II | Snow Duration: NA | HORZ(LL): 0.137 J | |
| Des Ld: 45.00 | EXP: C Kzt: NA | | HORZ(TL): 0.318 J | |
| NCBCLL: 10.00 | Mean Height: 15.00 ft TCDL: 4.2 psf | Building Code: | Creep Factor: 2.0 | |
| Soffit: 0.00 | BCDL: 5.0 psf | FBC 8th Ed. 2023 Res. | Max TC CSI: 0.728 | |
| Load Duration: 1.25 | MWFRS Parallel Dist: h to 2h | TPI Std: 2014 | Max BC CSI: 0.715 | |
| Spacing: 24.0 " | C&C Dist a: 4.00 ft | Rep Fac: Yes | Max Web CSI: 0.915 | |
| ' ' | Loc. from endwall: not in 13.00 ft | FT/RT:20(0)/10(0) | | |
| | GCpi: 0.18 | Plate Type(s): | | 1 |
| | Wind Duration: 1.60 | WAVE | VIEW Ver: 23.02.01A.1204.18 | |
| | | | | - |

Lumber

Top chord: 2x4 SP #2 N; T1 2x4 SP 2400f-2.0E; T5 2x4 SP #1: Bot chord: 2x6 SP #2 N; B1 2x4 SP #1; B2 2x8 SP 2400f-2.0E; Webs: 2x4 SP #3; W10,W12,W14 2x4 SP #2 N; Lt Wedge: 2x4 SP #3;

Bracing

(a) 1X4 #3SRB or better continuous lateral restraint to be equally spaced. Attach with (2) 8d Box or Gun nails(0.113"x2.5",min.). Restraint material to be supplied and attached at both ends to a suitable support by erection contractor.

(a) or scab reinforcement may be used in lieu of CLR restraint. substitute (1) scab for (1) CLR and (2) scabs for (2) CLR'S where shown. Scab reinforcement to be same size, species, grade, and 80% length of web member. Attach with 0.128x3" gun nails @ 6" oc.

Plating Notes

All plates are 2X4 except as noted.

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

| B-C | 1408 - 2688 | F-G | 1739 | - 2715 |
|-------|-------------|-------|------|--------|
| C - D | 2387 - 4567 | G-H | 1570 | - 2348 |
| D-E | 1805 - 3139 | H - I | 1600 | - 2677 |
| E-F | 1852 - 2931 | I - J | 1754 | - 3351 |

Brg Wid = 8.0 Min Req = 2.7 (Truss)

Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs)

Non-Gravity

/RL

/-

/Rw /U

/1319 /331

Chords Tens. Comp.

/1324 /428

Min Reg = 2.3 (Truss)

Maximum Bot Chord Forces Per Ply (lbs)

/Rh

/-

Wind reactions based on MWFRS Brg Wid = 8.0

Bearings B & J are a rigid surface. Bearings B & J require a seat plate.

Gravity

Chords Tens.Comp.

Loc R+

1931 /-

1934 /-

В

| Cnoras | rens.Comp. | Cnoras | rens. Comp. | |
|--------|-------------|--------|-------------|--|
| B - W | 2244 - 1175 | R - P | 3888 - 1954 | |
| C - V | 3951 - 1989 | P-N | 3819 - 1932 | |
| W - U | 2130 - 1108 | M - L | 2883 - 1420 | |
| V - T | 4019 - 2019 | L-J | 2885 - 1418 | |
| T-R | 4003 - 2014 | | | |

Maximum Web Forces Per Ply (lbs)

| Webs | Tens.Comp. | Webs | Tens. Comp. | |
|-------|-------------|-------|-------------|--|
| C-W | 321 - 544 | F-N | 973 - 503 | |
| C - U | 1574 - 3026 | N - M | 2512 - 1248 | |
| V - U | 2146 - 1111 | G - M | 461 - 583 | |
| T - D | 648 - 179 | M - H | 706 - 340 | |
| D - X | 792 - 1404 | M - I | 408 - 687 | |
| X - N | 823 - 1409 | | | |

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For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

SEQN: 230762/ HIPS Ply: 1 Job Number: 402471 Cust: R 8975 JRef: 1YdO89750003 T5 / FROM: ARZ ,7F5 ,BRAA ,15;16;21;25 ,RB01 / BRADEN/A DrwNo: 267.25.0813.23371 Qty: 1 Truss Label: A5T AK / YK 09/24/2025 7'8"4 13' 14'7"6 20' 34'11"4 40' 6'1" 5'3"12 5'4"10 7'11"4 5'0"12 ≡6X6 E ≡4X4 G ≡H0610 H F 6 12 W5 W10 (a) (a) QP B2 R ≡4X4 =4X6(B2) UT S =4X4 M ≡5X5 =4X4 ∥6X6 =4X4 ≡4X6(B2) 14'7"6 25'4"10

9'8"4

24'1"14

| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria |
|------------------------|--|------------------------------|---------------------------------|
| TCLL: 20.00 | Wind Std: ASCE 7-22 | Pg: NA Ct: NA CAT: NA | PP Deflection in loc L/defl L/# |
| 1.0220.00 | Speed: 160 mph | Pf: NA Ce: NA | VERT(LL): -0.461 S 378 240 |
| DCLL. 0.00 | Enclosure: Closed | Lu: NA Cs: NA | VERT(CL): 0.384 S 454 240 |
| 10.00 | Risk Category: II | Snow Duration: NA | HORZ(LL): -0.246 J |
| Dec 1 d: 45 00 | EXP: C Kzt: NA | | HORZ(TL): 0.286 J |
| NODOLL, 40 00 | Mean Height: 15.00 ft TCDL: 4.2 psf | Building Code: | Creep Factor: 2.0 |
| 0.00 | BCDL: 5.0 psf | FBC 8th Ed. 2023 Res. | Max TC CSI: 0.881 |
| | MWFRS Parallel Dist: h/2 to h | TPI Std: 2014 | Max BC CSI: 0.857 |
| Spacing: 24.0 " | C&C Dist a: 4.00 ft | Rep Fac: Yes | Max Web CSI: 0.833 |
| | Loc. from endwall: not in 13.00 ft | FT/RT:20(0)/10(0) | |
| | GCpi: 0.18 | Plate Type(s): | |
| | Wind Duration: 1.60 | WAVE, HS | VIEW Ver: 23.02.01A.1204.18 |

2'10"10 2'0"4

12'4"14 14'5"10

0"8 12'5"6

| | ▲ N | laximu | ım Rea | ctions (I | bs) | | |
|-----|------------|----------|-----------|------------|------------|--------|------|
| L/# | | G | ravity | | No | n-Grav | /ity |
| 240 | Loc | R+ | / R- | / Rh | / Rw | / U | /RI |
| 240 | В | 355 | /- | /- | /203 | /720 | /30 |
| - | 0 | 2573 | /- | /- | /1745 | /- | /- |
| _ | ٧ | 1074 | /- | /- | /867 | /720 | /- |
| | Wir | nd read | tions b | ased on I | MWFRS | | |
| | В | Brg V | /id = 8. | 0 Min f | Req = 1.5 | (Truss | 5) |
| | 0 | Brg V | /id = 3. | 5 Min f | Req = 3.2 | (Truss | s) |
| | ٧ | Brg V | /id = 8. | .0 Min I | Req = 1.5 | (Truss | s) |
| | Bea | arings I | 3, O, & | J are a ri | gid surfac | e. | |
| | Bea | arings I | 3 & J re | equire a s | eat plate. | | |
| | Mei | mbers | not liste | ed have fo | orces less | than 3 | 375# |
| 18 | _ | | | | rces Per | | - |
| | | | | | | | |

7'3"6

8'6"12

32'8"10

| Chords | Tens.C | comp. | Chords | Tens. | Comp. |
|--------|--------|-------|--------|-------|--------|
| B-C | 1231 | - 279 | F-G | 1717 | -76 |
| C - D | 2392 | - 62 | G - H | 1724 | -632 |
| D - E | 2373 | - 45 | H - I | 1827 | - 1463 |
| E-F | 1748 | - 75 | I - J | 1874 | - 1725 |

/300

Maximum Bot Chord Forces Per Plv (lbs)

| Chords | Tens.Comp. | Chords | Tens. Comp. |
|--------|------------|--------|-------------|
| B - U | 206 - 927 | R-P | 39 - 1314 |
| C-T | 132 - 2124 | O - M | 229 - 1503 |
| U - S | 207 - 876 | M - L | 814 - 1232 |
| T - R | 126 - 2052 | L-J | 1474 - 1570 |
| | | | |

Maximum Web Forces Per Ply (lbs)

| vvebs | rens.comp. | webs | rens. | comp. |
|-------|------------|-------|-------|--------|
| c-s | 1223 - 286 | N - O | 373 | - 903 |
| T - S | 196 - 858 | O - G | 330 | - 1519 |
| D - R | 458 - 480 | G - M | 850 | 0 |
| R-E | 499 - 946 | M - H | 74 | - 555 |
| P - O | 40 - 1263 | H - L | 603 | - 193 |
| E - N | 200 - 870 | L-I | 352 | - 434 |

| Cilolus | rens.comp. | Onorus | rens. comp. |
|---------|------------|--------|-------------|
| B - U | 206 - 927 | R-P | 39 - 1314 |
| C - T | 132 - 2124 | O - M | 229 - 1503 |
| U-S | 207 - 876 | M - L | 814 - 1232 |
| T - R | 126 - 2052 | L - J | 1474 - 1570 |
| | | | |

| Webs | Tens.Comp. | Webs | Tens. | Comp. |
|-------|------------|-------|-------|--------|
| C-S | 1223 - 286 | N - O | 373 | - 903 |
| T - S | 196 - 858 | O - G | 330 | - 1519 |
| D - R | 458 - 480 | G - M | 850 | 0 |
| R-E | 499 - 946 | M - H | 74 | - 555 |
| P - O | 40 - 1263 | H-L | 603 | - 193 |
| E - N | 200 - 870 | L-I | 352 | - 434 |
| • | | | | |

COA#0-278, Yoonhwak Kim, FL PE #86367 Fl09/24/2026 ficate of Product Approval #FL 1999

Lumber

Top chord: 2x4 SP #2 N; Bot chord: 2x4 SP #2 N; B2 2x4 SP #1; Webs: 2x4 SP #3; W5,W10,W13 2x4 SP #2 N; Lt Wedge: 2x4 SP #3;Rt Wedge: 2x4 SP #3;

10"8 1'8"12

1'8"12

6'9"2

9'6"4

Bracing

(a) 1X4 #3SRB or better continuous lateral restraint to be equally spaced. Attach with (2) 8d Box or Gun nails(0.113"x2.5",min.). Restraint material to be supplied and attached at both ends to a suitable support by erection contractor.

(a) or scab reinforcement may be used in lieu of CLR restraint. substitute (1) scab for (1) CLR and (2) scabs for (2) CLR'S where shown. Scab reinforcement to be same size, species, grade, and 80% length of web member. Attach with 0.128x3" gun nails @ 6" oc.

Plating Notes

All plates are 2X4 except as noted.

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Laterally brace top chord below filler and bottom chord above filler at 24" o.c., including a lateral brace at chord ends (If no rigid diaphragm exists at that point).

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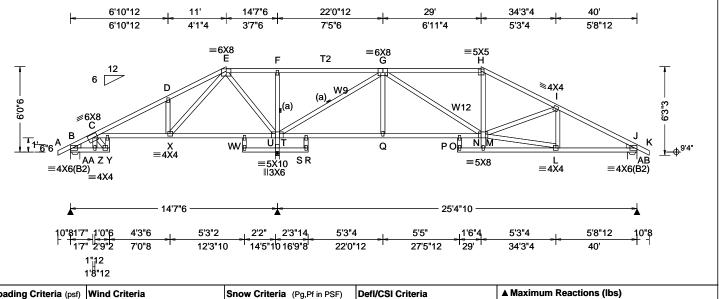
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SEQN: 837523 / HIPS Ply: 1 Job Number: 402471 Cust: R 8975 JRef: 1YdO89750003 T8 / FROM: ARZ Qty: 1 ,7F5 ,BRAA ,15;16;21;25 ,RB01 / BRADEN/A DrwNo: 267.25.0813.24008 Truss Label: A6T KD / YK 09/24/2025



| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria |
|------------------------|---|------------------------------|---------------------------------|
| TCLL: 20.00 | Wind Std: ASCE 7-22 | Pg: NA Ct: NA CAT: NA | PP Deflection in loc L/defl L/# |
| TCDL: 15.00 | Speed: 160 mph | Pf: NA Ce: NA | VERT(LL): 0.066 P 999 240 |
| BCLL: 0.00 | Enclosure: Closed | Lu: NA Cs: NA | VERT(CL): 0.204 Y 853 240 |
| BCDL: 10.00 | Risk Category: II | Snow Duration: NA | HORZ(LL): 0.032 J |
| Des Ld: 45.00 | EXP: C Kzt: NA Mean Height: 15.00 ft | | HORZ(TL): 0.075 J |
| NCBCLL: 10.00 | TCDL: 4.2 psf | Building Code: | Creep Factor: 2.0 |
| Soffit: 0.00 | BCDL: 5.0 psf | FBC 8th Ed. 2023 Res. | Max TC CSI: 0.703 |
| Load Duration: 1.25 | MWFRS Parallel Dist: h/2 to h | TPI Std: 2014 | Max BC CSI: 0.605 |
| Spacing: 24.0 " | C&C Dist a: 4.00 ft | Rep Fac: Yes | Max Web CSI: 0.813 |
| | Loc. from endwall: not in 6.50 ft | FT/RT:20(0)/10(0) | |
| | GCpi: 0.18 | Plate Type(s): | |
| | Wind Duration: 1.60 | WAVE | VIEW Ver: 23.02.01A.1204.18 |
| Lumber | | | |

Top chord: 2x4 SP #2 N; T2 2x4 SP #1; Bot chord: 2x4 SP #2 N; Webs: 2x4 SP #3; W9,W12 2x4 SP #2 N; Lt Wedge: 2x4 SP #3;Rt Wedge: 2x4 SP #3;

Bracing

(a) 1X4 #3SRB or better continuous lateral restraint to be equally spaced. Attach with (2) 8d Box or Gun nails(0.113"x2.5",min.). Restraint material to be supplied and attached at both ends to a suitable support by erection contractor.

(a) or scab reinforcement may be used in lieu of CLR restraint. substitute (1) scab for (1) CLR and (2) scabs for (2) CLR'S where shown. Scab reinforcement to be same size, species, grade, and 80% length of web member. Attach with 0.128x3" gun nails @ 6" oc.

Plating Notes

All plates are 2X4 except as noted.

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Note: Laterally brace bottom chord above filler at 2'0" O.C.Max. including a lateral brace at chord ends.

| ı | U | 2634 | /- | /- | /1/2 | 3 | /964 | /- | |
|---|---|--------|----------|----------|-----------|-----|--------|--------|--|
| ı | AB | 1055 | /- | /- | /833 | | /370 | /- | |
| ı | Win | d read | ctions b | ased on | MWFRS | 3 | | | |
| ı | В | Brg V | Vid = 8 | .0 Min | Req = 1 | .5 | (Truss | s) | |
| ı | U | Brg V | Vid = 3 | .5 Min | Req = 3 | .3 | (Truss | s) | |
| ı | AB | Brg V | Vid = 8 | .0 Min | Req = 1 | .5 | (Truss | s) | |
| ı | Bearings B, U, & J are a rigid surface. | | | | | | | | |
| ı | Bea | rings | B&Jr | equire a | seat plat | e. | | | |
| 4 | Mer | nbers | not list | ed have | forces le | SS | than 3 | 375# | |
| ı | Max | cimun | Top (| Chord F | orces Pe | r F | ly (lb | s) | |
| _ | Cho | ords 1 | Γens.C | omp. | Chords | • | Tens. | Ćomp. | |
| | C - | D | 469 | - 141 | G-H | | 810 | - 1193 | |
| | Č | _ | 440 | 40 | ŭ :: | | 040 | 4404 | |

/Rh

Non-Gravity

/109

/RL

/Rw /U

Gravity

Loc R+

В

| C-D D-F | - 141 - 18 | G - H H - I | - 1193 - 1404 |
|------------|-------------------|----------------|----------------------|
| E-F F-G | - 594 | i - J | - 1612 |

Maximum Bot Chord Forces Per Ply (lbs)

| Chords | Tens.Comp. | Chords | Tens. | Comp. |
|--------|------------|--------|-------|-------|
| X - V | 534 - 789 | Q-0 | 648 | - 255 |
| V - T | 538 - 773 | 0 - M | 638 | - 240 |
| T - S | 663 - 250 |) L-J | 1353 | - 596 |
| S-Q | 648 - 255 | 5 | | |

Maximum Web Forces Per Ply (lbs)

| Webs | Tens.Comp. | Webs | Tens. Comp. |
|-------|------------|-------|-------------|
| D-X | 368 - 415 | T - U | 1477 - 2508 |
| X - E | 808 - 430 | T - G | 1300 - 2149 |
| E-T | 656 - 1224 | G - M | 881 - 323 |
| F-T | 514 - 473 | M - L | 1342 - 580 |

COA#0-278, Yoonhwak Kim, FL PE #86367 Flood 24 Oct Ficate of Product Approval #FL 1999

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SEQN: 837526 / HIPS Ply: 1 Job Number: 402471 Cust: R 8975 JRef: 1YdO89750003 T10 / Qty: 1 FROM: ARZ ,7F5 ,BRAA ,15;16;21;25 ,RB01 / BRADEN/A DrwNo: 267.25.0813.23403 Truss Label: A7T KD / YK 09/24/2025 14'7"6 22'0"12 28'5"12 31' 35'5" 40' 4'5" 4'7' 5'7"6 7'5"6 6'5' 2'6"4 4'5' =5X5 D =6X8 =5X5 Ε G .(a) P O Q _5X8 υ^βΤ ≡5Χ5 SR =6X6 =4X10 =4X6(B2)14'7"6 25'4"10 8'8'8 5'9"2 5'3"4 10"4 8'10"4 10"8 5'5" 10"8

22'0"12

27'5"12

28'4"

2'9"12

31'1"12

| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria |
|------------------------|--|------------------------------|---------------------------------|
| TCLL: 20.00 | Wind Std: ASCE 7-22 | Pg: NA Ct: NA CAT: NA | PP Deflection in loc L/defl L/# |
| TCDL: 15.00 | Speed: 160 mph | Pf: NA Ce: NA | VERT(LL): 0.104 G 999 240 |
| BCLL: 0.00 | Enclosure: Closed | Lu: NA Cs: NA | VERT(CL): 0.242 G 999 240 |
| BCDL: 10.00 | Risk Category: II | Snow Duration: NA | HORZ(LL): 0.022 J |
| Des Ld: 45.00 | EXP: C Kzt: NA | | HORZ(TL): 0.051 J |
| NCBCLL: 10.00 | Mean Height: 15.00 ft TCDL: 4.2 psf | Building Code: | Creep Factor: 2.0 |
| Soffit: 0.00 | BCDL: 5.0 psf | FBC 8th Ed. 2023 Res. | Max TC CSI: 0.781 |
| Load Duration: 1.25 | MWFRS Parallel Dist: h/2 to h | TPI Std: 2014 | Max BC CSI: 0.805 |
| Spacing: 24.0 " | C&C Dist a: 4.00 ft | Rep Fac: Yes | Max Web CSI: 0.833 |
| | Loc. from endwall: not in 6.50 ft | FT/RT:20(0)/10(0) | |
| | GCpi: 0.18 | Plate Type(s): | |
| | Wind Duration: 1.60 | WAVE | VIEW Ver: 23.02.01A.1204.18 |

14'5"10

14'9"2

2'0"6

16'9"8

▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ /Rh /Rw /U В 680 /504 /269 2020 /-/-/1200 /712 1212 /894 /467 Wind reactions based on MWFRS Brg Wid = 8.0 Min Req = 1.5 (Truss) Brg Wid = 3.5Min Req = 2.4 (Truss) Brg Wid = 8.0 Min Req = 1.7 (Truss) Bearings B, U, & J are a rigid surface. Bearings B & J require a seat plate. Members not listed have forces less than 375#

40'

Chords Tens.Comp. Chords Tens. Comp. B - C 503 - 879 G-H 1430 - 2008 343 - 524 C-D H - I 1050 - 1640 F-G 1432 - 2015 1 - .1 1169 - 1943

Maximum Top Chord Forces Per Ply (lbs)

/RL

/218

-718

1496

/-

Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - V 724 - 388 Q - O 1588 -878 T - S 1640 - 904 O - M 1574 - 853 S-Q 1588 - 878 1655 - 905 L-J

Maximum Web Forces Per Ply (lbs) Tens.Comp. Webs Tens. Comp. 1198 2021 415 - 104 F - M 634 - 284 - 732 G - M 441 - 425 326 1025 - 1459 M - H 1001 - 723

M - L

- 561

Webs C-V V - D D-U U - T E-T 585

COA#0-278, Yoonhwak Kim, FL PE #86367 Flood24Oe2fficate of Product Approval #FL 1999

Lumber

Top chord: 2x4 SP #2 N; Bot chord: 2x4 SP #2 N; Webs: 2x4 SP #3;

Lt Wedge: 2x4 SP #3;Rt Wedge: 2x4 SP #3;

Bracing

(a) 1X4 #3SRB or better continuous lateral restraint to be equally spaced. Attach with (2) 8d Box or Gun nails(0.113"x2.5",min.). Restraint material to be supplied and attached at both ends to a suitable support by erection contractor.

8'8'8

(a) or scab reinforcement may be used in lieu of CLR restraint. substitute (1) scab for (1) CLR and (2) scabs for (2) CLR'S where shown. Scab reinforcement to be same size, species, grade, and 80% length of web member. Attach with 0.128x3" gun nails @ 6" oc.

Plating Notes

All plates are 2X4 except as noted.

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Note: Laterally brace bottom chord above filler at 2'0" O.C.Max. including a lateral brace at chord ends.

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

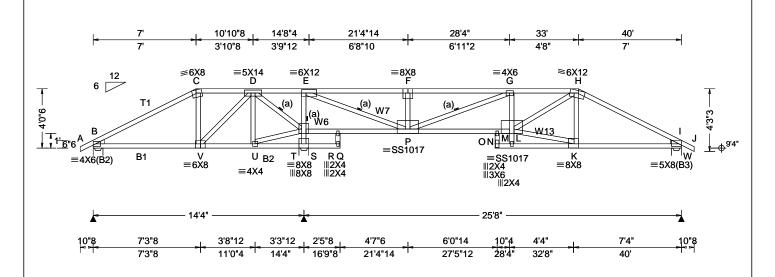
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For more information see these web sites: Alpine: alpineitw.com: TPI: binst.org: SBCA: sbcacomponents.com: ICC: iccsafe.org: AWC: awc.or



SEQN: 837770 HIPS Ply: 1 Job Number: 402471 Cust: R 8975 JRef: 1YdO89750003 T39 FROM: ARZ Qty: 1 ,7F5 ,BRAA ,15;16;21;25 ,RB01 / BRADEN/A DrwNo: 267.25.0920.32770 Page 1 of 2 Truss Label: A8GT KD / YK 09/24/2025



| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria |
|------------------------|---|------------------------------|---------------------------------|
| TCLL: 20.00 | Wind Std: ASCE 7-22 | Pg: NA Ct: NA CAT: NA | PP Deflection in loc L/defl L/# |
| TCDL: 15.00 | Speed: 160 mph | Pf: NA Ce: NA | VERT(LL): 0.266 G 999 240 |
| BCLL: 0.00 | Enclosure: Closed | Lu: NA Cs: NA | VERT(CL): 0.603 G 509 240 |
| BCDL: 10.00 | Risk Category: II | Snow Duration: NA | HORZ(LL): 0.057 I |
| Des Ld: 45.00 | EXP: C Kzt: NA Mean Height: 15.00 ft | | HORZ(TL): 0.129 I |
| NCBCLL: 10.00 | TCDL: 4.2 psf | Building Code: | Creep Factor: 2.0 |
| Soffit: 0.00 | BCDL: 5.0 psf | FBC 8th Ed. 2023 Res. | Max TC CSI: 0.955 |
| Load Duration: 1.25 | MWFRS Parallel Dist: 0 to h/2 | TPI Std: 2014 | Max BC CSI: 0.878 |
| Spacing: 24.0 " | C&C Dist a: 4.00 ft | Rep Fac: Varies by Ld Case | Max Web CSI: 0.950 |
| | Loc. from endwall: not in 6.50 ft | FT/RT:20(0)/10(0) | |
| | GCpi: 0.18 | Plate Type(s): | |
| | Wind Duration: 1.60 | WAVE, 18SS | VIEW Ver: 23.02.01A.1204.18 |
| Lumber | | | |

| eria | | | 4 | |
|--------------------|-----|-----|---|--|
| n in lo 0.266 G | | | L | |
| 0.603 | 509 | 240 | В | |
| 0.057 I | - | - | Т | |
| 0.129 I | - | - | ٧ | |
| : 2.0 | | | ٧ | |
| 0.95 | 5 | | В | |
| 0.87 | 8 | | T | |
| l: 0.95 | 0 | | V | |
| | | | В | |
| | | | Ň | |
| 3.02.01A.1204.18 | | | | |
| | | | _ | |

| ▲ M | ▲ Maximum Reactions (lbs) | | | | | | |
|--|---|-----------|-----------|--------------|--------|-------|--|
| | (| 3ravity | | No | n-Grav | ity | |
| Loc | R+ | / R- | / Rh | / Rw | / U | / RL | |
| В | 815 | /- | /- | /- | /260 | /- | |
| Т | 5998 | /- | /- | /- | /2602 | /- | |
| W | 2445 | /- | /- | /- | /995 | /- | |
| Win | nd rea | ctions ba | sed on N | IWFRS | | | |
| В | Brg \ | Nid = 8.0 | Min F | Req = 1.5 | (Truss |) | |
| Т | Brg \ | Nid = 8.0 | Min F | Req = 7.1 | (Truss |) | |
| W | Brg \ | Nid = 8.0 | Min F | Req = 2.0 | (Truss |) | |
| Bea | arings | B, T, & I | are a rig | id surface | e | | |
| Bea | arings | B, T, & I | require a | a seat pla | te. | | |
| Mer | Members not listed have forces less than 375# | | | | | | |
| Maximum Top Chord Forces Per Ply (lbs) | | | | | | | |
| Cho | ords | Tens.Cor | np. (| Chords | Tens. | Ćomp. | |
| | _ | | | | | | |

| . oo.o. | 00.00 | | О Ор. | |
|-------------|--|--|---|--|
| 308 - 1052 | F-G | 1366 | - 3342 | |
| 180 - 796 | G - H | 2478 | - 5949 | |
| 3435 - 1599 | H - I | 1813 | - 4390 | |
| 1366 - 3342 | | | | |
| | 308 - 1052 180 - 796 3435 - 1599 | 308 - 1052 F - G 180 - 796 G - H 3435 - 1599 H - I | 308 - 1052 F - G 1366 180 - 796 G - H 2478 3435 - 1599 H - I 1813 | |

Maximum Bot Chord Forces Per Ply (lbs)

Bracing

B2 2x4 SP #1;

W7 2x4 SP 2400f-2.0E;

(a) 1X4 #3SRB or better continuous lateral restraint to be equally spaced. Attach with (2) 8d Box or Gun nails(0.113"x2.5",min.). Restraint material to be supplied and attached at both ends to a suitable support by erection contractor.

Top chord: 2x4 SP 2400f-2.0E; T1 2x4 SP #2 N; Bot chord: 2x4 SP 2400f-2.0E; B1 2x4 SP #2 N;

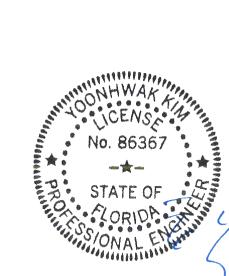
Webs: 2x4 SP #3; W6,W13 2x4 SP #2 N;

Lt Wedge: 2x4 SP #3;Rt Wedge: 2x4 SP #3;

(a) or scab reinforcement may be used in lieu of CLR restraint. substitute (1) scab for (1) CLR and (2) scabs for (2) CLR'S where shown. Scab reinforcement to be same size, species, grade, and 80% length of web member. Attach with 0.128x3" gun nails @ 6" oc.

Wind loads and reactions based on MWFRS. Wind loading based on both gable and hip roof types.

Note: Laterally brace bottom chord above filler at 2'0" O.C.Max. including a lateral brace at chord ends.



| Cilolus | rens.comp. | Chorus | rens. Comp. |
|---------|-------------|--------|-------------|
| B - V | 841 - 221 | R-P | 1539 - 3317 |
| V - U | 386 - 824 | P - N | 5995 - 2507 |
| U - T | 160 - 385 | N - L | 5903 - 2460 |
| S-R | 1500 - 3185 | K - I | 3794 - 1552 |

Maximum Web Forces Per Ply (lbs)

| vvebs | rens.comp. | vvebs | rens. Comp. |
|----------------|---------------------------|------------|---------------------------|
| C - V V - D | 540 - 651 | E-P F-P | 7158 - 3124 753 - 1090 |
| V - D D - S | 2015 - 816 1504 - 3408 | P-G | 1227 - 2863 |
| U - S | 239 - 462 | L-K | 3692 - 1486 |
| S - T | 2530 - 5402 | L-H | 2494 - 1095 |
| Ş-E | 1791 - 3570 | | |

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SEQN: 837770 HIPS Ply: 1 Job Number: 402471 Cust: R 8975 JRef:1YdO89750003 T39 FROM: ARZ Qty: 1 ,7F5 ,BRAA ,15;16;21;25 ,RB01 / BRADEN/A DrwNo: 267.25.0920.32770 Page 2 of 2 Truss Label: A8GT KD / YK 09/24/2025

Special Loads

| • | | | | | | | |
|-------|--------|-----------|-------|---------|--------|---------|-------|
| (I | Lumber | Dur.Fac.= | 1.25 | / Plate | Dur.F | ac.=1. | .25) |
| TC: | From | 74 plf at | -(| 0.88 to | 74 | plf at | 7.00 |
| TC: | From | 37 plf at | 7 | 7.00 to | 37 | plf at | 33.00 |
| TC: | From | 74 plf at | 33 | 3.00 to | 74 | plf at | 40.88 |
| BC: | From | 20 plf at | : (| 0.00 to | 20 | plf at | 7.29 |
| BC: | From | 10 plf at | | | | plf at | 32.97 |
| | From | | | | | plf at | 40.00 |
| TC: | 522 lb | Conc. Loa | ad at | 7.03,3 | 2.97 | | |
| TC: | 237 lb | Conc. Loa | ad at | 8.94,1 | 0.94,1 | 2.94,1 | 14.94 |
| | ,30.94 | | | | | | |
| TC: | 230 lb | Conc. Loa | ad at | 16.94, | 18.94, | 20.94, | 22.94 |
| 24.94 | ,26.94 | | | | | | |
| BC: | 567 lb | Conc. Loa | ad at | 7.03,3 | 32.97 | | |
| BC: | 135 lb | Conc. Loa | ad at | 8.94,1 | 0.94, | 12.94,1 | 14.94 |
| | ,30.94 | | | | | | |
| BC: | 127 lb | Conc. Loa | ad at | 16.94, | 18.94, | 20.94, | 22.94 |
| 24.94 | ,26.94 | | | | | | |
| | | | | | | | |



COA#0-278, Yoonhwak Kim, FL PE #86367 Flood 242026 icate of Product Approval #FL 1999

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

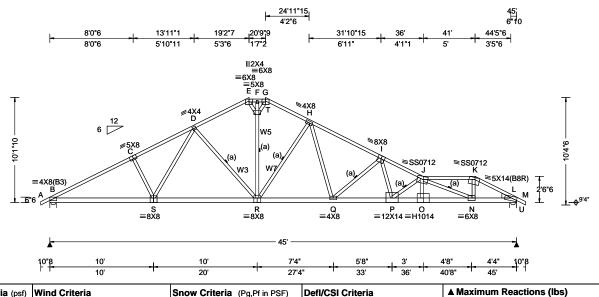
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SEQN: 837693 / COMN Ply: 1 Job Number: 402471 Cust: R 8975 JRef: 1YdO89750003 T54 / FROM: ARZ ,7F5 ,BRAA ,15;16;21;25 ,RB01 / BRADEN/A DrwNo: 267.25.0813.23794 Qty: 1 Truss Label: A9G KD / YK 09/24/2025



| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria |
|------------------------|--|------------------------------|---------------------------------|
| TCLL: 20.00 | Wind Std: ASCE 7-22 | Pg: NA Ct: NA CAT: NA | PP Deflection in loc L/defl L/# |
| TCDL: 15.00 | Speed: 160 mph | Pf: NA Ce: NA | VERT(LL): 0.352 P 999 268 |
| BCLL: 0.00 | Enclosure: Closed | Lu: NA Cs: NA | VERT(CL): 0.808 P 664 268 |
| BCDL: 10.00 | Risk Category: II | Snow Duration: NA | HORZ(LL): 0.090 E |
| Des Ld: 45.00 | EXP: C Kzt: NA | | HORZ(TL): 0.208 E |
| NCBCLL: 10.00 | Mean Height: 15.00 ft TCDL: 4.2 psf | Building Code: | Creep Factor: 2.0 |
| Soffit: 0.00 | BCDL: 5.0 psf | FBC 8th Ed. 2023 Res. | Max TC CSI: 0.694 |
| Load Duration: 1.25 | MWFRS Parallel Dist: 0 to h/2 | TPI Std: 2014 | Max BC CSI: 0.804 |
| Spacing: 24.0 " | C&C Dist a: 4.50 ft | Rep Fac: Varies by Ld Case | Max Web CSI: 0.951 |
| ' | Loc. from endwall: not in 13.00 ft | FT/RT:20(0)/10(0) | |
| | GCpi: 0.18 | Plate Type(s): | |
| | Wind Duration: 1.60 | WAVE, HS, 18SS | VIEW Ver: 23.02.01A.1204.18 |
| | | | |

Lumber

Top chord: 2x4 SP 2400f-2.0E; Bot chord: 2x6 SP 2400f-2.0E; Webs: 2x4 SP #3; W3,W5,W7 2x4 SP #2 N;

Rt Wedge: 2x6 SP #2 N;

Bracing

(a) 1X4 #3SRB or better continuous lateral restraint to be equally spaced. Attach with (2) 8d Box or Gun nails(0.113"x2.5",min.). Restraint material to be supplied and attached at both ends to a suitable support by erection contractor.

(a) or scab reinforcement may be used in lieu of CLR restraint. substitute (1) scab for (1) CLR and (2) scabs for (2) CLR'S where shown. Scab reinforcement to be same size, species, grade, and 80% length of web member. Attach with 0.128x3" gun nails @ 6" oc.

Special Loads

| • | | | | |
|-------------|------------|---------------|--------------|-------|
| (Lumber | Dur.Fac.=1 | .25 / Plate [| Dur.Fac.=1.2 | 25) |
| TC: From | | -0.88 to | | 45.88 |
| BC: From | 20 plf at | 0.00 to | 20 plf at | 40.06 |
| BC: From | 10 plf at | 40.06 to | 10 plf at | 40.97 |
| | 20 plf at | | 20 plf at | 45.00 |
| TC: 130 lb | Conc. Load | l at 40.97 | - | |
| TC: 137 lb | Conc. Load | l at 41.00 | | |
| BC: 1110 lb | Conc. Load | at 40.06 | | |
| | | | | |

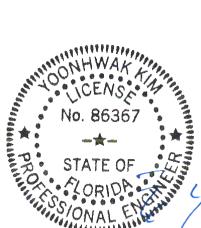
Wind

Wind loads and reactions based on MWFRS. Wind loading based on both gable and hip roof types.

BC: 184 lb Conc. Load at 40.97

Additional Notes

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



Non-Gravity Gravity Loc R+ /R /Rh /Rw /U /RL В 2322 /-/841 U 3562 /-/-/1177 /-Wind reactions based on MWFRS Brg Wid = 8.0 Min Reg = 1.9 (Truss) Brg Wid = 8.0 Min Req = 3.0 (Truss)

Bearings B & L are a rigid surface. Bearings B & L require a seat plate.

Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs)

Chords Tens.Comp.

| B-C | 1511 - 4149 | G-H | 1069 | - 3012 |
|-------|-------------|-------|------|--------|
| C - D | 1375 - 3909 | H - I | 1626 | - 4663 |
| D-E | 1079 - 3031 | I - J | 2454 | - 7189 |
| E-F | 1130 - 3339 | J - K | 1988 | - 6244 |
| F-G | 1140 - 3354 | K-L | 2179 | - 6712 |

Chords Tens. Comp.

Maximum Bot Chord Forces Per Ply (lbs)

| Chords | Tens.Comp. | Chords | Tens. Comp. | |
|--------|-------------|--------|-------------|--|
| B-S | 3592 - 1285 | P - O | 9411 - 3135 | |
| S - R | 3112 - 1112 | O - N | 9401 - 3137 | |
| R-Q | 3559 - 1249 | N - L | 5908 - 1908 | |
| Q - P | 5849 - 2011 | | | |

Maximum Web Forces Per Ply (lbs)

| vvebs | rens.Comp. | vvebs | rens. Comp. |
|-------|------------|-------|-------------|
| C-S | 245 - 438 | H-Q | 1778 - 518 |
| S - D | 654 - 139 | Q - I | 821 - 2382 |
| D - R | 291 - 758 | I-P | 2339 - 695 |
| E-T | 1284 - 384 | P-J | 1181 - 3704 |
| R-H | 593 - 1663 | J - N | 1263 - 3471 |
| T - R | 2139 - 660 | N - K | 2496 - 594 |
| T - G | 1318 - 406 | | |

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FROM: ARZ ,7F5 ,BRAA ,15;16;21;25 ,RB01 / BRADEN/A DrwNo: 267.25.0813.23873 Qty: 1 Truss Label: A10 KD / YK 09/24/2025 8'9"9 17'2"7 22'9"9 28'3' 5'7"2 5' 8'9"9 8'4"15 5'5"8 5'9' =6X6 D ≡5X8 E **≥4**X6 w3 5X8 W5 9'1"10 **≥8X10** =5X8 H W4 (a) 3,6,6 W2 -ф^{9'4"} <u>↓</u>__6"6 " 0 ≡4X4 P ≡5X8 N ≡6X8 M ≡4X6 K ∥2X4 =5X8(B2) =H0610 =5X8(B2) 45 8'8"4 8'2"11 6'0"6 5'7"4 5'5"8 5'1"12 5'10"4 10"8 8'8"4 16'10"15 22'11"5 28'6"8 34' 39'1"12 45' ▲ Maximum Reactions (lbs)

| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | |
|------------------------|---|------------------------------|---------------------------------|---|
| TCLL: 20.00 | Wind Std: ASCE 7-22 | Pg: NA Ct: NA CAT: NA | PP Deflection in loc L/defl L/# | ı |
| TCDL: 15.00 | Speed: 160 mph | Pf: NA Ce: NA | VERT(LL): 0.278 M 999 268 | |
| BCLL: 0.00 | Enclosure: Closed | Lu: NA Cs: NA | VERT(CL): 0.644 M 834 268 | |
| BCDL: 10.00 | Risk Category: II | Snow Duration: NA | HORZ(LL): 0.090 I | |
| Des Ld: 45.00 | EXP: C Kzt: NA Mean Height: 15.00 ft | | HORZ(TL): 0.210 l | |
| NCBCLL: 10.00 | TCDL: 4.2 psf | Building Code: | Creep Factor: 2.0 | |
| Soffit: 0.00 | BCDL: 5.0 psf | FBC 8th Ed. 2023 Res. | Max TC CSI: 0.569 | |
| Load Duration: 1.25 | MWFRS Parallel Dist: h to 2h | TPI Std: 2014 | Max BC CSI: 0.613 | |
| Spacing: 24.0 " | C&C Dist a: 4.50 ft | Rep Fac: Yes | Max Web CSI: 0.952 | |
| | Loc. from endwall: not in 13.00 ft | | | |
| | GCpi: 0.18 | Plate Type(s): | | 1 |
| | Wind Duration: 1.60 | WAVE, HS | VIEW Ver: 23.02.01A.1204.18 | |
| Lumber | • | | • | _ |

Job Number: 402471

SEQN: 837563 /

COMN

Ply: 1

Top chord: 2x4 SP 2400f-2.0E; Bot chord: 2x4 SP 2400f-2.0E; Webs: 2x4 SP #3; W2,W3,W4,W5 2x4 SP #2 N; Lt Wedge: 2x4 SP #3;Rt Wedge: 2x4 SP #3;

Bracing

(a) 1X4 #3SRB or better continuous lateral restraint to be equally spaced. Attach with (2) 8d Box or Gun nails(0.113"x2.5",min.). Restraint material to be supplied and attached at both ends to a suitable support by erection contractor.

(a) or scab reinforcement may be used in lieu of CLR restraint. substitute (1) scab for (1) CLR and (2) scabs for (2) CLR'S where shown. Scab reinforcement to be same size, species, grade, and 80% length of web member. Attach with 0.128x3" gun nails @ 6" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

Gravity Non-Gravity Loc R+ /R /Rh /Rw /U

В 2169 /-/1473 /365 /394 2169 /-/-/1460 /352 /-Wind reactions based on MWFRS

/RL

Cust: R 8975 JRef: 1YdO89750003 T21 /

Brg Wid = 8.0Min Reg = 1.8 (Truss)

Brg Wid = 8.0 Min Req = 1.8 (Truss) Bearings B & I are a rigid surface. Bearings B & I require a seat plate.

Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs)

Chords Tens.Comp. Chords Tens. Comp. B - C 1895 - 3810 F-G 2365 - 4230 C-D 1742 - 3032 G-H 3149 - 5500 D-E 1772 - 2700 H - I 2199 - 3901 E-F 1878 - 3118

Maximum Bot Chord Forces Per Ply (lbs)

| Chords | Tens.Comp. | Chords | Tens. Co | omp. |
|----------------------------------|--|-------------------------|----------------------------|------|
| B - P P - O O - N N - M | 3274 - 1570 3272 - 1572 2574 - 1158 3647 - 1765 | M - L L - K K - I | 5500 - 3388 - 3388 - | 1829 |
| | | | | |

Maximum Web Forces Per Ply (lbs)

| webs | rens.Comp. | webs | rens. Comp. | | |
|-------|------------|-------|-------------|--|--|
| C-0 | 469 - 781 | F-M | 1163 - 595 | | |
| O - D | 602 - 169 | M - G | 1299 - 2092 | | |
| D - N | 393 - 257 | G-L | 757 - 1216 | | |
| E - N | 965 - 490 | L-H | 2500 - 1336 | | |
| N - F | 882 - 1403 | | | | |

COA#0-278, Yoonhwak Kim, FL PE #86367 Florida (2026) icate of Product Approval #FL 1999

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

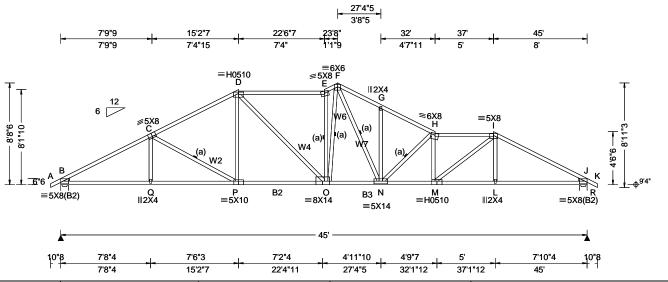
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For more information see these web sites: Alpine: alpineitw.com: TPI: binst.org: SBCA: sbcacomponents.com: ICC: iccsafe.org: AWC: awc.org



SEQN: 837583 / COMN Ply: 1 Job Number: 402471 Cust: R 8975 JRef: 1YdO89750003 T24 / FROM: ARZ Qty: 1 ,7F5 ,BRAA ,15;16;21;25 ,RB01 / BRADEN/A DrwNo: 267.25.0813.23512 Truss Label: A11 KD / YK 09/24/2025



| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | • |
|---|--|--|---|----------------|
| TCLL: 20.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 0.00 | Wind Std: ASCE 7-22 Speed: 160 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 4.50 ft Loc. from endwall: not in 13.00 ft GCpi: 0.18 Wind Duration: 1.60 | Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS | PP Deflection in loc L/defl L/# VERT(LL): 0.276 E 999 268 VERT(CL): 0.641 E 839 268 HORZ(LL): 0.094 J HORZ(TL): 0.218 J Creep Factor: 2.0 Max TC CSI: 0.563 Max BC CSI: 0.947 Max Web CSI: 0.816 VIEW Ver: 23.02.01A.1204.18 | L BRVBRBB MC B |
| Lumber | | | | |

Top chord: 2x4 SP 2400f-2.0E; Bot chord: 2x4 SP 2400f-2.0E; B2 2x4 SP #2 N; B3 2x4 SP #1; Webs: 2x4 SP #3; W2,W4,W6,W7 2x4 SP #2 N; Lt Wedge: 2x4 SP #3;Rt Wedge: 2x4 SP #3;

(a) 1X4 #3SRB or better continuous lateral restraint to be equally spaced. Attach with (2) 8d Box or Gun nails(0.113"x2.5",min.). Restraint material to be supplied and attached at both ends to a suitable support by erection contractor.

(a) or scab reinforcement may be used in lieu of CLR restraint. substitute (1) scab for (1) CLR and (2) scabs for (2) CLR'S where shown. Scab reinforcement to be same size, species, grade, and 80% length of web member. Attach with 0.128x3" gun nails @ 6" oc.

Wind loads based on MWFRS with additional C&C

Wind loading based on both gable and hip roof types.

Additional Notes

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

▲ Maximum Reactions (lbs)

| # | Gravity | | | Non-Gravity | | | |
|---|---------------------------------------|---|---|---|--|----------------------------------|------------------|
| 3 | Loc | R+ | / R- | / Rh | /Rw | / U | / RL |
| 3 | B : Wind B R Bear Bear | 2165 2168 d read Brg W Brg W rings E | /- /- tions b /id = 8 /id = 8 3 & J a 3 & J r | /- /- ased on M 0 Min F 0 Min F re a rigid equire a s | /1468 /1461 //WFRS Req = 1.8 Req = 1.8 surface. eat plate. | /376 /352 (Truss (Truss | /374 /- s) |
| | | | | ed have fo | | | |
| | | | | Chord Fo | | | • |
| - | Cho | rds T | ens.Co | omp. (| Chords | Tens. | Comp. |
| | | | | | | | |

| Chords | Tens.Comp. | Chords | Tens. | Comp. |
|--------|-------------|--------|-------|--------|
| B-C | 2021 - 3821 | F-G | 2555 | - 3884 |
| C - D | 1929 - 3206 | G-H | 2415 | - 3934 |
| D-E | 2057 - 3046 | H-I | 2881 | - 4643 |

2257 - 3845

Maximum Bot Chord Forces Per Ply (lbs)

2355 - 3433

E-F

| Cnoras | rens.Comp. | Choras | i ens. | Comp. |
|--------|-------------|--------|--------|--------|
| B - Q | 3290 - 1678 | N - M | 4643 | - 2585 |
| Q - P | 3288 - 1680 | M - L | 3314 | - 1856 |
| P - O | 2757 - 1358 | L-J | 3316 | - 1853 |
| O - N | 2810 - 1359 | | | |

Maximum Web Forces Per Ply (lbs)

| vvebs | rens.comp. | webs | rens. Comp. |
|-------|-------------|-------|-------------|
| C - P | 366 - 594 | F-N | 1537 - 1050 |
| P - D | 553 - 120 | N - H | 1075 - 1628 |
| D - O | 466 - 341 | H - M | 666 - 1001 |
| E - O | 1474 - 1885 | M - I | 1729 - 983 |
| 0 - F | 1723 - 1206 | | |



COA#0-278, Yoonhwak Kim, FL PE #86367 Fl09/24/2026 Ficate of Product Approval #FL 1999

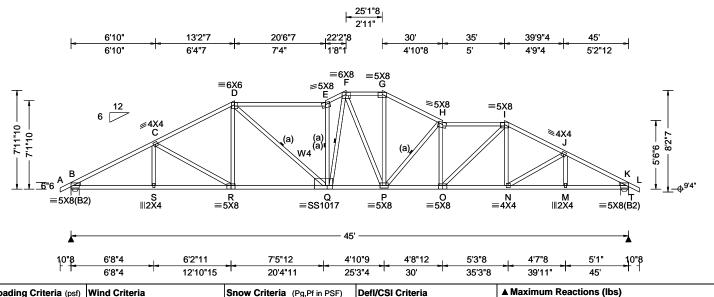
WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

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Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

SEQN: 837590 / COMN Ply: 1 Job Number: 402471 Cust: R 8975 JRef: 1YdO89750003 T23 / FROM: ARZ ,7F5 ,BRAA ,15;16;21;25 ,RB01 / BRADEN/A DrwNo: 267.25.0813.23372 Qty: 1 Truss Label: A12 KD / YK 09/24/2025



| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | ▲ Maximum Reactions (lbs) | 1 |
|--|---|--|---|---|------------------------------------|
| TCLL: 20.00 TCDL: 15.00 | Wind Std: ASCE 7-22 Speed: 160 mph | Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA | PP Deflection in loc L/defl L/# VERT(LL): 0.259 E 999 268 | | Non-Gravity / Rw / U / RL |
| BCLL: 0.00 BCDL: 10.00 | Enclosure: Closed Risk Category: II | Lu: NA Cs: NA Snow Duration: NA | VERT(CL): 0.600 E 896 268 HORZ(LL): 0.085 K - | , | /1462 /391 /343 /1460 /419 /- |
| Des Ld: 45.00 | EXP: C Kzt: NA Mean Height: 15.00 ft | Building Code: | HORZ(TL): 0.198 K Creep Factor: 2.0 | Wind reactions based on MW | |
| NCBCLL: 10.00 Soffit: 0.00 | TCDL: 4.2 psf BCDL: 5.0 psf | FBC 8th Ed. 2023 Res. | Max TC CSI: 0.525 | T Brg Wid = 8.0 Min Red Bearings B & K are a rigid sur | = 1.8 (Truss) |
| Load Duration: 1.25 Spacing: 24.0 " | MWFRS Parallel Dist: h to 2h C&C Dist a: 4.50 ft | TPI Std: 2014 Rep Fac: Yes | Max BC CSI: 0.610 Max Web CSI: 0.862 | Bearings B & K require a seat | t plate. |
| | Loc. from endwall: not in 13.00 ft GCpi: 0.18 | FT/RT:20(0)/10(0) Plate Type(s): | | Maximum Top Chord Force Chords Tens.Comp. Cho | s Per Ply (lbs) ords Tens. Comp |
| Lumber | Wind Duration: 1.60 | WAVE, 18SS | VIEW Ver: 23.02.01A.1204.18 | B - C 2178 - 3839 G - | H 2353 - 353 |

Lumber

Top chord: 2x4 SP 2400f-2.0E; Bot chord: 2x4 SP 2400f-2.0E; Webs: 2x4 SP #3; W4 2x4 SP #2 N; Lt Wedge: 2x4 SP #3;Rt Wedge: 2x4 SP #3;

Bracing

(a) 1X4 #3SRB or better continuous lateral restraint to be equally spaced. Attach with (2) 8d Box or Gun nails(0.113"x2.5",min.). Restraint material to be supplied and attached at both ends to a suitable support by erection contractor.

(a) or scab reinforcement may be used in lieu of CLR restraint. substitute (1) scab for (1) CLR and (2) scabs for (2) CLR'S where shown. Scab reinforcement to be same size, species, grade, and 80% length of web member. Attach with 0.128x3" gun nails @ 6" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below. B - C 2178 - 3839 G-H 2353 - 3530 C-D 2122 - 3378 H - I 2708 - 4054 D-E 2390 - 3478 I-J 2312 E-F 2702 - 3887 J - K 2283

2177 - 3064

F-G

Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - S 3313 - 1864 P - O

- 3609

- 3820

4054 - 2357 S - R 3312 - 1867 O - N 3174 - 1816 R - Q 2933 - 1635 N - M 3305 - 1922 3094 - 1677 3304 - 1918 Q - P M - K

Maximum Web Forces Per Ply (lbs) Webs Tens Comp

| W CD3 | rens.comp. | W CD3 | rens. comp. | |
|-------|-------------|-------|-------------|--|
| C-R | 264 - 451 | G-P | 1232 - 768 | |
| R - D | 480 -85 | P - H | 1038 - 1430 | |
| D - Q | 734 - 564 | H - O | 655 - 833 | |
| E - Q | 1601 - 2070 | 0-1 | 1264 - 813 | |
| Q-F | 1722 - 1230 | | | |

COA#0-278, Yoonhwak Kim, FL PE #86367 Floricate of Product Approval #FL 1999

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SEQN: 837598 / COMN Ply: 1 Job Number: 402471 Cust: R 8975 JRef: 1YdO89750003 T22 / FROM: ARZ ,7F5 ,BRAA ,15;16;21;25 ,RB01 / BRADEN/A DrwNo: 267.25.0813.23732 Qty: 1 Truss Label: A13 KD / YK 09/24/2025 18'6"7 38'9"4 45' 5'10' 11'2"7 20'2"8 27'1"8 33 1'8"1 10"8 5'10" 5'4"7 7'4" 6'11" 5'9"4 6'2"12 ≡6X8 ≅6X6 =6X8 **T5** G _H $\equiv 5X8$ =6X6 ≢5X8 E ∕ **≥4X4** - 6'11"10 -6'1"10 — (a) (a) .(a) W4^(a) W7 6"6 R ∥2X4 Q ≡5X8 B2 P ≡8X8 **B3** 0 ≡5X14 =5X8 =5X8(B2) M ∥2X4 =5X8(B2) 45' 5'8"4 5'2"11 7'9"4 9'3"13 5'3"8 5'7"8 6'1' 5'8"4 10'10"15 18'8"3 28' 33'3"8 38'11' 45

| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | Τ |
|------------------------|--|------------------------------|---------------------------------|---|
| TCLL: 20.00 | Wind Std: ASCE 7-22 | Pg: NA Ct: NA CAT: NA | PP Deflection in loc L/defl L/# | |
| TCDL: 15.00 | Speed: 160 mph | Pf: NA Ce: NA | VERT(LL): 0.277 E 999 268 | |
| BCLL: 0.00 | Enclosure: Closed | Lu: NA Cs: NA | VERT(CL): 0.642 E 837 268 | |
| BCDL: 10.00 | Risk Category: II | Snow Duration: NA | HORZ(LL): 0.095 K | |
| Des Ld: 45.00 | EXP: C Kzt: NA | | HORZ(TL): 0.220 K | |
| NCBCLL: 10.00 | Mean Height: 15.00 ft TCDL: 4.2 psf | Building Code: | Creep Factor: 2.0 | |
| Soffit: 0.00 | BCDL: 5.0 psf | FBC 8th Ed. 2023 Res. | Max TC CSI: 0.521 | |
| Load Duration: 1.25 | MWFRS Parallel Dist: h/2 to h | TPI Std: 2014 | Max BC CSI: 0.808 | |
| Spacing: 24.0 " | C&C Dist a: 4.50 ft | Rep Fac: Yes | Max Web CSI: 0.992 | |
| | Loc. from endwall: not in 13.00 ft | FT/RT:20(0)/10(0) | | |
| | GCpi: 0.18 | Plate Type(s): | | 4 |
| | Wind Duration: 1.60 | WAVE | VIEW Ver: 23.02.01A.1204.18 | |
| Lumber | | | | - |

В Wind reactions based on MWFRS

Brg Wid = 8.0Min Reg = 1.8 (Truss) Brg Wid = 8.0 Min Req = 1.8 (Truss) Bearings B & K are a rigid surface. Bearings B & K require a seat plate. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

/Rh

/-

Non-Gravity

/1452 /803 /301

/RL

/-

/Rw /U

/1453 /802

▲ Maximum Reactions (lbs) Gravity

/R

Loc R+

2169 /-

2169 /-

| B - C | 2348 - 3831 | G-H | 2706 | - 3871 |
|-------|-------------|-------|------|--------|
| C - D | 2360 - 3532 | H - I | 2551 | - 3619 |
| D - E | 2815 - 4002 | I - J | 2292 | - 3456 |
| E-F | 3126 - 4421 | J-K | 2316 | - 3845 |
| F-G | 2504 - 3478 | | | |

Bracing

(a) 1X4 #3SRB or better continuous lateral restraint to be equally spaced. Attach with (2) 8d Box or Gun nails(0.113"x2.5",min.). Restraint material to be supplied and attached at both ends to a suitable support by erection contractor.

Top chord: 2x4 SP 2400f-2.0E; T5 2x6 SP SS; Bot chord: 2x4 SP 2400f-2.0E; B2,B3 2x4 SP #1; Webs: 2x4 SP #3; W4,W7 2x4 SP #2 N; Lt Wedge: 2x4 SP #3; Rt Wedge: 2x4 SP #3;

(a) or scab reinforcement may be used in lieu of CLR restraint. substitute (1) scab for (1) CLR and (2) scabs for (2) CLR'S where shown. Scab reinforcement to be same size, species, grade, and 80% length of web member. Attach with 0.128x3" gun nails @ 6" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

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| Chords | Tens.Comp. | Chords | Tens. Comp. |
|----------------|----------------------------|----------------|----------------------------|
| B-R | 3310 - 2024 | O - N N - M | 3014 - 1753 |
| R - Q Q - P | 3310 - 2027 3100 - 1900 | M - M M - K | 3323 - 1942 3323 - 1939 |
| P - O | 3547 - 2174 | | |

Maximum Web Forces Per Ply (lbs)

| Webs | Tens.Comp. | Webs | Tens. Comp. | |
|-------|-------------|-------|-------------|--|
| D-P | 1139 - 821 | O - H | 1134 - 1588 | |
| E-P | 1712 - 2243 | O - I | 958 - 713 | |
| P - F | 1767 - 1194 | N - J | 233 - 393 | |
| G - O | 1069 - 589 | | | |



COA#0-278, Yoonhwak Kim, FL PE #86367 Fload 2026 icate of Product Approval #FL 1999

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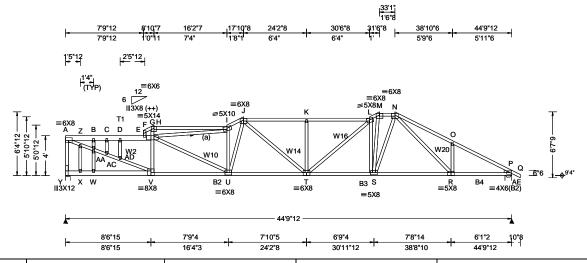
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SEQN: 837778 SPEC Ply: 2 Job Number: 402471 Cust: R 8975 JRef: 1YdO89750003 T12 Qty: 1 FROM: ARZ ,7F5 ,BRAA ,15;16;21;25 ,RB01 / BRADEN/A DrwNo: 267.25.0919.40050 Truss Label: A14G KD / YK 09/24/2025

2 Complete Trusses Required



| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,F | Pf in PSF) | Defl/CSI Crite | eria | | |
|------------------------|---|----------------------|------------|----------------|----------|---------|--------|
| TCLL: 20.00 | Wind Std: ASCE 7-22 | Pg: NA Ct: NA | CAT: NA | PP Deflection | in lo | : L/def | fl L/# |
| TCDL: 15.00 | Speed: 160 mph | Pf: NA | Ce: NA | VERT(LL): (|).259 I | 999 | 268 |
| BCLL: 0.00 | Enclosure: Closed | Lu: NA Cs: NA | | VERT(CL): (|).595 I | 902 | 268 |
| 10.00 | Risk Category: II | Snow Duration: NA | | HORZ(LL): (| 0.060 P | - | - |
| Dec 1 d: 45 00 | EXP: C Kzt: NA Mean Height: 15.00 ft | | | HORZ(TL): (|).137 P | - | - |
| MODOLL, A AA | TCDL: 4.2 psf | Building Code: | | Creep Factor: | 2.0 | | |
| 0 - 40:4. | BCDL: 5.0 psf | FBC 8th Ed. 2023 R | es. | Max TC CSI: | 0.685 | i | |
| | MWFRS Parallel Dist: 0 to h/2 | TPI Std: 2014 | | Max BC CSI: | | | |
| Spacing: 24.0 " | C&C Dist a: 4.48 ft | Rep Fac: Varies by I | Ld Case | Max Web CS | l: 0.884 | | |
| | Loc. from endwall: not in 6.50 ft | FT/RT:20(0)/10(0) | | | | | |
| | GCpi: 0.18 | Plate Type(s): | | | | | |
| | Wind Duration: 1.60 | WAVE | | VIEW Ver: 23 | .02.01A | 1204. | 18 |

Lumber

Top chord: 2x4 SP #2 N; T1 2x6 SP 2400f-2.0E; Bot chord: 2x6 SP 2400f-2.0E; B2 2x4 SP 2400f-2.0E; B3 2x4 SP #2 N;

B4 2x4 SP #1;

Webs: 2x4 SP #3; W2 2x4 SP 2400f-2.0E; W10,W14, W16,W20 2x4 SP #2 N;

Rt Wedge: 2x4 SP #3;

Bracing

(a) 1X4 #3SRB or better continuous lateral restraint to be equally spaced. Attach with (2) 8d Box or Gun nails(0.113"x2.5",min.). Restraint material to be supplied and attached at both ends to a suitable support by erection contractor.

Nailnote

Nail Schedule:0.128"x3", min. nails Top Chord: 1 Row @12.00" o.c. Bot Chord: 1 Row @12.00" o.c. Webs : 1 Row @ 4" o.c.

BC: 1164 lb Conc. Load at 7.68

Use equal spacing between rows and stagger nails in each row to avoid splitting.

| Special Load | 1S | | | |
|--------------|-------------|--------------|--------------|-------|
| (Lumber | Dur.Fac.=1. | 25 / Plate D | Dur.Fac.=1.2 | 25) |
| TC: From | 37 plf at | 0.00 to | 37 plf at | 8.58 |
| TC: From | 74 plf at | 8.58 to | 74 plf at | 45.69 |
| BC: From | 10 plf at | 0.00 to | 10 plf at | 8.58 |
| BC: From | 20 plf at | 8.58 to | 20 plf at | 44.81 |
| TC: 354 lb | Conc. Load | at 0.88 | | |
| TC: 327 lb | Conc. Load | at 2.88 | | |
| TC: 654 lb | Conc. Load | at 4.98 | | |
| TC: 47 lb | Conc. Load | at 7.08 | | |
| | | | | |

Plating Notes

All plates are 2X4 except as noted.

(++) - This plate works for both joints covered.

Wind loads and reactions based on MWFRS.

Left end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Laterally brace top chord below filler and borrown chord above filler at 24" o.c., including a rate leaf brace at chord ends (If no rigid diap) radim exists at that point?

▲ Maximum Reactions (lbs) Gravity

Non-Gravity Loc R+ /R /Rh /Rw /U / RL 3966 /-/1606 /-AE 2434 /-/-/909 /-Wind reactions based on MWFRS

Brg Wid = 5.8 Min Reg = 1.6 (Truss)

AE Brg Wid = 8.0 Min Req = 1.5 (Truss) Bearings Y & P are a rigid surface.

Bearings Y & P require a seat plate.

Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

| A - B B - C C - D D - F | 1245 - 3294 1245 - 3294 1245 - 3294 1246 - 3299 | J - K K - L L - M M - N | 988 857 726 | - 2662 - 2662 - 2315 - 1979 |
|----------------------------------|--|----------------------------------|-------------------|--------------------------------------|
| F-G | 1222 - 3233 | N - O | 802 | - 2162 |
| I - J | 1290 - 3458 | O - P | 818 | - 2203 |

Maximum Bot Chord Forces Per Plv (lbs)

| Chords | Tens.Comp. | Chords | Tens. (| Comp. |
|--------|-------------|--------|---------|-------|
| V - U | 3304 - 1251 | S - R | 1763 | - 654 |
| U - T | 2693 - 1005 | R-P | 1912 | - 703 |
| T-S | 2191 - 808 | | | |

Maximum Web Forces Per Ply (lbs) Webs

| ****** | rens.comp. | ****** | rens. comp. |
|--------|-------------|--------|-------------|
| A - Y | 679 - 1680 | G-I | 1107 - 2987 |
| A - Z | 3550 - 1341 | G - U | 513 - 194 |
| Z -AA | 3576 - 1352 | I - U | 526 - 1314 |
| AA-AC | 3557 - 1343 | U - J | 1492 - 539 |
| AC-AD | 3562 - 1345 | T - L | 623 - 237 |
| AD- V | 3519 - 1328 | L-S | 601 - 1522 |
| y - G | 243 - 613 | S - M | 990 - 374 |
| G-H | 206 - 408 | S - N | 657 - 217 |

COA#0-278, Yoonhwak Kim, FL PE #86367 Fl09/24/2026 Ficate of Product Approval #FL 1999

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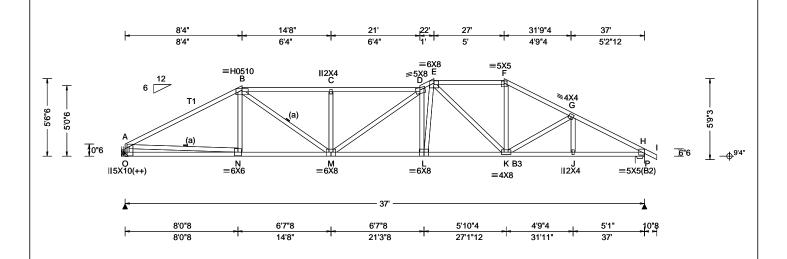
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SEQN: 837666 / SPEC Ply: 1 Job Number: 402471 Cust: R 8975 JRef: 1YdO89750003 T1 / FROM: ARZ Qty: 1 ,7F5 ,BRAA ,15;16;21;25 ,RB01 / BRADEN/A DrwNo: 267.25.0813.23810 Truss Label: A15 KD / YK 09/24/2025



| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | |
|------------------------|---|------------------------------|---------------------------------|---|
| TCLL: 20.00 | Wind Std: ASCE 7-22 | Pg: NA Ct: NA CAT: NA | PP Deflection in loc L/defl L/# | |
| TCDL: 15.00 | Speed: 160 mph | Pf: NA Ce: NA | VERT(LL): 0.237 D 999 240 | |
| BCLL: 0.00 | Enclosure: Closed | Lu: NA Cs: NA | VERT(CL): 0.550 D 804 240 | |
| BCDL: 10.00 | Risk Category: II | Snow Duration: NA | HORZ(LL): 0.068 H | |
| Des Ld: 45.00 | EXP: C Kzt: NA Mean Height: 15.00 ft | | HORZ(TL): 0.157 H | |
| NCBCLL: 10.00 | TCDL: 4.2 psf | Building Code: | Creep Factor: 2.0 | |
| Soffit: 0.00 | BCDL: 5.0 psf | FBC 8th Ed. 2023 Res. | Max TC CSI: 0.940 | |
| Load Duration: 1.25 | MWFRS Parallel Dist: h/2 to h | TPI Std: 2014 | Max BC CSI: 0.947 | |
| Spacing: 24.0 " | C&C Dist a: 3.70 ft | Rep Fac: Yes | Max Web CSI: 0.886 | |
| | Loc. from endwall: not in 9.00 ft | FT/RT:20(0)/10(0) | | |
| | GCpi: 0.18 | Plate Type(s): | | 4 |
| Lumban | Wind Duration: 1.60 | WAVE, HS | VIEW Ver: 23.02.01A.1204.18 | |

Lumber

Top chord: 2x4 SP #2 N; T1 2x4 SP 2400f-2.0E; Bot chord: 2x4 SP #2 N; B3 2x4 SP #1; Webs: 2x4 SP #3;

Bracing

(a) 1X4 #3SRB or better continuous lateral restraint to be equally spaced. Attach with (2) 8d Box or Gun nails(0.113"x2.5",min.). Restraint material to be supplied and attached at both ends to a suitable support by erection contractor.

(a) or scab reinforcement may be used in lieu of CLR restraint. substitute (1) scab for (1) CLR and (2) scabs for (2) CLR's where shown. Scab reinforcement to be same size, species, grade, and 80% length of web member. Attach with 0.128x3" gun nails @ 6" oc.

Plating Notes

(++) - This plate works for both joints covered.

Hangers / Ties

(J) Hanger Support Required, by others

Wind

Wind loads based on MWFRS with additional C&C member design.

Left end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Loc R+ /Rh /Rw /U 1726 /-/1097 /637 1800 /-/1195 /668 /-Wind reactions based on MWFRS Brg Wid = -Min Reg = Brg Wid = 8.0 Min Req = 2.1 (Truss) Bearing H is a rigid surface. Bearing H requires a seat plate. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs)

Non-Gravity

▲ Maximum Reactions (lbs) Gravity

Chords Tens.Comp. Chords Tens. Comp. A - B 1893 - 2888 E-F 1840 - 2446

B - C 2509 - 3378 F-G 1963 - 2816 C-D 2509 - 3379 G-H 1951 - 3090 D-E 2710 - 3653

Maximum Bot Chord Forces Per Ply (lbs)

| Chords | Tens.Comp. | Chords | Tens. Comp. |
|--------|-------------|--------|-------------|
| O - N | 409 - 263 | L-K | 2996 - 1910 |
| N - M | 2480 - 1565 | K-J | 2664 - 1630 |
| M - L | 3376 - 2192 | J - H | 2663 - 1626 |

Maximum Web Forces Per Ply (lbs)

| os Tens. Comp. |
|----------------|
| _ 1465 - 1825 |
| 1959 - 1403 |
| 635 - 779 |
| (818 - 482 |
| |



COA#0-278, Yoonhwak Kim, FL PE #86367 Fl09.024/2026 icate of Product Approval #FL 1999

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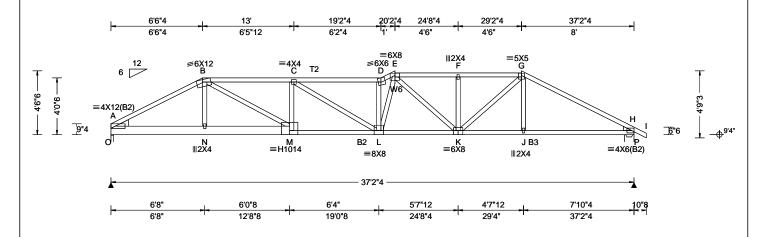
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SEQN: 837766 COMN Ply: 2 Job Number: 402471 Cust: R 8975 JRef: 1YdO89750003 T35 Qty: 1 FROM: ARZ ,7F5 ,BRAA ,15;16;21;25 ,RB01 / BRADEN/A DrwNo: 267.25.0920.06300 Truss Label: A16G KD / YK 09/24/2025

2 Complete Trusses Required



| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria |
|------------------------|---|------------------------------|---------------------------------|
| TCLL: 20.00 | Wind Std: ASCE 7-22 | Pg: NA Ct: NA CAT: NA | PP Deflection in loc L/defl L/# |
| TCDL: 15.00 | Speed: 160 mph | Pf: NA Ce: NA | VERT(LL): 0.340 D 999 240 |
| BCLL: 0.00 | Enclosure: Closed | Lu: NA Cs: NA | VERT(CL): 0.775 D 573 240 |
| BCDL: 10.00 | Risk Category: II | Snow Duration: NA | HORZ(LL): 0.074 H |
| Des Ld: 45.00 | EXP: C Kzt: NA Mean Height: 15.00 ft | | HORZ(TL): 0.169 H |
| NCBCLL: 0.00 | TCDL: 4.2 psf | Building Code: | Creep Factor: 2.0 |
| Soffit: 0.00 | BCDL: 5.0 psf | FBC 8th Ed. 2023 Res. | Max TC CSI: 0.954 |
| Load Duration: 1.25 | MWFRS Parallel Dist: 0 to h/2 | TPI Std: 2014 | Max BC CSI: 0.858 |
| Spacing: 24.0 " | C&C Dist a: 3.72 ft | Rep Fac: No | Max Web CSI: 0.927 |
| | Loc. from endwall: not in 4.50 ft | FT/RT:20(0)/10(0) | |
| | GCpi: 0.18 | Plate Type(s): | |
| | Wind Duration: 1.60 | WAVE, HS | VIEW Ver: 23.02.01A.1204.18 |

Lumber

Top chord: 2x4 SP #2 N; T2 2x4 SP 2400f-2.0E; Bot chord: 2x8 SP 2400f-2.0E; B2 2x4 SP 2400f-2.0E; B3 2x4 SP #1; Webs: 2x4 SP #3; W6 2x4 SP #2 N; Lt Wedge: 2x6 SP #2 N;Rt Wedge: 2x4 SP #3;

Nailnote

Nail Schedule:0.128"x3", min. nails Top Chord: 1 Row @12.00" o.c. Bot Chord: 1 Row @12.00" o.c. Webs : 1 Row @ 4" o.c. Use equal spacing between rows and stagger nails in each row to avoid splitting.

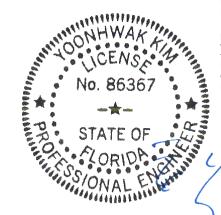
Special Loads

| Opeciai Load | 13 | | | |
|--------------|-------------|---------------|--------------|-------|
| (Lumber | Dur.Fac.=1. | .25 / Plate [| Our.Fac.=1.2 | 25) |
| TC: From | 74 plf at | 0.00 to | 74 plf at | 6.52 |
| TC: From | | 6.52 to | | 12.12 |
| TC: From | 74 plf at | 12.12 to | 74 plf at | 38.06 |
| BC: From | | 0.00 to | 20 plf at | 6.55 |
| BC: From | 10 plf at | 6.55 to | 10 plf at | 12.71 |
| BC: From | | | 20 plf at | 37.19 |
| TC: 522 lb | Conc. Load | at 6.55 | | |
| TC: 237 lb | Conc. Load | at 8.58,10 | .58 | |
| BC: 567 lb | Conc. Load | at 6.55 | | |
| BC: 135 lb | Conc. Load | at 8.58,10 | .58 | |
| BC: 2947 lb | Conc. Load | at 12.12 | | |
| | | | | |

Wind loads and reactions based on MWFRS. Wind loading based on both gable and hip roof types.

Blocking

Apply additional nailing over the following bearings with fasteners at 9" oc perpendicular to grain and 4" oc parallel to grain. In lieu of additional nailing, apply blocking reinforcement to prevent buckling of members over the bearings: Bearing 1 located at 0.0' (blocking >= 3.50" if used)



| ▲ Maximum Reactions (lbs) | | | | | | |
|---------------------------|---------|------------|-----------|------------|----------|--------|
| | G | ravity | | No | n-Grav | ity |
| Loc | R+ | / R- | /Rh | / Rw | / U | / RL |
| 0 | 4978 | /- | /- | /- | /1953 | /- |
| Р | 3075 | /- | /- | /- | /1186 | /- |
| Win | d reac | tions bas | sed on | MWFRS | | |
| 0 | Brg V | Vid = 2.2 | Min | Req = 2.1 | (Truss |) |
| Р | Brg V | Vid = 8.0 | Min | Req = 1.8 | (Truss |) |
| Bea | rings (| O & H are | e a rigio | d surface. | • | - |
| Bea | rings (| O & H re | quire a | seat plate | | |
| Men | nbers | not listed | l have f | orces less | than 3 | 75# |
| Max | imum | Top Ch | ord Fo | rces Per | Ply (lbs | s) |
| Cho | rds T | ens.Con | np. | Chords | Tens. | Ćomp. |
| A - E | 3 | 1856 - 46 | 691 | E-F | 1420 | - 3679 |
| B - 0 | 2 | 2460 - 62 | 244 | F-G | 1420 | - 3679 |

Maximum Bot Chord Forces Per Ply (lbs)

2013 - 5196

2207 - 5697

C-D

D-E

| Chords | Tens.Comp. | Chords | Tens. Comp | |
|--------|-------------|--------|-------------|---|
| A - N | 4136 - 1631 | L-K | 4505 - 1750 |) |
| N - M | 4151 - 1630 | K-J | 2496 - 958 | 3 |
| M - L | 6222 - 2456 | J - H | 2496 - 956 | 3 |

- 2863

1109

Maximum Web Forces Per Ply (lbs)

| Webs | Tens.Comp. | Webs | Tens. Comp. |
|-------------------------|--|-------------------------|--------------------------------------|
| B - M C - L L - D | 2433 - 964 521 - 1206 867 - 2218 | L - E E - K K - G | 2926 -1119 450 -1128 1610 -629 |
| | | | |

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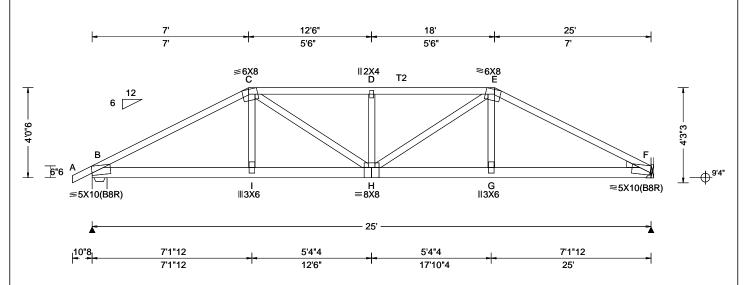
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155 Harlem Ave North Building, 4th Floor Glenview, IL 60025

SEQN: 837762 HIPS Ply: 1 Job Number: 402471 Cust: R 8975 JRef: 1YdO89750003 T27 FROM: ARZ Qty: 1 ,7F5 ,BRAA ,15;16;21;25 ,RB01 / BRADEN/A DrwNo: 267.25.0920.53987 Truss Label: B1G KD / YK 09/24/2025



| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | DefI/CSI Criteria |
|------------------------|--|------------------------------|---------------------------------|
| TCLL: 20.00 | Wind Std: ASCE 7-22 | Pg: NA Ct: NA CAT: NA | PP Deflection in loc L/defl L/# |
| TCDL: 15.00 | Speed: 160 mph | Pf: NA Ce: NA | VERT(LL): 0.178 D 999 240 |
| BCLL: 0.00 | Enclosure: Closed | Lu: NA Cs: NA | VERT(CL): 0.404 D 735 240 |
| BCDL: 10.00 | Risk Category: II | Snow Duration: NA | HORZ(LL): 0.046 F |
| Des Ld: 45.00 | EXP: C Kzt: NA | | HORZ(TL): 0.103 F |
| NCBCLL: 10.00 | Mean Height: 15.00 ft TCDL: 4.2 psf | Building Code: | Creep Factor: 2.0 |
| Soffit: 0.00 | BCDL: 5.0 psf | FBC 8th Ed. 2023 Res. | Max TC CSI: 0.936 |
| Load Duration: 1.25 | MWFRS Parallel Dist: 0 to h/2 | TPI Std: 2014 | Max BC CSI: 0.685 |
| Spacing: 24.0 " | C&C Dist a: 3.00 ft | Rep Fac: Varies by Ld Case | Max Web CSI: 0.526 |
| ' | Loc. from endwall: not in 4.50 ft | FT/RT:20(0)/10(0) | |
| | GCpi: 0.18 | Plate Type(s): | |
| | Wind Duration: 1.60 | WAVE | VIEW Ver: 23.02.01A.1204.18 |
| Laurekan | • | | • |

Lumber

Top chord: 2x4 SP #1; T2 2x4 SP 2400f-2.0E; Bot chord: 2x6 SP SS;

Webs: 2x4 SP #3; Rt Wedge: 2x4 SP #3;

Special Loads

----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25) 74 plf at 37 plf at TC: From -0.88 to 7.00 74 plf at 37 plf at TC: From 7.00 to 18.00 TC: From 74 plf at 18.00 to 74 plf at 25.00 BC: From 20 plf at 0.00 to 20 plf at 7.03 From 10 plf at 7.03 to 10 plf at 17.97 to 20 plf at 17.97 to 20 plf at 25 522 lb Conc. Load at 7.03,17.97 237 lb Conc. Load at 7.94, 9.94,11.94,13.94 BC: From BC: From 17 97 25.00 TC: 15.94 BC: 567 lb Conc. Load at 7.03,17.97 BC: 135 lb Conc. Load at 7.04 0.04 135 lb Conc. Load at 7.94, 9.94,11.94,13.94 15.94

Hangers / Ties

(J) Hanger Support Required, by others

Wind

Wind loads and reactions based on MWFRS. Wind loading based on both gable and hip roof types.

▲ Maximum Reactions (lbs)

C-D

| | G | ravity | | No | on-Grav | rity |
|-------|--------|------------|----------|------------|----------|--------|
| Loc | R+ | / R- | /Rh | / Rw | / U | / RL |
| В | 3040 | /- | /- | /- | /1239 | /- |
| F | 2883 | /- | /- | /- | /1161 | /- |
| Win | d read | tions bas | sed on I | MWFRS | | |
| В | Brg W | /id = 8.0 | Min | Req = 3.6 | (Truss | i) |
| F | Brg W | /id = - | Min | Req = - | | |
| Bea | ring B | is a rigid | surfac | e. | | |
| Bea | ring B | requires | a seat | plate. | | |
| Mer | nbers | not listed | l have f | orces less | s than 3 | 75# |
| Max | timum | Top Ch | ord Fo | rces Per | Ply (lbs | s) |
| Cho | rds T | ens.Con | ıp. | Chords | Tens. | Comp. |
| B - (| c | 2327 - 56 | 558 | D-E | 2481 | - 5987 |

Maximum Bot Chord Forces Per Ply (lbs)

| 0110100 | rono.comp. | Cilorac | rono. Comp. |
|---------|-------------|---------|-------------|
| | 4050 0005 | 0 | 4005 4050 |
| B - I | 4953 - 2025 | H - G | 4835 - 1959 |
| I - H | 4980 - 2023 | G-F | 4810 - 1960 |

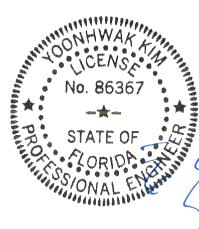
E-F

2256 - 5497

Maximum Web Forces Per Ply (lbs)

2481 - 5988

| Webs | Tens.C | comp. | Webs | Tens. (| Comp. |
|-------|--------|-------|-------|---------|-------|
| C - I | 675 | 0 | H - E | 1382 | - 625 |
| C - H | 1208 | - 548 | G-E | 628 | 0 |
| РШ | 670 | 012 | | | |



COA#0-278, Yoonhwak Kim, FL PE #86367 Flori 24/2026 icate of Product Approval #FL 1999

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SEQN: 103923/ SPEC Ply: 1 Job Number: 402471 Cust: R 8975 JRef: 1YdO89750003 T28 / FROM: ARZ Qty: 1 ,7F5 ,BRAA ,15;16;21;25 ,RB01 / BRADEN/A DrwNo: 267.25.0813.23074 Truss Label: C1 / WHK 09/24/2025 3'2" =5X5 D 13'4' H ≡3X8 **≡4X4(B1)** ≡4X4(B1) 7'8"4 4'2" 4'2" – 1'10**"**8 – ----- 1'10"8 ----4'2' 8'4" Loading Criteria (psf) Wind Criteria Snow Criteria (Pg,Pf in PSF) Defl/CSI Criteria ▲ Maximum Reactions (lbs) Gravity Non-Gravity Wind Std: ASCE 7-22 Pg: NA Ct: NA CAT: NA TCLL: 20.00 PP Deflection in loc L/defl L/# Loc R+ /Rh /Rw /U /RL Speed: 160 mph TCDL: 15.00 Pf: NA Ce: NA VERT(LL): 0.034 E 999 240 Enclosure: Closed VERT(CL): 0.073 E BCI I · 0.00 Lu: NA Cs: NA 999 240 В 496 /368 /183 /118 Risk Category: II BCDL: 10.00 Snow Duration: NA HORZ(LL): 0.010 D /-/441 /213 /-575 EXP: C Kzt: NA Wind reactions based on MWFRS HORZ(TL): 0.022 D Des Ld: 45.00 Mean Height: 15.00 ft Brg Wid = 12.0 Min Req = 2.0 **Building Code:** Creep Factor: 2.0 NCBCLL: 10.00 TCDL: 4.2 psf Brg Wid = 3.5 Min Req = 2.0 FBC 8th Ed. 2023 Res. Max TC CSI: 0.482 Soffit: 0.00 BCDL: 5.0 psf Bearings B & I are a rigid surface. TPI Std: 2014 Max BC CSI: 0.552 Load Duration: 1.25 MWFRS Parallel Dist: 0 to h/2 Bearing B requires a seat plate. Rep Fac: Yes Max Web CSI: 0.085 Spacing: 24.0 " C&C Dist a: 3.00 ft Members not listed have forces less than 375# FT/RT:20(0)/10(0) Loc. from endwall: not in 4.50 ft GCpi: 0.18 Plate Type(s):

Lumber

Top chord: 2x4 SP #2 N; Bot chord: 2x4 SP #2 N; Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind Duration: 1.60

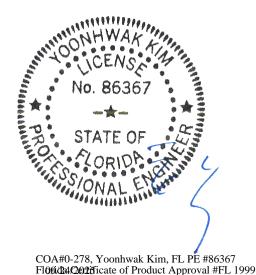
<u>WA</u>VE

Right cantilever is exposed to wind

Wind loading based on both gable and hip roof types.

Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 440 - 346 E-F 432 - 339

Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Tens. Comp. Chords H-F 719 - 812



VIEW Ver: 23.02.01A.1204.18

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SEQN: 837772 FROM: ARZ

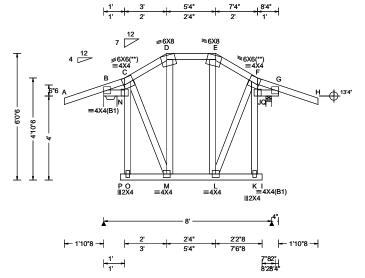
SPEC

Ply: 1 Qty: 1 Job Number: 402471

,7F5 ,BRAA ,15;16;21;25 ,RB01 / BRADEN/A

Truss Label: C2G

Cust: R 8975 JRef: 1YdO89750003 T18 DrwNo: 267.25.0921.06127 KD / YK 09/24/2025



| ▲ Ma | ıximu | m Reac | tions (lbs |) | | |
|------|--------|------------|--------------|----------|----------|------|
| | Gı | avity | | No | n-Grav | ity |
| Loc | R+ | / R- | / Rh | /Rw | / U | / RL |
| В 6 | 664 | /- | /- | /- | /307 | /- |
| Q 8 | 301 | /- | /- | /- | /365 | /- |
| Wind | l reac | tions bas | sed on MV | VFRS | | |
| В | Brg W | id = 7.6 | Min Re | q = 1.5 | (Truss |) |
| Q I | Brg W | id = 3.2 | Min Re | q = 1.5 | (Truss |) |
| Bear | ings E | 8 & Q are | e a rigid su | urface. | | |
| Bear | ing B | requires | a seat pla | ate. | | |
| Mem | bers r | not listed | have force | es less | than 3 | 75# |
| Maxi | imum | Top Ch | ord Force | es Per l | Ply (lbs | s) |
| Chor | ds T | ens.Con | np. | | - ` | • |
| B-C | ; | 159 -4 | 106 | | | |

Maximum Web Forces Per Ply (lbs)

Tens.Comp. 423 - 184

Webs

Top chord: 2x4 SP #2 N; Bot chord: 2x4 SP #2 N; Webs: 2x4 SP #3;

Special Loads

| (Lumber | Dur.Fac.=1. | .25 / Plate D | Our.Fac.=1.2 | 25) |
|------------|-------------|---------------|--------------|-------|
| TC: From | 72 plf at | -2.06 to | 72 plf at | 0.81 |
| TC: From | 75 plf at | 0.81 to | 75 plf at | 2.81 |
| TC: From | 36 plf at | 2.81 to | 36 plf at | 5.15 |
| TC: From | 75 plf at | 5.15 to | 75 plf at | 7.15 |
| TC: From | 72 plf at | 7.15 to | 72 plf at | 10.02 |
| BC: From | 20 plf at | -0.19 to | 20 plf at | 0.81 |
| BC: From | 10 plf at | 0.81 to | 10 plf at | 7.15 |
| BC: From | 20 plf at | 7.15 to | 20 plf at | 8.15 |
| TC: 70 lb | Conc. Load | at 2.88 | • | |
| TC: 140 lb | Conc. Load | at 4.98 | | |
| TC: 134 lb | Conc. Load | at 5.11 | | |
| BC: 86 lb | Conc. Load | at 2.82 | | |
| BC: 73 lb | Conc. Load | at 4.98 | | |
| BC: 64 lb | Conc. Load | at 5.11 | | |
| | | | | |

Plating Notes

(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Wind

Wind loads and reactions based on MWFRS.

Right cantilever is exposed to wind

Wind loading based on both gable and hip roof types.



COA#0-278, Yoonhwak Kim, FL PE #86367 Flon 24 Co26 icate of Product Approval #FL 1999

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

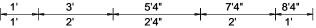
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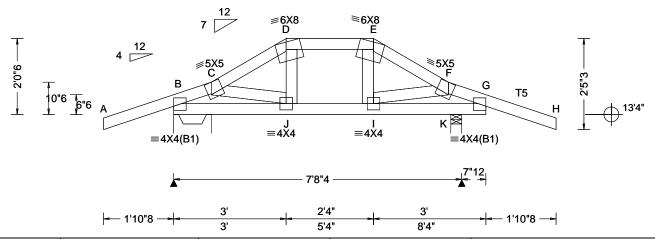
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SEQN: 837774 SPEC Ply: 1 Job Number: 402471 Cust: R 8975 JRef:1YdO89750003 T25 FROM: ARZ ,7F5 ,BRAA ,15;16;21;25 ,RB01 / BRADEN/A DrwNo: 267.25.0921.30007 Qty: 1 Truss Label: C3G KD / YK 09/24/2025





| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria |
|---|--|--|---|
| TCLL: 20.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 0.00 | Wind Std: ASCE 7-22 Speed: 160 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60 | Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE | PP Deflection in loc L/defl L/# VERT(LL): 0.033 J 999 240 VERT(CL): 0.064 J 999 240 HORZ(LL): 0.014 D HORZ(TL): 0.028 D Creep Factor: 2.0 Max TC CSI: 0.383 Max BC CSI: 0.562 Max Web CSI: 0.102 VIEW Ver: 23.02.01A.1204.18 |
| Lumban | | I | 1 |

| | fl L/# 240 | Lo | С |
|----------|---------------|----|-----|
| 999 | 240 | В | 6 |
| - | - | Κ | 6 |
| - | - | Wi | nd |
| | | В | E |
| 3 | | Κ | E |
| 2 | | Ве | ari |
| <u>-</u> | | Ве | ari |
| _ | | Me | ml |
| | | Ma | IX |
| | | Ch | oro |
| .1204. | 18 | | _ |

| | (| Gravity | | Non-Gravity | | |
|-------|--|-------------|-----------|-------------|--------|-------|
| Loc | R+ | / R- | / Rh | / Rw | / U | / RL |
| В | 604 | /- | /- | /- | /249 | /- |
| K | 682 | /- | /- | /- | /286 | /- |
| Win | d rea | ctions b | ased on | MWFRS | | |
| В | Brg | Wid = 12 | 2.0 Min | Req = 1.5 | (Truss | s) |
| K | Brg | Wid = 3. | 5 Min | Req = 1.5 | (Truss | s) |
| Bea | rings | В&Ка | re a rigi | d surface. | | |
| Bea | ring l | B require | es a sea | t plate. | | |
| Mer | nber | s not liste | ed have | forces less | than 3 | 375# |
| Max | Maximum Top Chord Forces Per Ply (lbs) | | | | | |
| Cho | rds | Tens.Co | mp. | Chords | Tens. | Ćomp. |
| B - (| С | 146 | - 502 | E-F | 132 | - 478 |
| C - I | D | - | - 509 | F-G | 62 | - 412 |

▲ Maximum Reactions (lbs)

110 - 405

Top chord: 2x4 SP #2 N; T5 2x4 SP #1; Bot chord: 2x4 SP #1; Webs: 2x4 SP #3;

Special Loads

| (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25) | | | | | |
|--|------------|----------|-----------|-------|--|
| TC: From | 72 plf at | -1.88 to | 72 plf at | 1.00 | |
| TC: From | 75 plf at | 1.00 to | 75 plf at | 3.00 | |
| TC: From | 36 plf at | 3.00 to | 36 plf at | 5.33 | |
| TC: From | 75 plf at | 5.33 to | 75 plf at | 7.33 | |
| TC: From | 72 plf at | 7.33 to | 72 plf at | 10.21 | |
| BC: From | 20 plf at | 0.00 to | 20 plf at | 3.03 | |
| | 10 plf at | 3.03 to | 10 plf at | 5.15 | |
| BC: From | 20 plf at | | 20 plf at | 8.33 | |
| TC: 135 lb | Conc. Load | at 3.03 | | | |
| TC: 180 lb | Conc. Load | at 5.15 | | | |
| BC: 53 lb Conc. Load at 3.03 | | | | | |
| BC: -42 lb | Conc. Load | at 5.15 | | | |

Wind loads and reactions based on MWFRS.

Right cantilever is exposed to wind

Wind loading based on both gable and hip roof types.

| Maximu | Maximum Bot Chord Forces Per Ply (lbs) | | | | |
|----------------|--|--------|---------|-------|--|
| Chords | Tens.Comp. | Chords | Tens. (| Comp. | |
| B - J J - I | 406 - 105 405 - 110 | | 582 | - 34 | |



COA#0-278, Yoonhwak Kim, FL PE #86367 Fload24Ce26ficate of Product Approval #FL 1999

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SEQN: 837764 FROM: ARZ

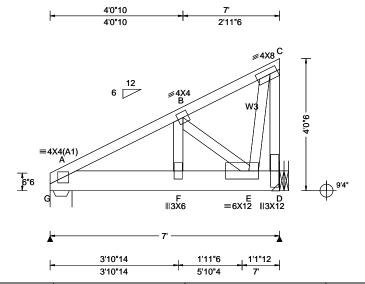
MONO

Ply: 1 Qty: 1 Job Number: 402471

,7F5 ,BRAA ,15;16;21;25 ,RB01 / BRADEN/A

Truss Label: M1G

Cust: R 8975 JRef: 1YdO89750003 T38 DrwNo: 267.25.0922.25063 KD / YK 09/24/2025



| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | DefI/CSI Criteria |
|---|--|---|---|
| TCLL: 20.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Std: ASCE 7-22 Speed: 160 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 | Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): | PP Deflection in loc L/defl L/# VERT(LL): 0.016 F 999 240 VERT(CL): 0.035 F 999 240 HORZ(LL): -0.004 C HORZ(TL): 0.008 C Creep Factor: 2.0 Max TC CSI: 0.160 Max BC CSI: 0.272 Max Web CSI: 0.578 |
| lmbar | Wind Duration: 1.60 | WAVE | VIEW Ver: 23.02.01A.1204.18 |

▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ /Rh /Rw /U /RL G 889 /330 /-2947 /-/-/1164 /-Wind reactions based on MWFRS Brg Wid = 8.0Min Reg = 1.5 (Truss) Brg Wid = -Min Req = -Bearing G is a rigid surface. Bearing G requires a seat plate. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 508 - 1344 B-C 285

Lumber

Top chord: 2x4 SP #2 N; Bot chord: 2x8 SP 2400f-2.0E; Webs: 2x4 SP #3; W3 2x4 SP #2 N;

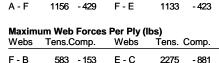
Special Loads

---(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25) TC: From 74 plf at 0.00 to BC: From 10 plf at 0.00 to BC: 137 lb Conc. Load at 2.09 0.00 to 0.00 to 74 plf at 10 plf at 231 lb Conc. Load at 4.09 BC: 2883 lb Conc. Load at 6.06

Hangers / Ties

(J) Hanger Support Required, by others

Wind loads and reactions based on MWFRS. Right end vertical not exposed to wind pressure. Wind loading based on both gable and hip roof types.



Chords

C - D

Tens. Comp.

791

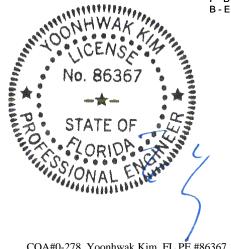
- 2004

Maximum Bot Chord Forces Per Ply (lbs)

- 776

278

Chords Tens.Comp.



COA#0-278, Yoonhwak Kim, FL PE #86367 Fl09.024 C026 ficate of Product Approval #FL 1999

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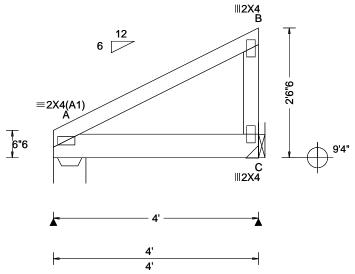
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SEQN: 837691 / MONO Ply: 1 Job Number: 402471 Cust: R 8975 JRef: 1YdO89750003 T52 / FROM: MTV Qty: 1 ,7F5 ,BRAA ,15;16;21;25 ,RB01 / BRADEN/A DrwNo: 267.25.0813.23481 Truss Label: M2G KD / YK 09/24/2025



| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | DefI/CSI Criteria | | | |
|------------------------|---|------------------------------|---------------------------------|--|--|--|
| TCLL: 20.00 | Wind Std: ASCE 7-22 | Pg: NA Ct: NA CAT: NA | PP Deflection in loc L/defl L/# | | | |
| TCDL: 15.00 | Speed: 160 mph | Pf: NA Ce: NA | VERT(LL): NA | | | |
| BCLL: 0.00 | Enclosure: Closed | Lu: NA Cs: NA | VERT(CL): NA | | | |
| BCDL: 10.00 | Risk Category: II | Snow Duration: NA | HORZ(LL): 0.009 A | | | |
| Des Ld: 45.00 | EXP: C Kzt: NA Mean Height: 15.00 ft | | HORZ(TL): 0.020 A | | | |
| NCBCLL: 10.00 | TCDL: 4.2 psf | Building Code: | Creep Factor: 2.0 | | | |
| Soffit: 0.00 | BCDL: 5.0 psf | FBC 8th Ed. 2023 Res. | Max TC CSI: 0.534 | | | |
| Load Duration: 1.25 | MWFRS Parallel Dist: 0 to h/2 | TPI Std: 2014 | Max BC CSI: 0.902 | | | |
| Spacing: 24.0 " | C&C Dist a: 3.00 ft | Rep Fac: Varies by Ld Case | Max Web CSI: 0.151 | | | |
| | Loc. from endwall: not in 4.50 ft | FT/RT:20(0)/10(0) | | | | |
| | GCpi: 0.18 | Plate Type(s): | | | | |
| | Wind Duration: 1.60 | WAVE | VIEW Ver: 23.02.01A.1204.18 | | | |
| Lumbor | | | | | | |

| ▲ M | aximu | ım Read | tions (lb | s) | | |
|------------|--------|------------|------------|-----------|----------|------|
| | G | ravity | - | No | on-Grav | vity |
| Loc | R+ | / R- | / Rh | /Rw | / U | / RL |
| Α | 1134 | /- | /- | /- | /221 | /- |
| С | 1110 | /- | /- | /- | /214 | /- |
| Win | d read | tions ba | sed on M | WFRS | | |
| Α | Brg V | /id = 7.6 | Min R | eq = 1.5 | (Truss | s) |
| С | Brg V | /id = - | Min R | eq = - | | |
| Bea | ring A | is a rigio | surface. | | | |
| Bea | ring A | requires | a seat p | late. | | |
| Men | nbers | not listed | d have for | rces less | s than 3 | 375# |
| | | | | | | |

Lumber

Top chord: 2x4 SP #2 N; Bot chord: 2x6 SP SS; Webs: 2x4 SP #3;

Special Loads

---(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25) TC: From 74 plf at 0.00 to BC: From 20 plf at 0.00 to BC: 1870 lb Conc. Load at 2.06 TC: From BC: From 0.00 to 0.00 to 74 plf at 20 plf at 4 00

Hangers / Ties

(J) Hanger Support Required, by others

Wind loads and reactions based on MWFRS. Right end vertical not exposed to wind pressure. Wind loading based on both gable and hip roof types.



COA#0-278, Yoonhwak Kim, FL PE #86367 Flori 24/2026 ficate of Product Approval #FL 1999

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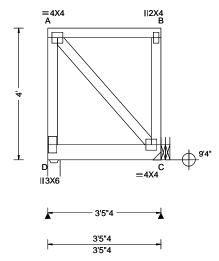
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155 Harlem Ave North Building, 4th Floor Glenview, IL 60025

SEQN: 837776 FLAT Ply: 2 Job Number: 402471 Cust: R 8975 JRef: 1YdO89750003 T53 DrwNo: 267.25.0922.10650 FROM: ARZ ,7F5 ,BRAA ,15;16;21;25 ,RB01 / BRADEN/A Qty: 1 Truss Label: FG1 KD / YK 09/24/2025

2 Complete Trusses Required



| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in F | PSF) Defl/CSI Criteria |
|------------------------|---|---|--------------------------------------|
| TCLL: 20.00 | Wind Std: ASCE 7-22 | Pg: NA Ct: NA CAT | : NA PP Deflection in loc L/defl L/# |
| TCDL: 15.00 | Speed: 160 mph | Pf: NA Ce: I | NA VERT(LL): 0.001 A 999 240 |
| BCLL: 0.00 | Enclosure: Closed | Lu: NA Cs: NA | VERT(CL): 0.002 A 999 240 |
| BCDL: 10.00 | Risk Category: II | Snow Duration: NA | HORZ(LL): -0.001 B |
| Des Ld: 45.00 | EXP: C Kzt: NA Mean Height: 15.00 ft | Duilding Code | HORZ(TL): 0.002 B |
| NCBCLL: 0.00 | TCDL: 4.2 psf | Building Code: FBC 8th Ed. 2023 Res. | Creep Factor: 2.0 Max TC CSI: 0.551 |
| Soffit: 0.00 | BCDL: 5.0 psf | | |
| Load Duration: 1.25 | MWFRS Parallel Dist: 0 to h/2 | TPI Std: 2014 | Max BC CSI: 0.820 |
| Spacing: 24.0 " | C&C Dist a: NA | Rep Fac: No | Max Web CSI: 0.109 |
| | Loc. from endwall: Any | FT/RT:20(0)/10(0) | |
| | GCpi: 0.18 | Plate Type(s): | |
| | Wind Duration: 1.60 | WAVE | VIEW Ver: 23.02.01A.1204.18 |
| Lumber | - | Additional Notes | - |

▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ /Rh /Rw /U /RL 1566 /-/637 1164 /-/-/-/445 Wind reactions based on MWFRS Brg Wid = 4.5 Min Req = 1.5 (Truss) Brg Wid = -Min Reg = Bearing D is a rigid surface. Bearing D requires a seat plate. Members not listed have forces less than 375#

Top chord: 2x4 SP #2 N; Bot chord: 2x6 SP #2 N; Webs: 2x4 SP #3;

Nailnote

Nail Schedule:0.128"x3", min. nails Top Chord: 1 Row @ 5.75" o.c. Bot Chord: 2 Rows @ 6.00" o.c. (Each Row) Webs : 1 Row @ 4" o.c. Use equal spacing between rows and stagger nails in each row to avoid splitting.

Special Loads

--(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25) TC: From 35 plf at 0.00 to 35 plf at 0.00 to 10 plf at 10 plf at TC: 801 lb Conc. Load at 0.65 TC: 49 lb Conc. Load at 3.33 BC: 1726 lb Conc. Load at 1.77

Hangers / Ties

(J) Hanger Support Required, by others

The TC of this truss shall be braced with attached spans at 24" oc in lieu of structural sheathing.

Wind loads and reactions based on MWFRS. End verticals not exposed to wind pressure.

THIS TRUSS MUST BE INSTALLED AS SHOWN AND NOT END FOR END.

additional Notes

Truss must be installed as shown with top chord up.



COA#0-278, Yoonhwak Kim, FL PE #86367 Fl09/04/002fficate of Product Approval #FL 1999

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

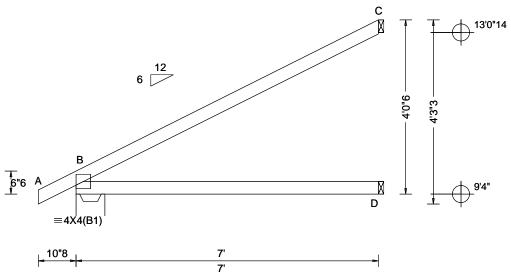
IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS
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For more information see these web sites: Alpine: alpineitw.com: TPI: binst.org: SBCA: sbcacomponents.com: ICC: iccsafe.org: AWC: awc.org



SEQN: 837604 / **EJAC** Ply: 1 Job Number: 402471 Cust: R 8975 JRef: 1YdO89750003 T32 / FROM: RAA ,7F5 ,BRAA ,15;16;21;25 ,RB01 / BRADEN/A Qty: 18 DrwNo: 267.25.0813.21993 Truss Label: EJ7 KD / WHK 09/24/2025



| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | |
|---|---|--|---|--|
| TCLL: 20.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 | Wind Criteria Wind Std: ASCE 7-22 Speed: 160 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft | Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) | Defi/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): -0.014 C HORZ(TL): 0.027 B Creep Factor: 2.0 Max TC CSI: 0.805 Max BC CSI: 0.588 Max Web CSI: 0.000 | |
| Lumbor | GCpi: 0.18 Wind Duration: 1.60 | Plate Type(s): WAVE | VIEW Ver: 23.02.01A.1204.18 | |

| ▲ Ma | ximu | ım Rea | actions (II | os) | | |
|-------|-------|----------|-------------|------------|----------|------|
| | G | ravity | | No | on-Grav | /ity |
| Loc | R+ | / R- | / Rh | / Rw | / U | / RL |
| B 4 | 01 | /- | /- | /317 | /99 | /205 |
| D 1 | 35 | /- | /- | /78 | /- | /- |
| C 2 | 37 | /- | /- | /189 | /182 | /- |
| Wind | reac | tions b | ased on N | /WFRS | | |
| ВЕ | 3rg V | /id = 8 | .0 Min F | Req = 1.5 | 5 | |
| D E | 3rg V | /id = 1 | .5 Min F | Req = - | | |
| C E | 3rg V | /id = 1 | .5 Min F | Req = - | | |
| Beari | ng B | is a rig | id surface | e | | |
| Beari | ng B | require | es a seat i | olate. | | |
| Mem | bers | not list | ed have fo | orces less | s than 3 | 375# |

Top chord: 2x4 SP #1; Bot chord: 2x4 SP #2 N;

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

Provide (3) 0.131"x3.0", min. toe-nails at top chord. Provide (2) 0.131"x3.0", min. toe-nails at bottom chord.



COA#0-278, Yoonhwak Kim, FL PE #86367 Floricate of Product Approval #FL 1999

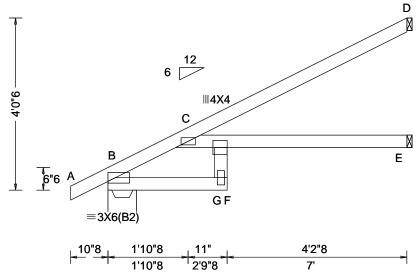
WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

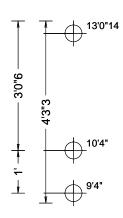
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SEQN: 837528 / **EJAC** Ply: 1 Job Number: 402471 Cust: R 8975 JRef: 1YdO89750003 T19 / FROM: ARZ Qty: 6 ,7F5 ,BRAA ,15;16;21;25 ,RB01 / BRADEN/A DrwNo: 267.25.0813.24092 Truss Label: EJ7T KD / YK 09/24/2025





| Loading Criteria (psf | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | DefI/CSI Criteria |
|-----------------------|--|------------------------------|---------------------------------|
| TCLL: 20.00 | Wind Std: ASCE 7-22 | Pg: NA Ct: NA CAT: NA | PP Deflection in loc L/defl L/# |
| TCDL: 15.00 | Speed: 160 mph | Pf: NA Ce: NA | VERT(LL): 0.138 F 601 240 |
| BCLL: 0.00 | Enclosure: Closed | Lu: NA Cs: NA | VERT(CL): 0.315 F 263 240 |
| BCDL: 10.00 | Risk Category: II | Snow Duration: NA | HORZ(LL): 0.071 C |
| Des Ld: 45.00 | EXP: C Kzt: NA | | HORZ(TL): 0.162 C |
| NCBCLL: 10.00 | Mean Height: 15.00 ft TCDL: 4.2 psf | Building Code: | Creep Factor: 2.0 |
| Soffit: 0.00 | BCDL: 5.0 psf | FBC 8th Ed. 2023 Res. | Max TC CSI: 0.709 |
| Load Duration: 1.25 | MWFRS Parallel Dist: h/2 to h | TPI Std: 2014 | Max BC CSI: 0.364 |
| Spacing: 24.0 " | C&C Dist a: 3.00 ft | Rep Fac: Yes | Max Web CSI: 0.314 |
| ' ' | Loc. from endwall: not in 9.00 ft | FT/RT:20(0)/10(0) | |
| | GCpi: 0.18 | Plate Type(s): | |
| | Wind Duration: 1.60 | WAVE | VIEW Ver: 23.02.01A.1204.18 |

| | ▲ M | laxim | um Rea | actions (Ib | os) | | |
|---|-----|---------|----------|-------------|--------------|----------|------|
| # | | G | avity | | No | on-Grav | vity |
|) | Loc | R+ | / R- | / Rh | / Rw | / U | / RL |
| 5 | В | 401 | /- | /- | /317 | /99 | /205 |
| | Е | 127 | /- | /0 | /82 | /0 | /- |
| | D | 230 | /- | /- | /186 | /168 | /- |
| | Win | d read | ctions b | ased on N | MWFRS | | |
| | В | Brg V | Vid = 8 | .0 Min F | Req = 1.5 | (Trus | s) |
| | Е | Brg V | Vid = 1 | .5 Min F | Req = - | - | - |
| | D | | | .5 Min F | | | |
| | Bea | ıring E | is a rig | gid surface |) <u>.</u> | | |
| | Bea | ıring B | requir | es a seat p | olate. | | |
| | Mer | nbers | not list | ed have fo | rces les | s than 3 | 375# |
| | | | | | | | |

Lumber

Top chord: 2x4 SP #1; Bot chord: 2x4 SP #1; Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4 except as noted.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

Provide (3) 0.131"x3.0", min. toe-nails at top chord. Provide (2) 0.131"x3.0", min. toe-nails at bottom chord.

Note: Laterally brace bottom chord above filler at 2'0" O.C.Max. including a lateral brace at chord ends.



COA#0-278, Yoonhwak Kim, FL PE #86367 Floricate of Product Approval #FL 1999

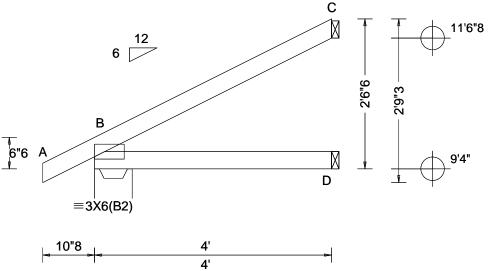
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SEQN: 837687 / MONO Ply: 1 Job Number: 402471 Cust: R 8975 JRef: 1YdO89750003 T26 / FROM: MTV ,7F5 ,BRAA ,15;16;21;25 ,RB01 / BRADEN/A Qty: 1 DrwNo: 267.25.0813.23983 Truss Label: EJ4 KD / YK 09/24/2025



Defl/CSI Criteria

| Loading Criteria (psi) | Willia Ciliteria | SHOW CITICITIA (F9,F1111F3F) | Deli/Col Cilleria |
|------------------------|---|------------------------------|---------------------------------|
| TCLL: 20.00 | Wind Std: ASCE 7-22 | Pg: NA Ct: NA CAT: NA | PP Deflection in loc L/defl L/# |
| TCDL: 15.00 | Speed: 160 mph | Pf: NA Ce: NA | VERT(LL): NA |
| BCLL: 0.00 | Enclosure: Closed | Lu: NA Cs: NA | VERT(CL): NA |
| BCDL: 10.00 | Risk Category: II | Snow Duration: NA | HORZ(LL): 0.002 B |
| Des Ld: 45.00 | EXP: C Kzt: NA Mean Height: 15.00 ft | | HORZ(TL): 0.005 B |
| NCBCLL: 10.00 | TCDL: 4.2 psf | Building Code: | Creep Factor: 2.0 |
| Soffit: 0.00 | BCDL: 5.0 psf | FBC 8th Ed. 2023 Res. | Max TC CSI: 0.436 |
| Load Duration: 1.25 | MWFRS Parallel Dist: 0 to h/2 | TPI Std: 2014 | Max BC CSI: 0.176 |
| Spacing: 24.0 " | C&C Dist a: 3.00 ft | Rep Fac: Yes | Max Web CSI: 0.000 |
| - | Loc. from endwall: not in 4.50 ft | FT/RT:20(0)/10(0) | |
| | GCpi: 0.18 | Plate Type(s): | |
| | Wind Duration: 1.60 | WAVE | VIEW Ver: 23.02.01A.1204.18 |
| Lumber | • | • | |

Snow Criteria (Pa Pf in PSE)

| num Rea | actions (II | os) | | |
|------------|--|--|----------|--|
| Gravity | | No | on-Grav | vity |
| / R- | / Rh | / Rw | / U | / RL |
| /- | /- | /218 | /70 | /125 |
| /- | /- | /43 | /- | /- |
| /- | /- | /103 | /104 | /- |
| actions b | ased on N | /WFRS | | |
| Wid = 7 | .6 Min F | Req = 1.5 | (Trus | s) |
| Wid = 1 | .5 Min F | Req = - | • | • |
| Wid = 1 | .5 Min F | Req = - | | |
| B is a rig | gid surface |). | | |
| B requir | es a seat ¡ | olate. | | |
| s not list | ed have fo | orces les | s than 3 | 375# |
| | Gravity /R- /- /- actions b Wid = 7 Wid = 1 Wid = 1 B is a rig B require | Gravity /R- /Rh /- /- /- /- /- cactions based on N Wid = 7.6 Min F Wid = 1.5 Min F Wid = 1.5 Min F B is a rigid surface B requires a seat | Gravity | Gravity Non-Gravity / Rh / Rw / U / Non-Gravity / Rh / Rw / U / Non-Gravity / Rw / U / Non-Gravity / Rw / U / Non-Gravity / Non- |

Top chord: 2x4 SP #2 N; Bot chord: 2x4 SP #2 N;

Loading Criteria (nef) Wind Criteria

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

Provide (2) 0.131"x3.0", min. toe-nails at top chord. Provide (2) 0.131"x3.0", min. toe-nails at bottom chord.



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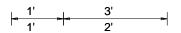
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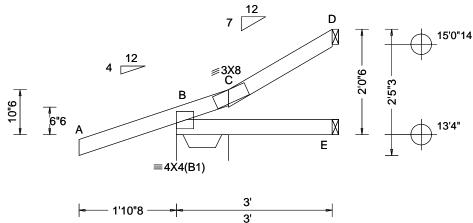
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SEQN: 103925/ **EJAC** Ply: 1 Job Number: 402471 Cust: R 8975 JRef: 1YdO89750003 T42 / FROM: ARZ ,7F5 ,BRAA ,15;16;21;25 ,RB01 / BRADEN/A DrwNo: 267.25.0813.22886 Qty: 3 Truss Label: EJ3 / WHK 09/24/2025





| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | ١. |
|---|--|------------------------------|---|----|
| TCLL: 20.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Std: ASCE 7-22 Speed: 160 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60 | • | PP Deflection in loc L/defl L/# VERT(LL): 0.011 C 999 240 VERT(CL): 0.017 C 999 240 HORZ(LL): 0.006 C HORZ(TL): 0.008 C Creep Factor: 2.0 Max TC CSI: 0.504 Max BC CSI: 0.103 Max Web CSI: 0.000 VIEW Ver: 23.02.01A.1204.18 | |
| Lauraban | | | | |

| ▲ Ma | aximu | ım Rea | actions | (lbs) | | | |
|------|--------|----------|-----------|----------|-----|----------|------|
| | G | ravity | | | No | on-Grav | vity |
| Loc | R+ | / R- | / Rh | / F | ₹w | / U | / RL |
| в : | 327 | /- | /- | /2 | 65 | /139 | /105 |
| E : | 51 | /- | /- | /3 | 8 | /- | /- |
| D (| 68 | /- | /- | /6 | 0 | /62 | |
| Wind | d read | ctions b | ased or | MWFI | RS | | |
| В | Brg V | Vid = 1: | 2.0 Mir | Req = | 2.0 |) | |
| E | Brg V | Vid = 1 | .5 Mir | Req = | - | | |
| | | | .5 Mir | | | | |
| Bear | ring B | is a rig | gid surfa | ce. | | | |
| Bear | ring B | require | es a sea | t plate. | | | |
| Men | bers | not list | ed have | forces | les | s than 3 | 375# |

Top chord: 2x4 SP #2 N; Bot chord: 2x4 SP #2 N;

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

Provide (2) 0.131"x3.0", min. toe-nails at top chord. Provide (2) 0.131"x3.0", min. toe-nails at bottom chord.



COA#0-278, Yoonhwak Kim, FL PE #86367 Fl09/24/2026 icate of Product Approval #FL 1999

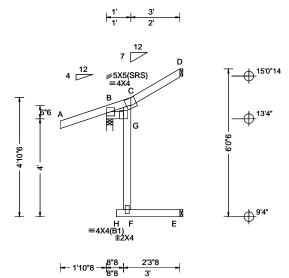
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SEQN: 103926/ **EJAC** Ply: 1 Job Number: 402471 Cust: R 8975 JRef: 1YdO89750003 T37 / FROM: ARZ Qty: 3 ,7F5 ,BRAA ,15;16;21;25 ,RB01 / BRADEN/A DrwNo: 267.25.0813.22415 Truss Label: EJ3A / WHK 09/24/2025



| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | DefI/CSI Criteria | ſ |
|---|--|--|---|---|
| TCLL: 20.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 0.00 | Wind Std: ASCE 7-22 Speed: 160 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.0 psf | Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 | DefI/CSI Criteria | 1 |
| Load Duration: 1.25 Spacing: 24.0 " | MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60 | Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | Max Web CSI: 0.035 VIEW Ver: 23.02.01A.1204.18 | |
| Lumber | | | | |

| / Rh /- /- | / Rw /270 | on-Grav / U /136 | / RL |
|------------------|--|---|--|
| /- | /270 | /136 | |
| • | | | /105 |
| /- | /17 | | |
| | /1/ | /- | /- |
| /- | /83 | /56 | /- |
| ased on N | MWFRS | | |
| 2 Min F | Req = 2.0 |) | |
| 5 Min F | Req = - | | |
| | | | |
| d surface | э. | | |
| d have fo | orces les | s than 3 | 375# |
| | /- ased on M 2 Min | /- /83 ased on MWFRS Min Req = 2.0 Min Req = - Min Req = - Min Req = - d surface. | /- /83 /56 ased on MWFRS 2 Min Req = 2.0 5 Min Req = - 5 Min Req = - |

Top chord: 2x4 SP #2 N; Bot chord: 2x4 SP #2 N; Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

Provide (2) 0.131"x3.0", min. toe-nails at top chord. Provide (2) 0.131"x3.0", min. toe-nails at bottom chord.



COA#0-278, Yoonhwak Kim, FL PE #86367 Flooda4Central FL 1999

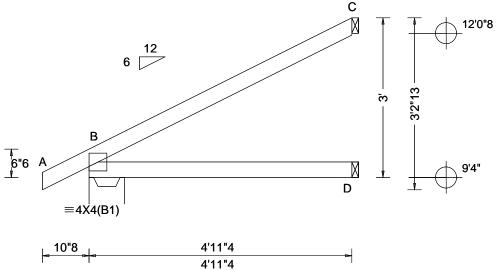
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SEQN: 837610/ JACK Ply: 1 Job Number: 402471 Cust: R 8975 JRef: 1YdO89750003 T29 / FROM: MTV ,7F5 ,BRAA ,15;16;21;25 ,RB01 / BRADEN/A DrwNo: 267.25.0813.22055 Qty: 8 Truss Label: CJ5 KD / WHK 09/24/2025



| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria |
|------------------------|---|------------------------------|---------------------------------|
| TCLL: 20.00 | Wind Std: ASCE 7-22 | Pg: NA Ct: NA CAT: NA | PP Deflection in loc L/defl L/# |
| TCDL: 15.00 | Speed: 160 mph | Pf: NA Ce: NA | VERT(LL): NA |
| BCLL: 0.00 | Enclosure: Closed | Lu: NA Cs: NA | VERT(CL): NA |
| BCDL: 10.00 | Risk Category: II | Snow Duration: NA | HORZ(LL): -0.006 C |
| Des Ld: 45.00 | EXP: C Kzt: NA Mean Height: 15.00 ft | | HORZ(TL): 0.010 B |
| NCBCLL: 10.00 | TCDL: 4.2 psf | Building Code: | Creep Factor: 2.0 |
| Soffit: 0.00 | BCDL: 5.0 psf | FBC 8th Ed. 2023 Res. | Max TC CSI: 0.654 |
| Load Duration: 1.25 | MWFRS Parallel Dist: 0 to h/2 | TPI Std: 2014 | Max BC CSI: 0.282 |
| Spacing: 24.0 " | C&C Dist a: 3.00 ft | Rep Fac: Yes | Max Web CSI: 0.000 |
| | Loc. from endwall: not in 4.50 ft | FT/RT:20(0)/10(0) | |
| | GCpi: 0.18 | Plate Type(s): | |
| | Wind Duration: 1.60 | WAVE | VIEW Ver: 23.02.01A.1204.18 |
| Lumber | | • | |

| ▲ Ma | ıximı | ım Rea | ctions (II | os) | | |
|----------------------------------|-------|----------|------------|-----------|----------|--------|
| | G | ravity | | No | on-Grav | vity . |
| Loc | R+ | / R- | / Rh | / Rw | / U | / RL |
| в 3 | 306 | /- | /- | /249 | /79 | /150 |
| D 9 | 96 | /- | /- | /55 | /- | /- |
| C 1 | 163 | /- | /- | /130 | /128 | /- |
| Wind reactions based on MWFRS | | | | | | |
| ΒΙ | Brg V | Vid = 8. | 0 Min F | Req = 1.5 | 5 | |
| D I | Brg V | Vid = 1. | 5 Min F | ?eq = - | | |
| | | | 5 Min F | | | |
| Bearing B is a rigid surface. | | | | | | |
| Bearing B requires a seat plate. | | | | | | |
| | _ | | ed have fo | | s than 3 | 375# |

Top chord: 2x4 SP #2 N; Bot chord: 2x4 SP #2 N;

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

Provide (2) 0.131"x3.0", min. toe-nails at top chord. Provide (2) 0.131"x3.0", min. toe-nails at bottom chord.



COA#0-278, Yoonhwak Kim, FL PE #86367 Flori 24/2026 icate of Product Approval #FL 1999



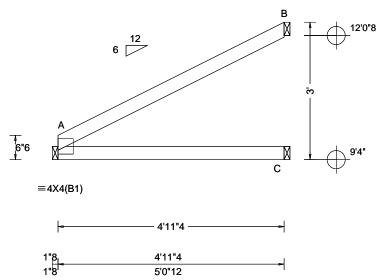
SEQN: 103928/ FROM: ARZ

JACK Ply: 1 Qty: 1

Job Number: 402471

,7F5 ,BRAA ,15;16;21;25 ,RB01 / BRADEN/A Truss Label: CJ5A

Cust: R 8975 JRef: 1YdO89750003 T14 / DrwNo: 267.25.0813.23027 KD / YK 09/24/2025



| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria |
|------------------------|--|------------------------------|---------------------------------|
| TCLL: 20.00 | Wind Std: ASCE 7-22 | Pg: NA Ct: NA CAT: NA | PP Deflection in loc L/defl L/# |
| TCDL: 15.00 | Speed: 160 mph | Pf: NA Ce: NA | VERT(LL): NA |
| BCLL: 0.00 | Enclosure: Closed | Lu: NA Cs: NA | VERT(CL): NA |
| BCDL: 10.00 | Risk Category: II | Snow Duration: NA | HORZ(LL): -0.006 A |
| Des Ld: 45.00 | EXP: C Kzt: NA | | HORZ(TL): 0.013 A |
| NCBCLL: 10.00 | Mean Height: 15.00 ft TCDL: 4.2 psf | Building Code: | Creep Factor: 2.0 |
| Soffit: 0.00 | BCDL: 5.0 psf | FBC 8th Ed. 2023 Res. | Max TC CSI: 0.663 |
| Load Duration: 1.25 | MWFRS Parallel Dist: 0 to h/2 | TPI Std: 2014 | Max BC CSI: 0.298 |
| Spacing: 24.0 " | C&C Dist a: 3.00 ft | Rep Fac: Yes | Max Web CSI: 0.000 |
| | Loc. from endwall: not in 4.50 ft | FT/RT:20(0)/10(0) | |
| | GCpi: 0.18 | Plate Type(s): | |
| | Wind Duration: 1.60 | WAVE | VIEW Ver: 23.02.01A.1204.18 |
| Lumber | | | |

| ▲ Maximum Reactions (lbs) | | | | | | | |
|---------------------------|---------|----------|------------|------------|----------|------|--|
| | G | ravity | | No | on-Grav | ∕ity | |
| Loc | R+ | / R- | / Rh | / Rw | / U | /RL | |
| Α | 231 | /- | /- | /166 | /45 | /131 | |
| С | 98 | /- | /- | /60 | /- | /- | |
| В | 168 | /- | /- | /135 | /128 | /- | |
| Wir | nd read | ctions b | ased on N | /WFRS | | | |
| Α | Brg V | Vid = 1. | 5 Min F | Req = - | | | |
| С | Brg V | Vid = 1. | 5 Min F | Req = - | | | |
| В | Brg V | Vid = 1. | 5 Min F | Req = - | | | |
| Mei | mbers | not list | ed have fo | orces less | s than 3 | 375# | |

Top chord: 2x4 SP #2 N; Bot chord: 2x4 SP #2 N;

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

Provide (2) 0.131"x3.0", min. toe-nails at top chord. Provide (3) 0.131"x3.0", min. toe-nails at bottom chord.



COA#0-278, Yoonhwak Kim, FL PE #86367 Fl09.024 Oct Ficate of Product Approval #FL 1999

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SEQN: 103929/ FROM: ARZ

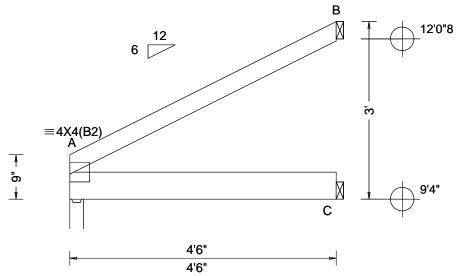
JACK Qty: 1

Ply: 1 Job Number: 402471

,7F5 ,BRAA ,15;16;21;25 ,RB01 / BRADEN/A

Truss Label: CJ5B

Cust: R 8975 JRef: 1YdO89750003 T9 / DrwNo: 267.25.0813.22416 / WHK 09/24/2025



| Loading | Officeria (pai) | *************************************** |
|----------|-----------------|---|
| TCLL: | 20.00 | Wind Std: |
| TCDL: | 15.00 | Speed: 16 |
| BCLL: | 0.00 | Enclosure: |
| BCDL: | 10.00 | Risk Categ |
| Des Ld: | 45.00 | EXP: C |
| NCBCLL: | | Mean Heig |
| Soffit: | | TCDL: 4.2 BCDL: 5.0 |
| | ation: 1.25 | MWFRS P |
| Spacing: | | C&C Dist a |
| opacing. | 24.0 | Loc. from |
| | | LOC. HOIH 6 |

Loading Criteria (psf) Wind Criteria **ASCE 7-22** 60 mph Closed gory: II Kzt: NA ght: 15.00 ft psf psf

arallel Dist: 0 to h/2 a: 3.00 ft endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF) Ct: NA CAT: NA Pg: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA

Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE

Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): -0.006 B HORZ(TL): 0.008 A Creep Factor: 2.0 Max TC CSI: 0.575

Max BC CSI: 0.112 Max Web CSI: 0.000

VIEW Ver: 23.02.01A.1204.18

▲ Maximum Reactions (lbs) Gravity

Non-Gravity /Rw /U Loc R+ /Rh /RL 212 /-/152 /40 /119 С /-/56 94 /-В 151 /121 /120 Wind reactions based on MWFRS Brg Wid = 2.8 Min Req = 2.0

Brg Wid = 1.5 Min Req = -Brg Wid = 1.5 Min Req = -

Bearing A is a rigid surface. Bearing A requires a seat plate.

Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2 N; Bot chord: 2x6 SP #2 N;

Wind loads based on MWFRS with additional C&C

Wind loading based on both gable and hip roof types.

Additional Notes

Provide (2) 0.131"x3.0", min. toe-nails at top chord. Provide (2) 0.131"x3.0", min. toe-nails at bottom chord.



COA#0-278, Yoonhwak Kim, FL PE #86367 Fl09/24/2026 Ficate of Product Approval #FL 1999

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155 Harlem Ave North Building, 4th Floor Glenview, IL 60025

For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

SEQN: 837608/ FROM: MTV

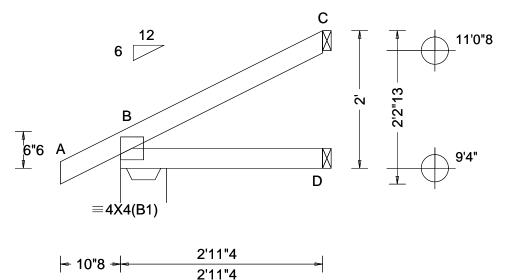
JACK

Ply: 1 Qty: 10 Job Number: 402471

,7F5 ,BRAA ,15;16;21;25 ,RB01 / BRADEN/A

Truss Label: CJ3

Cust: R 8975 JRef: 1YdO89750003 T30 / DrwNo: 267.25.0813.22024 KD / YK 09/24/2025



| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria |
|------------------------|---|------------------------------|---------------------------------|
| TCLL: 20.00 | Wind Std: ASCE 7-22 | Pg: NA Ct: NA CAT: NA | PP Deflection in loc L/defl L/# |
| TCDL: 15.00 | Speed: 160 mph | Pf: NA Ce: NA | VERT(LL): NA |
| BCLL: 0.00 | Enclosure: Closed | Lu: NA Cs: NA | VERT(CL): NA |
| BCDL: 10.00 | Risk Category: II | Snow Duration: NA | HORZ(LL): -0.001 C |
| Des Ld: 45.00 | EXP: C Kzt: NA Mean Height: 15.00 ft | | HORZ(TL): 0.002 C |
| NCBCLL: 10.00 | TCDL: 4.2 psf | Building Code: | Creep Factor: 2.0 |
| Soffit: 0.00 | BCDL: 5.0 psf | FBC 8th Ed. 2023 Res. | Max TC CSI: 0.332 |
| Load Duration: 1.25 | MWFRS Parallel Dist: 0 to h/2 | TPI Std: 2014 | Max BC CSI: 0.086 |
| Spacing: 24.0 " | C&C Dist a: 3.00 ft | Rep Fac: Yes | Max Web CSI: 0.000 |
| | Loc. from endwall: Any | FT/RT:20(0)/10(0) | |
| | GCpi: 0.18 | Plate Type(s): | |
| | Wind Duration: 1.60 | WAVE | VIEW Ver: 23.02.01A.1204.18 |
| Lumber | | | _ |

| ▲ Maximum Reactions (lbs) | | | | | | | |
|---|-------|----------|---------|---------|--------|------|--|
| | G | ravity | | No | on-Gra | vity | |
| Loc | R+ | / R- | / Rh | / Rw | / U | / RL | |
| В | 218 | /- | /- | /185 | /60 | /96 | |
| D | 55 | /- | /- | /29 | /- | /- | |
| С | 91 | /- | /- | /71 | /75 | /- | |
| Wind reactions based on MWFRS | | | | | | | |
| B Brg Wid = 8.0 Min Reg = 1.5 | | | | | | | |
| D | Brg V | Vid = 1. | 5 Min F | ?eq = - | | | |
| | | | 5 Min F | | | | |
| Bearing B is a rigid surface. | | | | | | | |
| Bearing B requires a seat plate. | | | | | | | |
| Bearing B requires a seat plate. Members not listed have forces less than 375# | | | | | | | |

Top chord: 2x4 SP #2 N; Bot chord: 2x4 SP #2 N;

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

Provide (2) 0.131"x3.0", min. toe-nails at top chord. Provide (2) 0.131"x3.0", min. toe-nails at bottom chord.



COA#0-278, Yoonhwak Kim, FL PE #86367 Floricate of Product Approval #FL 1999



SEQN: 103931/ FROM: ARZ

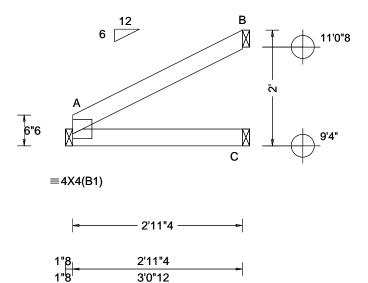
JACK

Ply: 1 Qty: 1 Job Number: 402471

,7F5 ,BRAA ,15;16;21;25 ,RB01 / BRADEN/A

Truss Label: CJ3A

Cust: R 8975 JRef: 1YdO89750003 T15 / DrwNo: 267.25.0813.22698 / WHK 09/24/2025



| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | DefI/CSI Criteria |
|---|---|--|--|
| TCLL: 20.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Std: ASCE 7-22 Speed: 160 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60 | Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): -0.002 A HORZ(TL): 0.003 A Creep Factor: 2.0 Max TC CSI: 0.365 Max BC CSI: 0.145 Max Web CSI: 0.000 VIEW Ver: 23.02.01A.1204.18 |

| | G | ravity | | No | on-Gra | vity |
|-----|--------|----------|-----------|---------|--------|------|
| Loc | R+ | / R- | / Rh | / Rw | / U | / RL |
| | 137 | /- | /- | /98 | /24 | /77 |
| С | 58 | /- | /- | /35 | /- | /- |
| В | 101 | /- | /- | /81 | /78 | /- |
| Win | d read | ctions b | ased on N | /WFRS | | |
| Α | Brg V | Vid = 1. | 5 Min F | Req = - | | |
| С | Brg V | Vid = 1. | 5 Min F | Req = - | | |
| В | Bra V | Vid = 1. | 5 Min F | Reg = - | | |

Top chord: 2x4 SP #2 N; Bot chord: 2x4 SP #2 N;

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

Provide (2) 0.131"x3.0", min. toe-nails at top chord. Provide (2) 0.131"x3.0", min. toe-nails at bottom chord.



COA#0-278, Yoonhwak Kim, FL PE #86367 Floricate of Product Approval #FL 1999

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SEQN: 103932/ FROM: ARZ

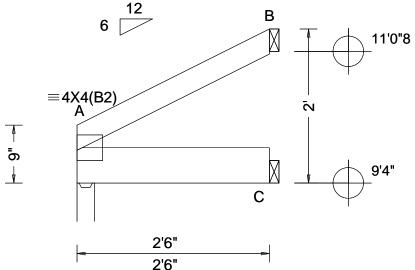
JACK Ply: 1 Qty: 1

Job Number: 402471

,7F5 ,BRAA ,15;16;21;25 ,RB01 / BRADEN/A

Truss Label: CJ3B

Cust: R 8975 JRef: 1YdO89750003 T20 / DrwNo: 267.25.0813.22086 / WHK 09/24/2025



| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria |
|---|---|--|---|
| TCLL: 20.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Criteria Wind Std: ASCE 7-22 Speed: 160 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60 | Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): -0.001 B HORZ(TL): 0.002 B Creep Factor: 2.0 Max TC CSI: 0.278 Max BC CSI: 0.037 Max Web CSI: 0.000 |
| Lumbor | | 1447.44 | 1 |

| ▲ Ma | axim | um Rea | actions (II | bs) | | |
|------|--------|----------|-------------|--------------|--------|------|
| | G | ravity | | No | on-Gra | vity |
| Loc | R+ | / R- | / Rh | / Rw | / U | / RL |
| A | 118 | /- | /- | /84 | /19 | /65 |
| C : | 51 | /- | /- | /29 | /- | /- |
| В | 86 | /- | /- | /69 | /70 | /- |
| Wine | d read | ctions b | ased on N | IWFRS | | |
| Α | Brg V | Vid = 2 | .8 Min F | Req = 2.0 |) | |
| С | Brg V | Vid = 1 | .5 Min F | Req = - | | |
| В | Brg V | Vid = 1 | .5 Min F | Req = - | | |
| Bear | ring A | is a rig | gid surface | e | | |
| Bear | ring A | requir | es a seat | plate. | | |
| | _ | | ed have fo | | s than | 375# |

Top chord: 2x4 SP #2 N; Bot chord: 2x6 SP #2 N;

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

Provide (2) 0.131"x3.0", min. toe-nails at top chord. Provide (2) 0.131"x3.0", min. toe-nails at bottom chord.



COA#0-278, Yoonhwak Kim, FL PE #86367 Fl09/04/002fficate of Product Approval #FL 1999

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For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

SEQN: 837606/ FROM: MTV

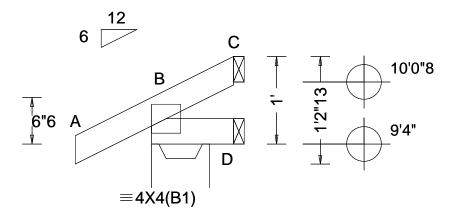
JACK

Ply: 1 Qty: 11 Job Number: 402471

,7F5 ,BRAA ,15;16;21;25 ,RB01 / BRADEN/A

Truss Label: CJ1

Cust: R 8975 JRef: 1YdO89750003 T31 / DrwNo: 267.25.0813.22056 KD / YK 09/24/2025





| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | DefI/CSI Criteria |
|------------------------|---|------------------------------|---------------------------------|
| TCLL: 20.00 | Wind Std: ASCE 7-22 | Pg: NA Ct: NA CAT: NA | PP Deflection in loc L/defl L/# |
| TCDL: 15.00 | Speed: 160 mph | Pf: NA Ce: NA | VERT(LL): NA |
| BCLL: 0.00 | Enclosure: Closed | Lu: NA Cs: NA | VERT(CL): NA |
| BCDL: 10.00 | Risk Category: II | Snow Duration: NA | HORZ(LL): -0.000 C |
| Des Ld: 45.00 | EXP: C Kzt: NA Mean Height: 15.00 ft | | HORZ(TL): 0.001 C |
| NCBCLL: 10.00 | TCDL: 4.2 psf | Building Code: | Creep Factor: 2.0 |
| Soffit: 0.00 | BCDL: 5.0 psf | FBC 8th Ed. 2023 Res. | Max TC CSI: 0.122 |
| Load Duration: 1.25 | MWFRS Parallel Dist: 0 to h/2 | TPI Std: 2014 | Max BC CSI: 0.013 |
| Spacing: 24.0 " | C&C Dist a: 3.00 ft | Rep Fac: Yes | Max Web CSI: 0.000 |
| | Loc. from endwall: Any | FT/RT:20(0)/10(0) | |
| | GCpi: 0.18 | Plate Type(s): | |
| | Wind Duration: 1.60 | WAVE | VIEW Ver: 23.02.01A.1204.18 |

| ▲ M | laxim | ım Rea | ections | (lbs) | | |
|-----|---------|----------|-----------|-------------|--------|------|
| | G | ravity | | No | on-Gra | vity |
| Loc | R+ | / R- | / Rh | / Rw | / U | / RL |
| В | 152 | /- | /- | /147 | /54 | /42 |
| D | 14 | /- | /- | /7 | /1 | /- |
| С | - | /-2 | /- | /20 | /21 | /- |
| Wir | nd read | ctions b | ased on | MWFRS | | |
| В | | | | Req = 1.5 | 5 | |
| D | Brg V | Vid = 1 | .5 Mir | Req = - | | |
| С | Brg V | Vid = 1 | .5 Mir | Req = - | | |
| Bea | aring E | is a rig | jid surfa | ce. | | |
| Bea | aring B | require | es a sea | t plate. | | |
| Mei | mbers | not list | ed have | forces less | s than | 375# |

Lumber

Top chord: 2x4 SP #2 N; Bot chord: 2x4 SP #2 N;

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

Provide (2) 0.131"x3.0", min. toe-nails at top chord. Provide (2) 0.131"x3.0", min. toe-nails at bottom chord.



COA#0-278, Yoonhwak Kim, FL PE #86367 Flood242026 icate of Product Approval #FL 1999

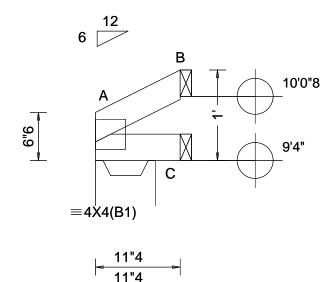
WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

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SEQN: 103934/ JACK Ply: 1 Job Number: 402471 Cust: R 8975 JRef: 1YdO89750003 T16 / FROM: ARZ ,7F5 ,BRAA ,15;16;21;25 ,RB01 / BRADEN/A Qty: 1 DrwNo: 267.25.0813.22400 Truss Label: CJ1A



| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | |
|------------------------|---|------------------------------|---------------------------------|----|
| TCLL: 20.00 | Wind Std: ASCE 7-22 | Pg: NA Ct: NA CAT: NA | PP Deflection in loc L/defl L/# | |
| TCDL: 15.00 | Speed: 160 mph | Pf: NA Ce: NA | VERT(LL): NA | ١. |
| BCLL: 0.00 | Enclosure: Closed | Lu: NA Cs: NA | VERT(CL): NA | ١. |
| BCDL: 10.00 | Risk Category: II | Snow Duration: NA | HORZ(LL): -0.000 B | , |
| Des Ld: 45.00 | EXP: C Kzt: NA Mean Height: 15.00 ft | | HORZ(TL): 0.000 B | ļ |
| NCBCLL: 10.00 | TCDL: 4.2 psf | Building Code: | Creep Factor: 2.0 | Ι, |
| Soffit: 0.00 | BCDL: 5.0 psf | FBC 8th Ed. 2023 Res. | Max TC CSI: 0.036 | 1 |
| Load Duration: 1.25 | MWFRS Parallel Dist: 0 to h/2 | TPI Std: 2014 | Max BC CSI: 0.009 | ľ |
| Spacing: 24.0 " | C&C Dist a: 3.00 ft | Rep Fac: Yes | Max Web CSI: 0.000 | Ιí |
| | Loc. from endwall: Any | FT/RT:20(0)/10(0) | | |
| | GCpi: 0.18 | Plate Type(s): | | l |
| | Wind Duration: 1.60 | WAVE | VIEW Ver: 23.02.01A.1204.18 | |

| | Grav | ity | Non-Gravity | | |
|--------|---------|----------------|----------------------------|-----|------|
| Loc I | R+ / F | R- / Rh | / Rw | / U | / RL |
| A 4 | B /- | /- /- /- | /33 /10 /25 MWFRS | /4 | /23 |
| C 1 | 7 /- | /- | /10 | /- | /- |
| B 30 | 0 /- | /- | /25 | /27 | /- |
| | | s based or | MWFRS | | |
| A B | rg Wid | = 8.0 Mir | Req = 2. | 0 | |
| СВ | rg Wid | = 1.5 Mir | Req = - | | |
| В В | rg Wid | = 1.5 Mir | Req = - | | |
| Rearin | na Aisa | a rigid surfa | ce. | | |

/ WHK

09/24/2025

Lumber

Top chord: 2x4 SP #2 N; Bot chord: 2x4 SP #2 N;

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

Provide (2) 0.131"x3.0", min. toe-nails at top chord. Provide (2) 0.131"x3.0", min. toe-nails at bottom chord.



COA#0-278, Yoonhwak Kim, FL PE #86367 Fl09/24/2026 Ficate of Product Approval #FL 1999

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SEQN: 103935/ FROM: ARZ

CJAC

Ply: 1 Qty: 3 Job Number: 402471

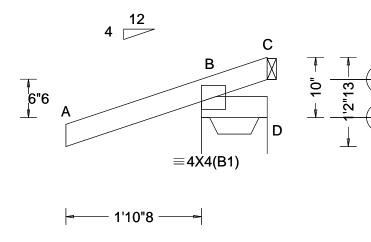
,7F5 ,BRAA ,15;16;21;25 ,RB01 / BRADEN/A

Truss Label: CJ1E

Cust: R 8975 JRef: 1YdO89750003 T34 / DrwNo: 267.25.0813.22352 / WHK 09/24/2025

13'10"5

13'4"



| L | 10"15 | _ |
|---|-------|---|
| Γ | 10"15 | |

| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | DefI/CSI Criteria |
|------------------------|--|------------------------------|---------------------------------|
| TCLL: 20.00 | Wind Std: ASCE 7-22 | Pg: NA Ct: NA CAT: NA | PP Deflection in loc L/defl L/# |
| TCDL: 15.00 | Speed: 160 mph | Pf: NA Ce: NA | VERT(LL): NA |
| BCLL: 0.00 | Enclosure: Closed | Lu: NA Cs: NA | VERT(CL): NA |
| BCDL: 10.00 | Risk Category: II | Snow Duration: NA | HORZ(LL): 0.001 B |
| Des Ld: 45.00 | EXP: C Kzt: NA | | HORZ(TL): 0.002 B |
| NCBCLL: 10.00 | Mean Height: 15.00 ft TCDL: 4.2 psf | Building Code: | Creep Factor: 2.0 |
| Soffit: 0.00 | BCDL: 5.0 psf | FBC 8th Ed. 2023 Res. | Max TC CSI: 0.642 |
| Load Duration: 1.25 | MWFRS Parallel Dist: 0 to h/2 | TPI Std: 2014 | Max BC CSI: 0.086 |
| Spacing: 24.0 " | C&C Dist a: 3.00 ft | Rep Fac: Yes | Max Web CSI: 0.000 |
| - | Loc. from endwall: Any | FT/RT:20(0)/10(0) | |
| | GCpi: 0.18 | Plate Type(s): | |
| | Wind Duration: 1.60 | WAVE | VIEW Ver: 23.02.01A.1204.18 |

▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ /Rh /Rw /U /RL 332 /-/298 /207 /-114 /-/104 /118 /-Wind reactions based on MWFRS Brg Wid = 10.9 Min Req = 2.0 Brg Wid = 1.5 Min Req = -Bearing B is a rigid surface. Bearing B requires a seat plate. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2 N; Bot chord: 2x4 SP #2 N;

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

Provide (2) 0.131"x3.0", min. toe-nails at top chord.



COA#0-278, Yoonhwak Kim, FL PE #86367 Fl09.024 Oct Ficate of Product Approval #FL 1999



SEQN: 103936/ CJAC Ply: 1 Job Number: 402471 Cust: R 8975 JRef: 1YdO89750003 T43 / FROM: ARZ Qty: 1 ,7F5 ,BRAA ,15;16;21;25 ,RB01 / BRADEN/A DrwNo: 267.25.0813.22401 Truss Label: CJ1F

> C 13'10"5 В 13'4" 12'11"3

> > 2'10"15

| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | |
|------------------------|---|------------------------------|---------------------------------|----|
| TCLL: 20.00 | Wind Std: ASCE 7-22 | Pg: NA Ct: NA CAT: NA | PP Deflection in loc L/defl L/# | |
| TCDL: 15.00 | Speed: 160 mph | Pf: NA Ce: NA | VERT(LL): -0.008 B 999 240 | |
| BCLL: 0.00 | Enclosure: Closed | Lu: NA Cs: NA | VERT(CL): 0.016 B 999 240 | |
| BCDL: 10.00 | Risk Category: II | Snow Duration: NA | HORZ(LL): -0.005 D | |
| Des Ld: 45.00 | EXP: C Kzt: NA Mean Height: 15.00 ft | | HORZ(TL): 0.009 D | |
| NCBCLL: 10.00 | TCDL: 4.2 psf | Building Code: | Creep Factor: 2.0 | |
| Soffit: 0.00 | BCDL: 5.0 psf | FBC 8th Ed. 2023 Res. | Max TC CSI: 0.425 | ŀ |
| Load Duration: 1.25 | MWFRS Parallel Dist: 0 to h/2 | TPI Std: 2014 | Max BC CSI: 0.010 | I, |
| Spacing: 24.0 " | C&C Dist a: 3.00 ft | Rep Fac: Yes | Max Web CSI: 0.000 | |
| | Loc. from endwall: Any | FT/RT:20(0)/10(0) | | |
| | GCpi: 0.18 | Plate Type(s): | | 1 |
| | Wind Duration: 1.60 | WAVE | VIEW Ver: 23.02.01A.1204.18 | |
| Lumber | | | | |

2'

| | ▲ M | laxim | um Rea | ictions (II | os) | | |
|---|-------------------------------|-------|----------|-------------|------------|--------|------|
| | | (| ravity | | No | on-Gra | vity |
| | Loc | R+ | / R- | / Rh | / Rw | / U | / RL |
| | Α | 103 | /- | /- | /56 | /50 | /49 |
| | D | 20 | /- | /- | /10 | /- | /- |
| | С | 105 | /- | /- | /61 | /68 | /- |
| | Win | d rea | ctions b | ased on N | /WFRS | | |
| | Α | Brg \ | Nid = 1 | .5 Min F | Req = - | | |
| | D | Brg \ | Nid = 3 | .2 Min F | Req = 1.5 | (Trus | s) |
| | С | Brg \ | Nid = 1 | .5 Min F | Req = - | - | • |
| | Bearing D is a rigid surface. | | | | | | |
| | Mer | nbers | not list | ed have fo | orces less | s than | 375# |
| _ | | | | | | | |

/ WHK

09/24/2025

Top chord: 2x4 SP #2 N; Bot chord: 2x4 SP #2 N;

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

Provide (2) 0.131"x3.0", min. toe-nails at top chord.



COA#0-278, Yoonhwak Kim, FL PE #86367 Floricate of Product Approval #FL 1999

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SEQN: 103937/ FROM: ARZ

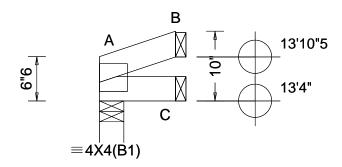
CJAC

Ply: 1 Qty: 1 Job Number: 402471

,7F5 ,BRAA ,15;16;21;25 ,RB01 / BRADEN/A

Truss Label: CJ1H

Cust: R 8975 JRef: 1YdO89750003 T13 / DrwNo: 267.25.0813.22997 / WHK 09/24/2025





| Loading Critoria (| Wind Criteria | Smarr Critoria (D. Di la DOC) | Defl/CSI Criteria |
|------------------------|--------------------------------|-------------------------------|---------------------------------|
| Loading Criteria (psf) | | Snow Criteria (Pg,Pf in PSF) | Defi/CSi Criteria |
| TCLL: 20.00 | Wind Std: ASCE 7-22 | Pg: NA Ct: NA CAT: NA | PP Deflection in loc L/defl L/# |
| TCDL: 15.00 | Speed: 160 mph | Pf: NA Ce: NA | VERT(LL): NA |
| BCLL: 0.00 | Enclosure: Closed | Lu: NA Cs: NA | VERT(CL): NA |
| BCDL: 10.00 | Risk Category: II | Snow Duration: NA | HORZ(LL): -0.000 A |
| Des Ld: 45.00 | EXP: C Kzt: NA | | HORZ(TL): 0.000 A |
| NCBCLL: 10.00 | Mean Height: 15.00 ft | Building Code: | Creep Factor: 2.0 |
| Soffit: 0.00 | TCDL: 4.2 psf BCDL: 5.0 psf | FBC 8th Ed. 2023 Res. | Max TC CSI: 0.031 |
| Load Duration: 1.25 | MWFRS Parallel Dist: 0 to h/2 | TPI Std: 2014 | Max BC CSI: 0.008 |
| Spacing: 24.0 " | C&C Dist a: 3.00 ft | Rep Fac: Yes | Max Web CSI: 0.000 |
| J | Loc. from endwall: Any | FT/RT:20(0)/10(0) | |
| | GCpi: 0.18 | Plate Type(s): | |
| | Wind Duration: 1.60 | WAVE | VIEW Ver: 23.02.01A.1204.18 |

▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ /Rh /Rw /U /RL 47 /-/29 /10 /16 С 16 /-/-/9 /-/-28 /16 /22 Wind reactions based on MWFRS Brg Wid = 3.5 Min Req = 2.0 Brg Wid = 1.5 Min Req = -Brg Wid = 1.5 Min Req = -Bearing A is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2 N; Bot chord: 2x4 SP #2 N;

Wind loads based on MWFRS with additional C&C

Wind loading based on both gable and hip roof types.

Additional Notes

Provide (2) 0.131"x3.0", min. toe-nails at top chord. Provide (2) 0.131"x3.0", min. toe-nails at bottom chord.



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SEQN: 103938/ FROM: ARZ

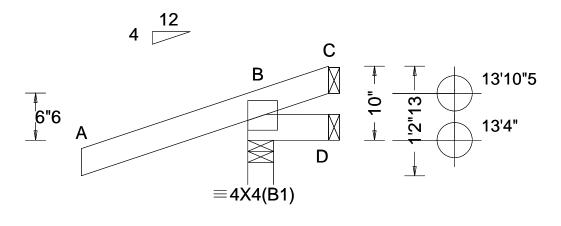
CJAC

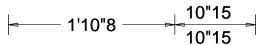
Ply: 1 Qty: 1 Job Number: 402471

,7F5 ,BRAA ,15;16;21;25 ,RB01 / BRADEN/A

Truss Label: CJ1C

Cust: R 8975 JRef: 1YdO89750003 T36 / DrwNo: 267.25.0813.23184 / WHK 09/24/2025





| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | 4 |
|---|---|--|---|--------------------------------------|
| TCLL: 20.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Std: ASCE 7-22 Speed: 160 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60 | Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE | PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.001 B HORZ(TL): 0.002 B Creep Factor: 2.0 Max TC CSI: 0.642 Max BC CSI: 0.090 Max Web CSI: 0.000 VIEW Ver: 23.02.01A.1204.18 | L E C C C C E E |

| | ▲ N | laximu | ım Read | tions (II | os) | | | |
|---|-----|---------|------------|-----------|------------|-------------|------|--|
| | | G | ravity | | No | Non-Gravity | | |
| | Loc | R+ | / R- | / Rh | / Rw | / U | / RL | |
| | В | 354 | /- | /- | /320 | /234 | /49 | |
| | D | 2 | /-22 | /- | /26 | /21 | /- | |
| | С | - | /-114 | /- | /105 | /119 | /- | |
| | Wir | nd read | tions ba | sed on N | /WFRS | | | |
| | В | Brg V | Vid = 3.5 | Min F | Req = 2.0 |) | | |
| | D | Brg V | Vid = 1.5 | Min F | Req = - | | | |
| | С | Brg V | Vid = 1.5 | Min F | Req = - | | | |
| | Bea | aring B | is a rigid | d surface | €. | | | |
| | Ме | mbers | not liste | d have fo | orces less | than 3 | 375# | |
| _ | | | | | | | | |

Top chord: 2x4 SP #2 N; Bot chord: 2x4 SP #2 N;

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

Provide (2) 0.131"x3.0", min. toe-nails at top chord. Provide (2) 0.131"x3.0", min. toe-nails at bottom chord.



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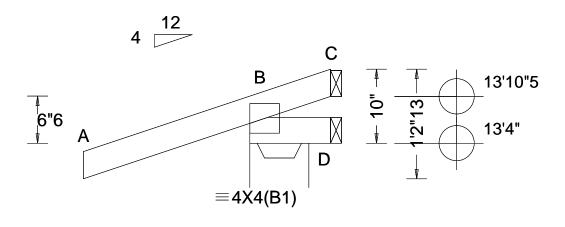
SEQN: 103939/ FROM: ARZ

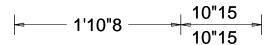
CJAC Ply: 1 Qty: 1

Job Number: 402471

,7F5 ,BRAA ,15;16;21;25 ,RB01 / BRADEN/A Truss Label: CJ1D

Cust: R 8975 JRef: 1YdO89750003 T17 / DrwNo: 267.25.0813.23089 / WHK 09/24/2025





| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria |
|------------------------|---|------------------------------|---------------------------------|
| TCLL: 20.00 | Wind Std: ASCE 7-22 | Pg: NA Ct: NA CAT: NA | PP Deflection in loc L/defl L/# |
| TCDL: 15.00 | Speed: 160 mph | Pf: NA Ce: NA | VERT(LL): NA |
| BCLL: 0.00 | Enclosure: Closed | Lu: NA Cs: NA | VERT(CL): NA |
| BCDL: 10.00 | Risk Category: II | Snow Duration: NA | HORZ(LL): 0.001 B |
| Des Ld: 45.00 | EXP: C Kzt: NA Mean Height: 15.00 ft | | HORZ(TL): 0.002 B |
| NCBCLL: 10.00 | TCDL: 4.2 psf | Building Code: | Creep Factor: 2.0 |
| Soffit: 0.00 | BCDL: 5.0 psf | FBC 8th Ed. 2023 Res. | Max TC CSI: 0.504 |
| Load Duration: 1.25 | MWFRS Parallel Dist: h/2 to h | TPI Std: 2014 | Max BC CSI: 0.073 |
| Spacing: 24.0 " | C&C Dist a: 3.00 ft | Rep Fac: Yes | Max Web CSI: 0.000 |
| | Loc. from endwall: not in 9.00 ft | FT/RT:20(0)/10(0) | |
| | GCpi: 0.18 | Plate Type(s): | |
| | Wind Duration: 1.60 | WAVE | VIEW Ver: 23.02.01A.1204.18 |
| Laurakan | | | |

| ▲ Max | cimu | n Rea | ctions | (lbs | s) | | |
|--------|-------|----------|--------|-------|---------|----------|------|
| | Gr | avity | | | No | on-Grav | vity |
| Loc I | ₹+ | / R- | / Rh | | /Rw | / U | / RL |
| В 3 | 54 | /- | /- | | /320 | /234 | /49 |
| D 2 | | /-22 | /- | | /26 | /21 | /- |
| С - | | /-114 | /- | | /105 | /119 | /- |
| Wind | react | ions ba | sed o | n M\ | NFRS | | |
| в в | rg W | id = 8.0 |) Mi | n Re | q = 2.0 |) | |
| D B | rg W | id = 1.5 | 5 Mi | n Re | eq = - | | |
| | | id = 1.5 | | | | | |
| Bearir | ng Bi | s a rigi | d surf | ace. | | | |
| Bearir | ng Bı | equire | s a se | at pl | ate. | | |
| Memb | ers r | ot liste | d hav | e for | ces les | s than 3 | 375# |

Lumber

Top chord: 2x4 SP #2 N; Bot chord: 2x4 SP #2 N;

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

Provide (2) 0.131"x3.0", min. toe-nails at top chord. Provide (2) 0.131"x3.0", min. toe-nails at bottom chord.

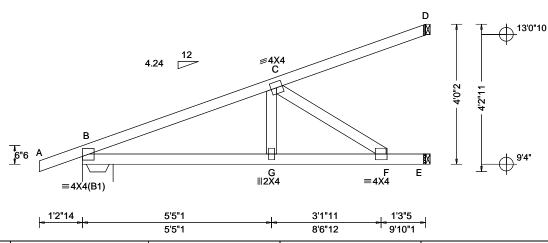


COA#0-278, Yoonhwak Kim, FL PE #86367 Floricate of Product Approval #FL 1999



SEQN: 837754/ HIP_ Ply: 1 Job Number: 402471 Cust: R 8975 JRef: 1YdO89750003 T33 / FROM: ARZ Qty: 5 ,7F5 ,BRAA ,15;16;21;25 ,RB01 / BRADEN/A DrwNo: 267.25.0813.22604 Truss Label: HJ7 / WHK 09/24/2025





| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria |
|------------------------|--|------------------------------|---------------------------------|
| TCLL: 20.00 | Wind Std: ASCE 7-22 | Pg: NA Ct: NA CAT: NA | PP Deflection in loc L/defl L/# |
| | Speed: 160 mph | Pf: NA Ce: NA | VERT(LL): 0.067 F 999 240 |
| DCLL. 0.00 | Enclosure: Closed | Lu: NA Cs: NA | VERT(CL): 0.156 F 749 240 |
| | Risk Category: II | Snow Duration: NA | HORZ(LL): 0.015 C |
| Dec d: 45.00 | EXP: C Kzt: NA | | HORZ(TL): 0.036 C |
| NCBCLL: 0.00 | Mean Height: 15.00 ft TCDL: 4.2 psf | Building Code: | Creep Factor: 2.0 |
| Soffit: 0.00 | BCDL: 5.0 psf | FBC 8th Ed. 2023 Res. | Max TC CSI: 0.706 |
| | MWFRS Parallel Dist: 0 to h/2 | TPI Std: 2014 | Max BC CSI: 0.782 |
| Spacing: 24.0 " | C&C Dist a: 3.00 ft | Rep Fac: No | Max Web CSI: 0.241 |
| | Loc. from endwall: NA | FT/RT:20(0)/10(0) | |
| | GCpi: 0.18 | Plate Type(s): | |
| | Wind Duration: 1.60 | WAVE | VIEW Ver: 23.02.01A.1204.18 |

Lumber

Top chord: 2x4 SP #2 N; Bot chord: 2x4 SP #1; Webs: 2x4 SP #3;

Loading

Hipjack supports 6-11-8 setback jacks with no webs.

Wind

Wind loads and reactions based on MWFRS. Wind loading based on both gable and hip roof types.

Provide (3) 0.131"x3.0", min. toe-nails at top chord. Provide (3) 0.162"x3.5" 16d Common nails, toe nail at bot chord

▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ /Rh /Rw /U /RL В 394 /-/-/150 /-432 /-/0 /97 /0 285 /166 /-/-Wind reactions based on MWFRS Brg Wid = 10.6 Min Req = 2.0 Brg Wid = 1.5 Min Req = Brg Wid = 1.5 Min Req = Bearing B is a rigid surface. Bearing B requires a seat plate. Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp.

296 - 787

Maximum Bot Chord Forces Per Ply (lbs)

| Chords | Tens.Comp. | Chords | Tens. Comp. | |
|--------|------------|--------|-------------|--|
| B-G | 714 - 265 | G-F | 703 - 266 | |

Maximum Web Forces Per Ply (lbs)

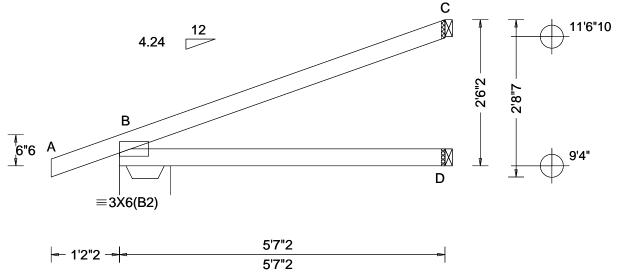
Webs Tens.Comp. 322 - 846



COA#0-278, Yoonhwak Kim, FL PE #86367 Flood 242026 icate of Product Approval #FL 1999



SEQN: 837549 / HIP_ Ply: 1 Job Number: 402471 Cust: R 8975 JRef: 1YdO89750003 T51 / FROM: ARZ Qty: 1 ,7F5 ,BRAA ,15;16;21;25 ,RB01 / BRADEN/A DrwNo: 267.25.0813.23575 Truss Label: HJ4 KD / YK 09/24/2025



Defl/CSI Criteria

| Loading Criteria (psi) | Willia Criteria | Show Chiteria (Pg,Phin PSF) | Deli/Col Cilleria |
|------------------------|---|-----------------------------|---------------------------------|
| TCLL: 20.00 | Wind Std: ASCE 7-22 | Pg: NA Ct: NA CAT: NA | PP Deflection in loc L/defl L/# |
| TCDL: 15.00 | Speed: 160 mph | Pf: NA Ce: NA | VERT(LL): NA |
| BCLL: 0.00 | Enclosure: Closed | Lu: NA Cs: NA | VERT(CL): NA |
| BCDL: 10.00 | Risk Category: II | Snow Duration: NA | HORZ(LL): 0.004 B |
| Des Ld: 45.00 | EXP: C Kzt: NA Mean Height: 15.00 ft | | HORZ(TL): 0.010 B |
| NCBCLL: 10.00 | TCDL: 4.2 psf | Building Code: | Creep Factor: 2.0 |
| Soffit: 0.00 | BCDL: 5.0 psf | FBC 8th Ed. 2023 Res. | Max TC CSI: 0.558 |
| Load Duration: 1.25 | MWFRS Parallel Dist: 0 to h/2 | TPI Std: 2014 | Max BC CSI: 0.378 |
| Spacing: 24.0 " | C&C Dist a: 3.00 ft | Rep Fac: Varies by Ld Case | Max Web CSI: 0.000 |
| | Loc. from endwall: Any | FT/RT:20(0)/10(0) | |
| | GCpi: 0.18 | Plate Type(s): | |
| | Wind Duration: 1.60 | WAVE | VIEW Ver: 23.02.01A.1204.18 |
| Lumber | | | |

Snow Criteria (Pa Pf in PSE)

| ▲ Ma | axim | um Rea | ctions (II | os) | | |
|------|--------|----------|-------------|-----------|----------|--------|
| | G | ravity | | No | on-Grav | vity . |
| Loc | R+ | / R- | / Rh | / Rw | / U | / RL |
| в : | 246 | /- | /- | /- | /90 | /- |
| D · | 107 | /- | /- | /6 | /- | /- |
| C · | 137 | /- | /- | /- | /109 | /- |
| Wind | d read | ctions b | ased on N | /WFRS | | |
| В | Brg V | Vid = 10 | 0.6 Min F | Req = 1.5 | (Trus | 3) |
| D | Brg V | Vid = 1. | 5 Min F | Req = - | - | - |
| | | | 5 Min F | | | |
| Bear | ring B | is a rig | id surface |).). | | |
| Bear | ring B | require | es a seat p | olate. | | |
| Mem | nhere | not list | ed have fo | arces les | s than ? | R75# |

Top chord: 2x4 SP #2 N; Bot chord: 2x4 SP #2 N;

Special Loads

---(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25) rom 2 plf at -1.24 t rom 2 plf at 0.00 t rom 2 plf at 0.00 t -2 lb Conc. Load at 1.37 TC: From -1.24 to 72 plf at 0.00 0.00 to 0.00 to 2 plf at 2 plf at 5.59 BC: From 5 59 TC: 182 lb Conc. Load at 4.20

27 lb Conc. Load at 1.37 BC: 111 lb Conc. Load at 4.20

Loading Criteria (nef) Wind Criteria

Wind

Wind loads and reactions based on MWFRS. Wind loading based on both gable and hip roof types.

Additional Notes

Provide (2) 0.131"x3.0", min. toe-nails at top chord. Provide (2) 0.131"x3.0", min. toe-nails at bottom chord.



COA#0-278, Yoonhwak Kim, FL PE #86367 Flood 242026 icate of Product Approval #FL 1999

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

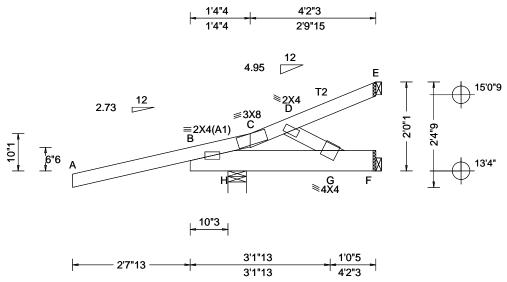
IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have continuous lateral restraint (CLR), installed with diagonal bracing installed on the CLR per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Detailis, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec. 2.

For more information see these web sites: Alpine: alpineitw.com: TPI: binst.org: SBCA: sbcacomponents.com: ICC: iccsafe.org: AWC: awc.org

For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

SEQN: 103944/ HIP_ Ply: 1 Job Number: 402471 Cust: R 8975 JRef: 1YdO89750003 T40 / FROM: RAA Qty: 1 ,7F5 ,BRAA ,15;16;21;25 ,RB01 / BRADEN/A DrwNo: 267.25.0813.22823 Truss Label: HJ3A / WHK 09/24/2025



| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria | 14 |
|--|--|--|---|----|
| TCLL: 20.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 0.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Std: ASCE 7-22 Speed: 160 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: NA GCpi: 0.18 | Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): | PP Deflection in loc L/defl L/# VERT(LL): 0.024 B 527 240 VERT(CL): 0.048 B 263 240 HORZ(LL): -0.009 E HORZ(TL): 0.018 E Creep Factor: 2.0 Max TC CSI: 0.351 Max BC CSI: 0.404 Max Web CSI: 0.080 | 1 |
| | Wind Duration: 1.60 | WAVE | VIEW Ver: 23.02.01A.1204.18 | |
| Lumber | | | | |

| A N | /laxim | um Read | ctions (II | os) | | |
|------------|---------|-----------|------------|------------|----------|------|
| | G | avity | | No | on-Grav | ∕ity |
| Loc | : R+ | / R- | / Rh | / Rw | / U | / RL |
| Н | 464 | /- | /- | /- | /243 | /- |
| F | - | /-145 | /0 | /91 | /- | /- |
| E | 43 | /- | /- | /- | /26 | /- |
| Wi | nd read | ctions ba | sed on N | /WFRS | | |
| | | | | Req = 2.0 |) | |
| F | Brg V | Vid = 1.5 | Min F | Req = - | | |
| E | Brg V | Vid = 1.5 | Min F | Req = - | | |
| | | | d surface | | | |
| Me | mbers | not liste | d have fo | orces less | s than 3 | 375# |

Top chord: 2x4 SP 2400f-2.0E; T2 2x4 SP #2 N; Bot chord: 2x6 SP #2 N; Webs: 2x4 SP #3;

Loading

The following trusses need concentrated loads at the end of their overhangs: 2-11-8 span/setback member on the 0-7-4 cant side requires 52 lbs and the 2-11-8 span/setback member on the 0-7-4 cant side requires

Sub-fascia beam assumptions: 4-10-0 sub-fascia beam on the 0-7-4 cantilever side. 4-10-0 sub-fascia beam on the 0-7-4 cantilever side.

Hipiack supports 2-11-8 setback jacks with 0-7-4 cantilever one face; 0-7-4 cantilever opposite face.

Wind loads and reactions based on MWFRS.

Left cantilever is exposed to wind

Wind loading based on both gable and hip roof types.

Additional Notes

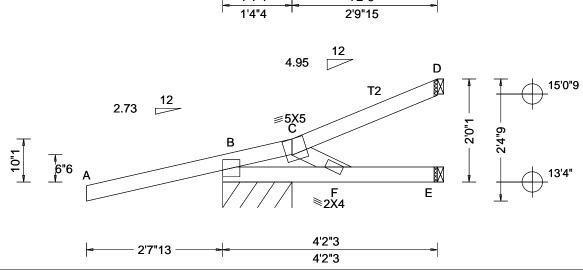
Provide (2) 0.131"x3.0", min. toe-nails at top chord. Provide (2) 0.131"x3.0", min. toe-nails at bottom chord.



COA#0-278, Yoonhwak Kim, FL PE #86367 Flood242026 icate of Product Approval #FL 1999



SEQN: 103945/ HIP_ Ply: 1 Job Number: 402471 Cust: R 8975 JRef: 1YdO89750003 T41 / FROM: ARZ ,7F5 ,BRAA ,15;16;21;25 ,RB01 / BRADEN/A DrwNo: 267.25.0813.22322 Qty: 1 Truss Label: HJ3B / WHK 09/24/2025



| | Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria |
|--|--|--|--|--|
| Wind Duration: 1.60 WAVE VIEW Ver: 23.02.01A.1204.18 | TCLL: 20.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 0.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Std: ASCE 7-22 Speed: 160 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: NA GCpi: 0.18 | Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): | PP Deflection in loc L/defl L/# VERT(LL): 0.002 F 999 240 VERT(CL): -0.004 F 999 240 HORZ(LL): -0.001 B HORZ(TL): 0.003 B Creep Factor: 2.0 Max TC CSI: 0.349 Max BC CSI: 0.075 Max Web CSI: 0.081 |

| | ▲ M | laxim | um Rea | actions (It | os), or *= | :PLF | |
|---|-----|---------|----------|-------------|--------------|----------|------|
| | | G | ravity | | No | on-Grav | vity |
| 0 | Loc | R+ | / R- | / Rh | / Rw | / U | / RL |
| 0 | В* | 216 | /- | /- | /- | /104 | /- |
| - | Е | 2 | /- | /- | /16 | /- | /- |
| - | D | 67 | /- | /- | | /42 | |
| | С | | /-105 | | | | |
| | Wir | nd read | ctions b | ased on N | IWFRS | | |
| | В | | | 6.2 Min F | | | |
| | Е | Brg V | Vid = 1 | .5 Min F | ?eq = - | | |
| | D | Brg V | Vid = 1 | .5 Min F | ?eq = - | | |
| | Bea | aring B | is a rig | jid surface |). | | |
| | Bea | ring B | require | es a seat p | olate. | | |
| | Mei | mbers | not list | ed have fo | rces les | s than 3 | 375# |

Top chord: 2x4 SP 2400f-2.0E; T2 2x4 SP #2 N; Bot chord: 2x4 SP #2 N; Webs: 2x4 SP #3;

Plating Notes

All plates are 4X4(B1) except as noted.

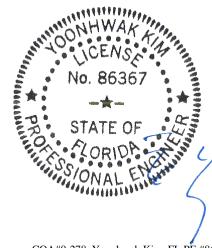
Loading

Hipjack supports 2-11-8 setback jacks with no webs.

Wind loads and reactions based on MWFRS. Wind loading based on both gable and hip roof types.

Additional Notes

Provide (2) 0.131"x3.0", min. toe-nails at top chord. Provide (2) 0.131"x3.0", min. toe-nails at bottom chord.



COA#0-278, Yoonhwak Kim, FL PE #86367 Floricate of Product Approval #FL 1999



SEQN: 103941/ FROM: ARZ

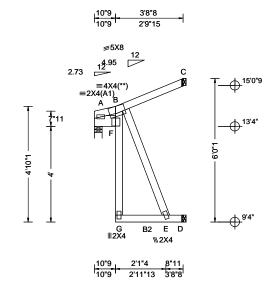
HIP_

Ply: 1 Qty: 1 Job Number: 402471

,7F5 ,BRAA ,15;16;21;25 ,RB01 / BRADEN/A

Truss Label: HJ3C

Cust: R 8975 JRef: 1YdO89750003 T45 / DrwNo: 267.25.0813.22057 / WHK 09/24/2025



| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | DefI/CSI Criteria | Γ. |
|--|--|---|--|----|
| TCLL: 20.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 0.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 " | Wind Std: ASCE 7-22 Speed: 160 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: NA GCpi: 0.18 Wind Duration: 1.60 | Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE | PP Deflection in loc L/defl L/# VERT(LL): 0.003 F 999 240 VERT(CL): 0.007 F 999 240 HORZ(LL): -0.005 E HORZ(TL): 0.007 E Creep Factor: 2.0 Max TC CSI: 0.104 Max BC CSI: 0.047 Max Web CSI: 0.120 VIEW Ver: 23.02.01A.1204.18 | |
| Lumber | | | | |

| ▲ M | axim | ım Rea | ctions (II | os) | | |
|-----|--------|-----------|------------|-----------|--------|------|
| | G | ravity | - | No | on-Gra | vity |
| Loc | R+ | / R- | / Rh | / Rw | / U | / RL |
| A | 49 | /- | /- | /- | /19 | /- |
| D | 27 | /- | /- | /4 | /- | /- |
| С | 64 | /- | /- | /- | /39 | /- |
| Win | d read | ctions b | ased on N | /WFRS | | |
| Α | Brg V | Vid = 3. | 8 Min F | Req = 2.0 |) | |
| D | Brg V | Vid = 1. | 5 Min F | Reg = - | | |
| С | Brg V | Vid = 1. | 5 Min F | Req = - | | |
| Bea | ring A | is a rig | id surface |). | | |
| Mer | nbers | not liste | ed have fo | orces les | s than | 375# |
| | | | | | | |

Top chord: 2x4 SP #2 N; Bot chord: 2x6 SP #2 N; B2 2x4 SP #2 N; Webs: 2x4 SP #3;

Plating Notes

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Loading

Hipjack supports 2-7-8 setback jacks with no webs.

Wind loads and reactions based on MWFRS. Wind loading based on both gable and hip roof types.

Additional Notes

Provide (2) 0.131"x3.0", min. toe-nails at top chord. Provide (2) 0.131"x3.0", min. toe-nails at bottom chord.



COA#0-278, Yoonhwak Kim, FL PE #86367 Flondad Centricate of Product Approval #FL 1999

SEQN: 103946/ FROM: ARZ

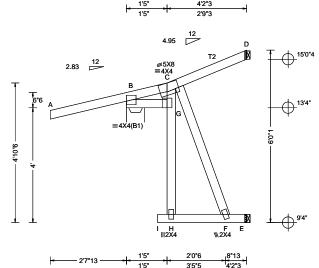
COMN Ply: 1 Qty: 1

Job Number: 402471

,7F5 ,BRAA ,15;16;21;25 ,RB01 / BRADEN/A

Truss Label: HJ3

Cust: R 8975 JRef: 1YdO89750003 T11 / DrwNo: 267.25.0813.22745 / WHK 09/24/2025



| Loading Criteria (psf) | Wind Criteria | Snow Criteria (Pg,Pf in PSF) | Defl/CSI Criteria |
|------------------------|---|------------------------------|---------------------------------|
| TCLL: 20.00 | Wind Std: ASCE 7-22 | Pg: NA Ct: NA CAT: NA | PP Deflection in loc L/defl L/# |
| TCDL: 15.00 | Speed: 160 mph | Pf: NA Ce: NA | VERT(LL): -0.012 I 999 240 |
| BCLL: 0.00 | Enclosure: Closed | Lu: NA Cs: NA | VERT(CL): -0.025 I 999 240 |
| BCDL: 10.00 | Risk Category: II | Snow Duration: NA | HORZ(LL): -0.021 F |
| Des Ld: 45.00 | EXP: C Kzt: NA Mean Height: 15.00 ft | | HORZ(TL): 0.043 F |
| NCBCLL: 0.00 | TCDL: 4.2 psf | Building Code: | Creep Factor: 2.0 |
| Soffit: 0.00 | BCDL: 5.0 psf | FBC 8th Ed. 2023 Res. | Max TC CSI: 0.349 |
| Load Duration: 1.25 | MWFRS Parallel Dist: 0 to h/2 | TPI Std: 2014 | Max BC CSI: 0.075 |
| Spacing: 24.0 " | C&C Dist a: 3.00 ft | Rep Fac: No | Max Web CSI: 0.348 |
| | Loc. from endwall: NA | FT/RT:20(0)/10(0) | |
| | GCpi: 0.18 | Plate Type(s): | |
| | Wind Duration: 1.60 | WAVE | VIEW Ver: 23.02.01A.1204.18 |
| Lumber | • | - | |

| ▲ M | laxim | um Rea | ctions (II | • | | |
|-----|---------|-----------|-------------|-----------|----------|------|
| | G | avity | | No | on-Grav | vity |
| Loc | R+ | / R- | / Rh | / Rw | / U | / RL |
| В | 325 | /- | /- | /- | /164 | /- |
| Е | - | /-12 | /- | /29 | /- | /- |
| D | 49 | /- | /- | /- | /30 | /- |
| Win | d rea | ctions b | ased on N | /WFRS | | |
| В | Brg V | Vid = 7. | 6 Min F | Req = 2.0 |) | |
| Е | Brg V | Vid = 1. | 5 Min F | . = eq | | |
| | | | 5 Min F | | | |
| Bea | ıring E | is a rig | id surface |). | | |
| Bea | ıring E | require | es a seat i | olate. | | |
| Mer | nhers | not liste | ed have fo | rces les | s than 3 | 375# |

Top chord: 2x4 SP 2400f-2.0E; T2 2x4 SP #2 N; Bot chord: 2x4 SP #2 N; Webs: 2x4 SP #3;

Loading

Hipjack supports 2-11-8 setback jacks with no webs.

Wind

Wind loads and reactions based on MWFRS. Wind loading based on both gable and hip roof types.

Additional Notes

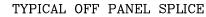
Provide (2) 0.131"x3.0", min. toe-nails at top chord. Provide (2) 0.131"x3.0", min. toe-nails at bottom chord.

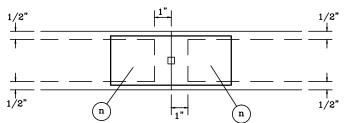


COA#0-278, Yoonhwak Kim, FL PE #86367 Floricate of Product Approval #FL 1999



TRULOX INFORMATION DETAIL

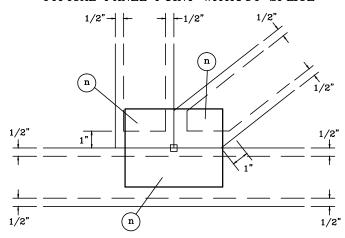




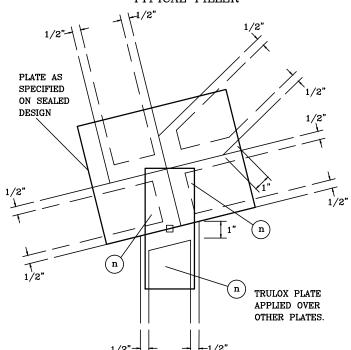
DO NOT APPLY NAILS WITHIN 1/2" OF LUMBER EDGES OR 1" OF LUMBER ENDS ON EACH FACE, AS SHOWN BY DASHED LINES.

NAILS MUST NOT SPLIT LUMBER.

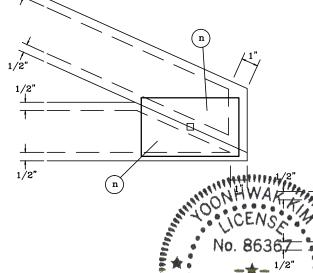
TYPICAL PANEL POINT WITHOUT SPLICE



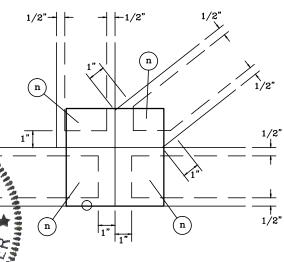




TYPICAL HEEL



TYPICAL PANEL POINT SPLICE



NOTES:

- (n) IS THE REQUIRED NUMBER OF 0.120" X 1.375" NAILS, OR EQUAL, PER FACE PER PLY AS SPECIFIED ON THE SEALED DESIGN REFERENCING THIS DETAIL.
- O LOCATES PLATE CORNER OR FLUSH EDGE.
- ☐ LOCATES PLATE CENTER.



TRULOX PLATING

PAGE 1 OF 1 DATE 10/01/14

09/24/2025 8, Yoonhwak Kim, FL PE #86367

155 Harlem Ave North Building, 4th Floor Glenview, IL 60025

Florida Certificate of Product Approval #FL 1999

ASCE 7-10: 140 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Dr: 120 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00

Dr: 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 1.00

Or: 100 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure D, Kzt = 1,00

| | | 2×4 | | | | * 100 mpm | | | Los o . " | | | | 1 | |
|---------------------|----------------|----------------|----------|---------------|---------------|----------------|-----------------|----------------|-----------------|----------------|----------------|-----------------|-----------|----------------|
| | | zx4 Vertica | Brace | No | (1) 1×4 *L | " RLOCE * | (1) 2x4 'L | ." Brace * | (2) 2×4 L | . Rudce ** | (1) 5×6 L | " Brace * | (5) 5×6 L | Brace ** |
| | Spacing | Species | Grade | - | Group A | Group B | Group A | Group B | Group A | Group B | Group A | Group B | Group A | Group B |
| † | | CDE | #1 / #2 | 4′ 3″ | 7′ 3″ | 7' 7" | 8′ 7 ″ | 8′ 11″ | 10′ 3″ | 10′ 8 ″ | 13′ 6″ | 14' 0" | 14' 0" | 14′ 0″ |
| | 1.7 | SPF | #3 | 4′ 1″ | 6′ 7 ″ | 7′ 1″ | 8′ 6 ″ | 8′ 10 ″ | 10′ 1″ | 10′ 6 ″ | 13′ 4″ | 13′ 10 ″ | 14′ 0″ | 14′ 0″ |
| D | Ų | HF | Stud | 4′ 1″ | 6′ 7″ | 7′ 0 ″ | 8′ 6 ″ | 8′ 10 ″ | 10′ 1″ | 10′ 6″ | 13′ 4″ | 13′ 10 ″ | 14′ 0″ | 14′ 0 ″ |
| Ϊ́ | 0 | | Standard | 4′ 1″ | 5′ 8 ″ | 6′ 0 ″ | 7' 7" | 8′ 1 ″ | 10′ 1″ | 10′ 6″ | 11′ 10″ | 12′ 8 ″ | 14′ 0″ | 14′ 0″ |
| ا به اا | | 0.0 | #1 | 4′ 6″ | 7′ 4″ | 7′ 8 ″ | 8′ 8 ″ | 9′ 0″ | 10′ 4″ | 10′ 9 ″ | 13′ 8 ″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| \parallel \perp | * | SP | #2 | 4′ 3″ | 7′ 3″ | 7′ 7″ | 8′ 7 ″ | 8′ 11″ | 10′ 3″ | 10′ 8 ″ | 13′ 6 ″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| | 4 | l | #3 | 4′ 2″ | 6′ 0 ″ | 6′ 4″ | 7′ 11″ | 8′ 6 ″ | 10′ 2″ | 10′ 7″ | 12′ 5 ″ | 13′ 4″ | 14′ 0″ | 14′ 0″ |
| | N | IDFL | Stud | 4′ 2″ | 6′ 0″ | 6′ 4″ | 7′ 11″ | 8′ 6 ″ | 10′ 2″ | 10′ 7″ | 12′ 5 ″ | 13′ 4″ | 14′ 0″ | 14′ 0″ |
| č | . – | | Standard | 4′ 0″ | 5′ 3″ | 5′ 7 ″ | 7′ 0 ″ | 7′ 6″ | 9′ 6″ | 10′ 2″ | 11′ 0″ | 11′ 10″ | 14' 0" | 14′ 0″ |
| I .∪ I | | CDE | #1 / #2 | 4′ 11″ | 8′ 4″ | 8′ 8 ″ | 9′ 10″ | 10′ 3 ″ | 11′ 8″ | 12′ 2 ′ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| ;; | <u>-</u> | SPF | #3 | 4′ 8″ | 8′ 1″ | 8′ 8 ″ | 9′ 8″ | 10′ 1″ | 11′ 7″ | 12′ 1″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| ; | U | HF | Stud | 4′ 8″ | 8′ 1 ″ | 8′ 6 ″ | 9′ 8 ″ | 10′ 1″ | 11′ 7″ | 12′ 1″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| Ιà | Ō | 1 11 | Standard | 4′ 8″ | 6′ 11″ | 7′ 5 ′ | 9′ 3 ″ | 9′ 11″ | 11′ 7″ | 12′ 1″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| ~ | | | #1 | 5′ 1 ″ | 8′ 5 ″ | 8′ 9 ″ | 9′ 11″ | 10′ 4″ | 11′ 10″ | 12′ 4″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| > | * | SP | #2 | 4′ 11″ | 8′ 4″ | 8′ 8 ″ | 9′ 10″ | 10′ 3″ | 11′ 8″ | 12′ 2 ″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| | Ý. | l | #3 | 4′ 9″ | 7′ 4″ | 7′ 9″ | 9′ 9″ | 10′ 2″ | 11′ 8″ | 12′ 1″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| lω | 16 | IDFL | Stud | 4′ 9″ | 7′ 4″ | 7′ 9 ′ | 9′ 9″ | 10′ 2″ | 11′ 8″ | 12′ 1″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| | | | Standard | 4′ 8″ | 6′ 5″ | 6′ 10 ″ | 8′ 7 ″ | 9′ 2 ′ | 11′ 7″ | 12′ 1″ | 13′ 6″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| Q | | SPF | #1 / #2 | 5′ 5 ″ | 9′ 2″ | 9′ 6″ | 10′ 10 ″ | 11′ 3″ | 11′ 8″ | 13′ 5 ″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| | . . | 2LL | #3 | 5′ 1 ′ | 9′ 0″ | 9′ 4″ | 10′ 8″ | 11′ 1″ | 12′ 9″ | 13′ 3″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| 0 | \cup | HF | Stud | 5′ 1 ′ | 9′ 0″ | 9′ 4″ | 10′ 8″ | 11′ 1″ | 12′ 9″ | 13′ 3″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| | Ö | | Standard | 5′ 1 ″ | 8′ 0″ | 8′ 6 ″ | 10′ 8″ | 11′ 1″ | 12′ 9″ | 13′ 3″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| $ \times $ | | | #1 | 5′ 8 ″ | 9′ 3″ | 9′ 8″ | 10′ 11″ | 11′ 4″ | 13′ 0″ | 13′ 6″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| d | * | SP | #2 | 5′ 5″ | 9′ 2″ | 9′ 6″ | 10′ 10″ | 11′ 3″ | 12′ 11″ | 13′ 5″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| ĺΣ | ù | L. | #3 | 5′ 3 ″ | 8′ 5 ″ | 9′ 0″ | 10′ 9″ | 11′ 2″ | 12′ 10 ″ | 13′ 4″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| | 10 | IDFL | Stud | 5′ 3 ″ | 8′ 5 ″ | 9′ 0″ | 10′ 9″ | 11′ 2″ | 12′ 10 ″ | 13′ 4″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| | | | Standard | 5′ 1 ″ | 7′ 5″ | 7′ 11″ | 9' 11" | 10′ 7″ | 12′ 9 ″ | 13′ 3″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |

| Bracing Group Speci | es and Grades: | | | | | | | | | | |
|------------------------------------|--------------------------------------|--|--|--|--|--|--|--|--|--|--|
| Group A: | | | | | | | | | | | |
| Spruce-Pine-Fir Hem-Fir | | | | | | | | | | | |
| #1 / #2 Standard | #2 Stud | | | | | | | | | | |
| #3 Stud | #3 Standard | | | | | | | | | | |
| Douglas Fir-Larch #3 Stud Standard | Southern Pine*** #3 Stud Standard | | | | | | | | | | |
| Group | В | | | | | | | | | | |
| Hem-Fi | r | | | | | | | | | | |
| #1 & B | tr | | | | | | | | | | |
| #1 | | | | | | | | | | | |
| Davides Fig. Length | Sauthana Blaswww | | | | | | | | | | |
| Douglas Fir-Larch | Southern Pine*** | | | | | | | | | | |
| #2 | #2 | | | | | | | | | | |

1x4 Braces shall be SRB (Stress-Rated Board) ***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards, Group B values may be used with these grades.

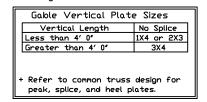
Gable Truss Detail Notes: Wind Load deflection criterion is L/240.

Provide uplift connections for 55 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0' outlookers with 2'0" overhang, or 12" plywood overhang.

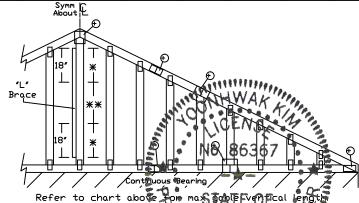
Attach "L" braces with 10d (0.128"x3.0" min) nails. * For (1) "L" brace: space nails at 2" o.c. in 18" end zones and 4" o.c. between zones. ₩ ¥For (2) "L" braces: space nails at 3" o.c. in 18" end zones and 6" o.c. between zones.

"L" bracing must be a minimum of 80% of web member length.



Refer to the Building Designer for conditions not addressed by this detail.

Gable Truss Diagonal brace option: vertical length may be doubled when diagonal brace is used. Connect diagonal brace for 450# at each end. Max web total length is 14'. 2x4 DF-L #2 or better diagonal brace; single Vertical length shown or double cut in table above. (as shown) at upper end. Connect diagonal at midpoint of vertical web.



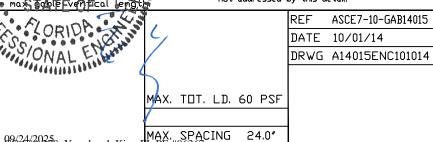
VARNINGI READ AND FOLLOW ALL NOTES ON THIS DRAWINGI ****IMPORTANT*** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and macing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI unless noted otherwise, top chord shall have properly attached structural sheathing and botton chord shall have a properly attached rigid celling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. Applicable to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions.

Alpine, a division of ITV Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.

A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see this job's general notes page and these web sites: ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org



Florida Certificate of Product Approval #FL 1999

ASCE 7-16: 140 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Dr: 120 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00

Dr: 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 1.00

Or: 100 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure D. Kzt = 1.00

| | | 2x4 Vertica | Brace | No | (1) 1×4 *L | Brace * | (1) 2×4 *L | " Brace * | (2) 2×4 L | " Brace ** | (1) 2×6 ' L | " Brace * | (2) 2x6 *L | Brace ** |
|---|----------|----------------|----------|---------------|---------------|----------------|---------------|----------------|-----------------|----------------|--------------------|-----------------|------------|----------------|
| ا ے اا | Spacing | Species | Grade | _ | Group A | Group B | Group A | Group B | Group A | Group B | Group A | Group B | Group A | Group B |
| 亡 | | CDE | #1 / #2 | 4′ 3″ | 7′ 3″ | 7' 7" | 8′ 7 ″ | 8′ 11 ″ | 10′ 3″ | 10′ 8 ″ | 13′ 6″ | 14' 0" | 14' 0" | 14′ 0″ |
| | 1.1 | SPF | #3 | 4′ 1 ″ | 6′ 7 ″ | 7′ 1″ | 8′ 6 ″ | 8′ 10 ″ | 10′ 1″ | 10′ 6 ″ | 13′ 4″ | 13′ 10 ″ | 14′ 0″ | 14′ 0″ |
| 21 | Ų | HF | Stud | 4′ 1″ | 6′ 7 ″ | 7′ 0 ″ | 8′ 6 ″ | 8′ 10 ″ | 10′ 1″ | 10′ 6″ | 13′ 4″ | 13′ 10 ″ | 14′ 0″ | 14′ 0 ″ |
| > | Ō | 1 11 | Standard | 4′ 1″ | 5′ 8 ″ | 6′ 0 ″ | 7′ 7″ | 8′ 1 ″ | 10′ 1″ | 10′ 6″ | 11′ 10″ | 12′ 8″ | 14′ 0″ | 14′ 0″ |
| ا به اا | | | #1 | 4′ 6″ | 7′ 4″ | 7′ 8 ″ | 8′ 8 ″ | 9′ 0″ | 10′ 4″ | 10′ 9 ′ | 13′ 8″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| | * | SP | #2 | 4′ 3″ | 7′ 3″ | 7′ 7″ | 8′ 7 ″ | 8′ 11 ″ | 10′ 3″ | 10′ 8 ″ | 13′ 6″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| | 4 | l | #3 | 4′ 2″ | 6′ 0 ″ | 6′ 4″ | 7′ 11″ | 8′ 6″ | 10′ 2″ | 10′ 7″ | 12′ 5″ | 13′ 4″ | 14′ 0″ | 14′ 0″ |
| | Ω | IDFLI | Stud | 4′ 2″ | 6′ 0″ | 6′ 4″ | 7′ 11″ | 8′ 6″ | 10′ 2″ | 10′ 7″ | 12′ 5 ″ | 13′ 4″ | 14′ 0″ | 14′ 0″ |
| II | | | Standard | 4′ 0″ | 5′ 3″ | 5′ 7 ″ | 7′ 0 ″ | 7′ 6 ″ | 9′ 6″ | 10′ 2 ′ | 11′ 0″ | 11′ 10″ | 14' 0" | 14′ 0″ |
| II <u>.</u> | | SPF | #1 / #2 | 4′ 11″ | 8′ 4″ | 8′ 8 ″ | 9′ 10″ | 10′ 3″ | 11′ 8″ | 12′ 2″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| 🚎 | | 766 | #3 | 4′ 8″ | 8′ 1″ | 8′ 8″ | 9′ 8″ | 10′ 1″ | 11′ 7″ | 12′ 1″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| `_ | \cup | HF | Stud | 4′ 8″ | 8′ 1″ | 8′ 6 ″ | 9′ 8″ | 10′ 1″ | 11′ 7″ | 12′ 1″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| ΠġΙ | Ō | 1 11 | Standard | 4′ 8 ″ | 6′ 11″ | 7′ 5 ′ | 9′ 3 ″ | 9′ 11″ | 11′ 7″ | 12′ 1″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| \parallel $\stackrel{\sim}{\smile}$ \parallel | | | #1 | 5′ 1 ″ | 8′ 5″ | 8′ 9 ″ | 9′ 11″ | 10′ 4″ | 11′ 10″ | 12′ 4″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| / | | SP | #2 | 4′ 11″ | 8′ 4″ | 8′ 8 ″ | 9′ 10″ | 10′ 3″ | 11′ 8″ | 12′ 2 ″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| | è | L | #3 | 4′ 9″ | 7′ 4″ | 7′ 9″ | 9′ 9″ | 10′ 2″ | 11′ 8″ | 12′ 1″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| ושו | Ţ | IDFLI | Stud | 4′ 9 ″ | 7′ 4″ | 7′ 9 ″ | 9′ 9″ | 10′ 2 ″ | 11′ 8″ | 12′ 1″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| Ⅱ≒▮ | | | Standard | 4′ 8″ | 6′ 5 ″ | 6′ 10 ″ | 8′ 7 ″ | 9′ 2 ′ | 11′ 7″ | 12′ 1″ | 13′ 6″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| abl | | SPF | #1 / #2 | 5′ 5 ″ | 9′ 2″ | 9′ 6″ | 10′ 10″ | 11′ 3″ | 11′ 8″ | 13′ 5 ′ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| 0 | | 766 | #3 | 5′ 1 ″ | 9′ 0″ | 9′ 4″ | 10′ 8″ | 11′ 1″ | 12′ 9″ | 13′ 3″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| 0 | Ū | HF | Stud | 5′ 1 ″ | 9′ 0″ | 9′ 4″ | 10′ 8″ | 11′ 1″ | 12′ 9″ | 13′ 3″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| | Ō | 1 11 | Standard | 5′ 1 ″ | 8′ 0 ″ | 8′ 6″ | 10′ 8″ | 11′ 1″ | 12′ 9″ | 13′ 3″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| $ \times $ | 0 | | #1 | 5′ 8 ″ | 9′ 3″ | 9′ 8″ | 10′ 11″ | 11′ 4″ | 13′ 0″ | 13′ 6″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| | * | SP | #2 | 5′ 5″ | 9′ 2″ | 9′ 6″ | 10′ 10″ | 11′ 3″ | 12′ 11″ | 13′ 5 ′ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| M A | ù | ₅ , | #3 | 5′ 3 ″ | 8′ 5 ″ | 9′ 0″ | 10′ 9″ | 11′ 2″ | 12′ 10 ″ | 13′ 4″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| | 1, | DFL | Stud | 5′ 3 ″ | 8′ 5 ″ | 9′ 0″ | 10′ 9″ | 11′ 2″ | 12′ 10 ″ | 13′ 4″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| | | | Standard | 5′ 1 ″ | 7′ 5″ | 7′ 11″ | 9′ 11″ | 10′ 7″ | 12′ 9″ | 13′ 3″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |

| Bracing Group Spe | cies and Grades: | | | | | | | | | |
|---------------------------------|------------------------|--|--|--|--|--|--|--|--|--|
| Group A: | | | | | | | | | | |
| Spruce-Pine-Fir | Hem-Fir | | | | | | | | | |
| #1 / #2 Standard | #2 Stud | | | | | | | | | |
| #3 Stud | #3 Standard | | | | | | | | | |
| Douglas Fir-Larch | Southern Pine*** | | | | | | | | | |
| #3 Stud | #3 Stud | | | | | | | | | |
| Standard | Standard | | | | | | | | | |
| Staridar a | S caridar a | | | | | | | | | |
| Group B: Hen-Fir #1 & Btr #1 | | | | | | | | | | |
| Douglas Fir-Larch #1 #2 | Southern Pine*** #1 #2 | | | | | | | | | |

1x4 Braces shall be SRB (Stress-Rated Board). ***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards, Group B values may be used with these grades.

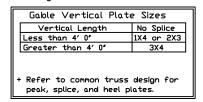
Gable Truss Detail Notes: Wind Load deflection criterion is L/240.

Provide uplift connections for 55 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0' outlookers with 2' 0" overhang, or 12" plywood overhang.

Attach "L" braces with 10d (0.128"x3.0" min) nails. * For (1) "L" brace: space nails at 2" o.c. in 18" end zones and 4" o.c. between zones. ₩ ¥For (2) "L" braces: space nails at 3" o.c. in 18" end zones and 6" o.c. between zones.

"L" bracing must be a minimum of 80% of web member length.



Refer to the Building Designer for conditions not addressed by this detail.

Gable Truss Diagonal brace option: vertical length may be doubled when diagonal brace is used. Connect diagonal brace for 450# at each end. Max web "L" Brace End total length is 14'. Zones, typ. 2x4 DF-L #2 or better diagonal brace; single Vertical length shown or double cut in table above. (as shown) at upper end. Connect diagonal at Refer to chart above for most midpoint of vertical web.

VARNINGI READ AND FOLLOW ALL NOTES ON THIS DRAWINGI ****IMPORTANT*** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Sullding Corponent Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and botton chord shall have a properly attached rigid celling. Cocations shown for pernament lateral restraint of webs shall have bracing installed per BCSI sections 83, 87 or Bio, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 1604-2 for standard plate positions. Refer to drawings 160A-Z for standard plate positions.

Alpine, a division of ITV Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.

A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see this Job's general notes page and these web sites: ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org



Florida Certificate of Product Approval #FL 1999

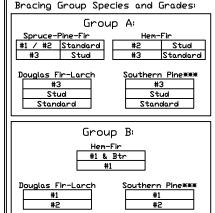
ASCE 7-10: 140 mph Wind Speed, 30' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Dr: 120 mph Wind Speed, 30' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00

Dr: 120 mph Wind Speed, 30' Mean Height, Enclosed, Exposure D, Kzt = 1.00

Or: 100 mph wind speed, 30' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

| | | | | | | • | · · | | | ar clarity Li | | | 1 | |
|-----------------------------|----------|------------------|----------|---------------|----------------|----------------|----------------|---------------|----------------|----------------|----------------|----------------|-----------|----------|
| | | 2x4 · Vertica | Brace | No | (1) 1×4 "L | " Brace * | (1) 2×4 *L | ." Brace * | (2) 2×4 L | " Brace ** | (1) 2x6 *L | * Brace * | (5) 5×6 L | Brace ** |
| \parallel $_{-}\parallel$ | Spacing | Species | Grade | _ | Group A | Group B | Group A | Group B | Group A | Group B | Group A | Group B | Group A | Group B |
| 무 | | CDE | #1 / #2 | 4′ 1″ | 6′ 11″ | 7′ 2″ | 8′ 2 ″ | 8′ 6″ | 9′ 9″ | 10′ 2″ | 12′ 10″ | 13′ 4″ | 14′ 0″ | 14′ 0″ |
| | | SPF | #3 | 3′ 10″ | 6′ 2″ | 6′ 7″ | 8′ 1″ | 8′ 5 ″ | 9′ 8″ | 10′ 0″ | 12′ 8″ | 13′ 2″ | 14′ 0″ | 14′ 0″ |
| ဤ | Ų | HF | Stud | 3′ 10″ | 6′ 2″ | 6′ 6 ″ | 8′ 1 ″ | 8′ 5″ | 9′ 8″ | 10′ 0″ | 12′ 8 ″ | 13′ 2″ | 14′ 0″ | 14′ 0″ |
| Č | 0 | | Standard | 3′ 10″ | 5′ 3 ″ | 5′ 7 ″ | 7′ 0 ″ | 7′ 6″ | 9′ 6″ | 10′ 0″ | 11′ 0″ | 11′ 10″ | 14′ 0″ | 14′ 0″ |
| 0. | | 00 | #1 | 4′ 2″ | 7′ 0″ | 7′ 3″ | 8′ 3″ | 8′ 7″ | 9′ 10″ | 10′ 3″ | 13′ 0″ | 13′ 6″ | 14′ 0″ | 14′ 0″ |
| $ \perp $ | | SP | #2 | 4′ 1″ | 6′ 11″ | 7′ 2″ | 8′ 2 ″ | 8′ 6″ | 9′ 9″ | 10′ 2″ | 12′ 10″ | 13′ 4″ | 14′ 0″ | 14′ 0″ |
| | 4 | l | #3 | 4′ 0″ | 5′ 7″ | 5′ 11″ | 7′ 5 ″ | 7′ 11″ | 9′ 8″ | 10′ 1″ | 11′ 7″ | 12′ 5″ | 14′ 0″ | 14′ 0″ |
| | Ω | IDFL | Stud | 4′ 0″ | 5′ 7″ | 5′ 11″ | 7′ 5″ | 7′ 11″ | 9′ 8″ | 10′ 1″ | 11′ 7″ | 12′ 5 ″ | 14′ 0″ | 14′ 0″ |
| 전 | | | Standard | 3′ 9″ | 4′ 11″ | 5′ 13 ″ | 6′ 6″ | 7′ 0″ | 8′ 10 ″ | 9′ 6″ | 10′ 3″ | 11′ 0″ | 13′ 11″ | 14′ 0″ |
| II <u>.</u> | | lene | #1 / #2 | 4′ 8″ | 7′ 11″ | 8′ 3 ″ | 9′ 4″ | 9′ 9″ | 11′ 2″ | 11′ 7″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| = | - | SPF | #3 | 4′ 5″ | 7′ 6″ | 8′ 3″ | 9′ 3″ | 9′ 7″ | 11′ 0″ | 11′ 6″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| | \cup | HF | Stud | 4′ 5 ″ | 7′ 6″ | 8′ 0 ″ | 9′ 3″ | 9′ 7″ | 11′ 0″ | 11′ 6″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| | Ō | 1 11 | Standard | 4′ 5 ″ | 6′ 5″ | 6′ 10″ | 8′ 7 ″ | 9′ 2″ | 11′ 0″ | 11′ 6″ | 13′ 6″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| \parallel | 0 | | #1 | 4′ 10″ | 8′ 0 ″ | 8′ 4″ | 9′ 6″ | 9′ 10″ | 11′ 3″ | 11′ 9″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| /- | * | SP | #2 | 4′ 8″ | 7′ 11″ | 8′ 3″ | 9′ 4″ | 9′ 9″ | 11′ 2″ | 11′ 7″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| | è | l | #3 | 4′ 7″ | 6′ 10 ″ | 7′ 3″ | 9′ 1″ | 9′ 8″ | 11′ 1″ | 11′ 6″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| ושו | Ţ | IDFL | Stud | 4′ 7″ | 6′ 10 ″ | 7′ 3″ | 9′ 1″ | 9′ 8″ | 11′ 1″ | 11′ 6″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| d | | | Standard | 4′ 5 ″ | 6′ 0 ″ | 6′ 5 ″ | 8′ 0″ | 8′ 7″ | 10′ 10″ | 11′ 6″ | 12′ 7″ | 13′ 15″ | 14′ 0″ | 14′ 0″ |
| 2 | | SPF | #1 / #2 | 5′ 2 ″ | 8′ 9″ | 9′ 1″ | 10′ 4″ | 10′ 9″ | 11′ 2″ | 12′ 9″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| | | | #3 | 4′ 10″ | 8′ 7 ″ | 8′ 11″ | 10′ 2″ | 10′ 7″ | 12′ 2″ | 12′ 8 ″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| 0 | Ų | HF | Stud | 4′ 10″ | 8′ 7″ | 8′ 11″ | 10′ 2″ | 10′ 7″ | 12′ 2″ | 12′ 8″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| | Ō | 1 11 | Standard | 4′ 10″ | 7′ 5″ | 7′ 11″ | 9′ 11″ | 10′ 7″ | 12′ 2″ | 12′ 8 ″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| $ \times $ | 0 | | #1 | 5′ 4 ″ | 8′ 10 ″ | 9′ 2″ | 10′ 5 ″ | 10′ 10″ | 12′ 5″ | 12′ 11″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| | * | SP | #2 | 5′ 2 ″ | 8′ 9″ | 9′ 1″ | 10′ 4″ | 10′ 9″ | 12′ 3″ | 12′ 9″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| M Q | ù | $ \nabla C $ | #3 | 5′ 0 ″ | 7′ 10″ | 8′ 4″ | 10′ 3″ | 10′ 8″ | 12′ 2″ | 12′ 8 ″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| | 10 | DFL | Stud | 5′ 0″ | 7′ 10″ | 8′ 4″ | 10′ 3″ | 10′ 8″ | 12′ 2″ | 12′ 8 ″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| | | | Standard | 4′ 10″ | 6′ 11″ | 7′ 4″ | 9′ 3″ | 9′ 10″ | 12′ 2″ | 12′ 8″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |



1x4 Braces shall be SRB (Stress-Rated Board) **For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards, Group B values may be used with these grades.

Gable Truss Detail Notes: Wind Load deflection criterion is L/240.

Provide uplift connections for 100 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.

Attach "L" braces with 10d (0.128"x3.0" min) nails. ★ For (1) "L" brace: space nalls at 2" o.c. in 18" end zones and 4" o.c. between zones. ₩ **For (2) "L" braces: space nails at 3" o.c. in 18" end zones and 6" o.c. between zones.

"L" bracing must be a minimum of 80% of web member length.

| Gable Vertical Plate | Sizes |
|---|-----------|
| Vertical Length | No Splice |
| Less than 4' 0" | 2X4 |
| Greater than 4' 0", but less than 11' 6" | 3X4 |
| Greater than 11' 6" | 4X4 |
| + Refer to common truss | |

Refer to the Building Designer for conditions not addressed by this detail.

Gable Truss Diagonal brace option: vertical length may be doubled when diagonal brace is used. Connect diagonal brace for 525# at each end. Max web total length is 14'. 2x6 DF-L #2 or better diagonal brace; single Vertical length shown or double cut in table above. (as shown) at upper end. Connect diagonal at

Symm C "L" Brace Refer to chart abova

VARNINGI READ AND FOLLOW ALL NOTES ON THIS DRAWING ***IMPORTANT*** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

Trusses require extreme care in fabricating, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety information, by FPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and botton chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise.

Refer to drawings 160A-Z for standard plate positions.

Alpine, a division of ITV Building Components Group Inc. shall not be responsible for any deviation from this drawing, any fallure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.

A seal on this drawing or cover page listing this drawing, indicates acceptance of professional

engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see this job's general notes page and these web sites: ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org



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midpoint of vertical web.

ASCE 7-16: 140 mph Wind Speed, 30' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Dr: 120 mph Wind Speed, 30' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00

Dr: 120 mph Wind Speed, 30' Mean Height, Enclosed, Exposure D, Kzt = 1.00

Or: 100 mph wind speed, 30' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

| | | 2x4 Vertica | Brace | No | (1) 1×4 *L | Brace * | (1) 2×4 *L | " Brace * | (2) 2×4 L | " Brace ** | (1) 2×6 *L | * Brace * | (2) 2×6 *L | Brace ** |
|-----------------|----------|----------------|----------|---------------|----------------|----------------|----------------|---------------|----------------|----------------|----------------|----------------|------------|----------|
| اعاا | Spacing | Species | Grade | _ | Group A | Group B | Group A | Group B | Group A | Group B | Group A | Group B | Group A | Group B |
| | | CDE | #1 / #2 | 4′ 1″ | 6′ 11″ | 7′ 2″ | 8′ 2 ″ | 8′ 6″ | 9′ 9″ | 10′ 2″ | 12′ 10″ | 13′ 4″ | 14′ 0″ | 14′ 0″ |
| | 1.7 | SPF | #3 | 3′ 10″ | 6′ 2″ | 6′ 7″ | 8′ 1″ | 8′ 5 ″ | 9′ 8″ | 10′ 0″ | 12′ 8 ″ | 13′ 2″ | 14′ 0″ | 14′ 0″ |
| 21 | Ų | HF | Stud | 3′ 10″ | 6′ 2″ | 6′ 6 ″ | 8′ 1″ | 8′ 5 ″ | 9′ 8″ | 10′ 0″ | 12′ 8″ | 13′ 2″ | 14′ 0″ | 14′ 0″ |
| | Ō | 1 11 | Standard | 3′ 10″ | 5′ 3″ | 5′ 7 ″ | 7′ 0″ | 7′ 6″ | 9′ 6″ | 10′ 0″ | 11′ 0″ | 11′ 10″ | 14′ 0″ | 14′ 0″ |
| 0. | | 0.0 | #1 | 4′ 2″ | 7′ 0″ | 7′ 3″ | 8′ 3″ | 8′ 7 ″ | 9′ 10″ | 10′ 3″ | 13′ 0″ | 13′ 6″ | 14′ 0″ | 14′ 0″ |
| $ \perp $ | * | SP | #2 | 4′ 1″ | 6′ 11″ | 7′ 2 ″ | 8′ 2 ″ | 8′ 6 ″ | 9′ 9″ | 10′ 2″ | 12′ 10″ | 13′ 4″ | 14′ 0″ | 14′ 0″ |
| | 4 | l | #3 | 4′ 0″ | 5′ 7″ | 5′ 11″ | 7′ 5 ″ | 7′ 11″ | 9′ 8″ | 10′ 1″ | 11′ 7″ | 12′ 5″ | 14′ 0″ | 14′ 0″ |
| | Ω | IDFL | Stud | 4′ 0″ | 5′ 7 ″ | 5′ 11″ | 7′ 5″ | 7′ 11″ | 9′ 8″ | 10′ 1″ | 11′ 7″ | 12′ 5 ″ | 14′ 0″ | 14′ 0″ |
| 설 | | | Standard | 3′ 9″ | 4′ 11″ | 5′ 13 ″ | 6′ 6″ | 7′ 0″ | 8′ 10 ″ | 9′ 6″ | 10′ 3″ | 11′ 0″ | 13′ 11″ | 14′ 0″ |
| II <u>.</u> U I | | | #1 / #2 | 4′ 8″ | 7′ 11″ | 8′ 3 ″ | 9′ 4″ | 9′ 9″ | 11′ 2″ | 11′ 7″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| = | | SPF | #3 | 4′ 5″ | 7′ 6″ | 8′ 3″ | 9′ 3″ | 9′ 7″ | 11′ 0″ | 11′ 6″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| | \cup | HF | Stud | 4′ 5 ″ | 7′ 6″ | 8′ 0 ″ | 9′ 3″ | 9′ 7″ | 11′ 0″ | 11′ 6″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| & | ō | 1 11 | Standard | 4′ 5 ″ | 6′ 5 ″ | 6′ 10 ″ | 8′ 7 ″ | 9′ 2″ | 11′ 0″ | 11′ 6″ | 13′ 6″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| \parallel | | | #1 | 4′ 10″ | 8′ 0 ″ | 8′ 4″ | 9′ 6″ | 9′ 10″ | 11′ 3″ | 11′ 9″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| / | | SP | #2 | 4′ 8″ | 7′ 11″ | 8′ 3″ | 9′ 4″ | 9′ 9″ | 11′ 2″ | 11′ 7″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| | è | Inc. | #3 | 4′ 7″ | 6′ 10″ | 7′ 3″ | 9′ 1″ | 9′ 8″ | 11′ 1″ | 11′ 6″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| ושו | Ţ | IDFLI | Stud | 4′ 7″ | 6′ 10 ″ | 7′ 3″ | 9′ 1″ | 9′ 8″ | 11′ 1″ | 11′ 6″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| | | | Standard | 4′ 5 ″ | 6′ 0 ″ | 6′ 5 ″ | 8′ 0 ″ | 8′ 7″ | 10′ 10″ | 11′ 6″ | 12′ 7″ | 13′ 15″ | 14′ 0″ | 14′ 0″ |
| ld a | | SPF | #1 / #2 | 5′ 2 ″ | 8′ 9″ | 9′ 1″ | 10′ 4″ | 10′ 9″ | 11′ 2″ | 12′ 9 ″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| Ø | | 766 | #3 | 4′ 10″ | 8′ 7 ″ | 8′ 11″ | 10′ 2″ | 10′ 7″ | 12′ 2″ | 12′ 8″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| 0 | Ū | HF | Stud | 4′ 10″ | 8′ 7″ | 8′ 11″ | 10′ 2″ | 10′ 7″ | 12′ 2″ | 12′ 8″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| | Ō | 1 11 | Standard | 4′ 10″ | 7′ 5″ | 7′ 11″ | 9′ 11″ | 10′ 7″ | 12′ 2″ | 12′ 8″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| $ \times $ | 0 | | #1 | 5′ 4″ | 8′ 10″ | 9′ 2″ | 10′ 5 ″ | 10′ 10″ | 12′ 5″ | 12′ 11″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| ll al | * | SP | #2 | 5′ 2 ″ | 8′ 9″ | 9′ 1″ | 10′ 4″ | 10′ 9″ | 12′ 3″ | 12′ 9″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| M M | ù | | #3 | 5′ 0 ″ | 7′ 10″ | 8′ 4″ | 10′ 3″ | 10′ 8″ | 12′ 2″ | 12′ 8″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| | Ţ | DFL | Stud | 5′ 0 ″ | 7′ 10″ | 8′ 4″ | 10′ 3″ | 10′ 8″ | 12′ 2″ | 12′ 8″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| | | | Standard | 4′ 10″ | 6′ 11″ | 7′ 4″ | 9′ 3″ | 9′ 10″ | 12′ 2″ | 12′ 8″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |

| Bracing (| Group Spe | cies and | Grades | | | | | | | |
|---------------------------------|-----------|----------|---------------------------------|--|--|--|--|--|--|--|
| Group A: | | | | | | | | | | |
| Spruce-Pine-Fir Hem-Fir | | | | | | | | | | |
| #1 / #2 | Standard | #2 | Stud | | | | | | | |
| #3 | Stud | #3 | Standard | | | | | | | |
| Douglas F #3 Stu Stand | ad . | S- | n Pine*** †3 tud ndard | | | | | | | |
| Douglas F | | Souther | n Pine*** | | | | | | | |

1x4 Braces shall be SRB (Stress-Rated Board). ***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards, Group B values may be used with these grades.

Gable Truss Detail Notes: Wind Load deflection criterion is L/240.

Provide uplift connections for 100 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.

Attach "L" braces with 10d (0.128"x3.0" min) nails. ★ For (1) "L" brace: space nalls at 2" o.c. in 18" end zones and 4" o.c. between zones. ₩ **For (2) "L" braces: space nails at 3" o.c. in 18" end zones and 6" o.c. between zones.

"L" bracing must be a minimum of 80% of web member length.

| Gable Vertical Plate | Sizes |
|--|-----------|
| Vertical Length | No Splice |
| Less than 4' 0" | 2X4 |
| Greater than 4' 0", but less than 11' 6" | 3X4 |
| Greater than 11' 6" | 4X4 |
| + Refer to common truss peak, splice, and heel pl | |

Refer to the Building Designer for conditions not addressed by this detail.

Diagonal brace option: vertical length may be doubled when diagonal brace is used. Connect diagonal brace for 525# at each end. Max web "L" Brace End total length is 14'. Zones, typ. 2x6 DF-L #2 or better diagonal brace; single Vertical length shown or double cut in table above. (as shown) at upper end. Connect diagonal at Refer to chart about

Gable Truss

VARNINGI READ AND FOLLOW ALL NOTES ON THIS DRAWING ***IMPORTANT*** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and mracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by FPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid celling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise.

Refer to drawings 160A-Z for standard plate positions.

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For more information see this job's general notes page and these web sites: ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org



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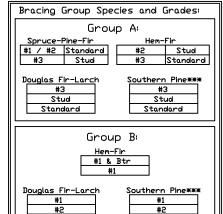
midpoint of vertical web.

ASCE 7-10: 160 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Dr: 140 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00

Dr: 140 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 1.00

| | | | | | | : 120 mph | Wind Spee | d, 15' Mea | n Height, F | artially Ei | nclosed, Ex | kposure D, | Kzt = 1.00 |) |
|------------|------|----------------|----------|----------------|---------------|----------------|----------------|---------------|----------------|----------------|----------------|------------|------------|----------|
| | | 2x4 Vertica | Brace | No | (1) 1×4 "L | Brace * | (1) 2×4 "L | " Brace * | (2) 2×4 *L | " Brace ** | (1) 2×6 *L | " Brace * | (2) 2x6 L | "Brace * |
| _ | 1 | Species | Grade | _ | Group A | Group B | Group A | Group B | Group A | Group B | Group A | Group B | Group A | Group B |
| 字 | | CDE | #1 / #2 | 3′ 10 ″ | 6′ 7″ | 6′ 10 ″ | 7′ 9 ″ | 8′ 1 ″ | 9′ 3″ | 9′ 7″ | 12′ 2 ″ | 12′ 8″ | 14′ 0″ | 14′ 0″ |
| | | SPF | #3 | 3′ 8″ | 5′ 9″ | 6′ 2″ | 7′ 8 ″ | 7′ 11″ | 9′ 1″ | 9′ 6″ | 12′ 0″ | 12′ 6″ | 14′ 0″ | 14′ 0″ |
| ВN | l Ō | HF | Stud | 3′ 8″ | 5′ 9″ | 6′ 1″ | 7′ 8 ″ | 7′ 11″ | 9′ 1″ | 9′ 6″ | 12′ 0 ″ | 12′ 6″ | 14′ 0″ | 14′ 0″ |
| | 10 | | Standard | 3′ 8″ | 4′ 11″ | 5′ 3 ″ | 6′ 7 ″ | 7′ 1″ | 8′ 11 ″ | 9′ 6″ | 10′ 4″ | 11′ 1″ | 14′ 0″ | 14′ 0″ |
| به | | | #1 | 4′0″ | 6′ 8″ | 6′ 11 ″ | 7′ 10″ | 8′ 2″ | 9′ 4″ | 9′ 8″ | 12′ 4″ | 12′ 9″ | 14′ 0″ | 14' 0" |
| | * | ISP I | #2 | 3′ 10 ″ | 6′ 7″ | 6′ 10 ″ | 7′ 9 ″ | 8′ 1″ | 9′ 3″ | 9′ 7″ | 12′ 2″ | 12′ 8″ | 14′ 0″ | 14′ 0″ |
| | 4 | | #3 | 3′ 9″ | 5′ 3 ″ | 5′ 7 ″ | 6′ 11″ | 7′ 5″ | 9′ 2″ | 9′ 7″ | 10′ 11″ | 11′ 8″ | 14′ 0″ | 14′ 0″ |
| 7 | N | IDFLI | Stud | 3′ 9″ | 5′ 3″ | 5′ 7″ | 6′ 11″ | 7′ 5″ | 9′ 2″ | 9′ 7″ | 10′ 11″ | 11′ 8″ | 14′ 0″ | 14′ 0″ |
| | | . | Standard | 3′ 6″ | 4′ 7″ | 4′ 11″ | 6′ 2 ″ | 6′ 7″ | 8′ 4 ″ | 8′ 11″ | 9′ 8″ | 10′ 4″ | 13′ 1″ | 14′ 0″ |
| ţ | | С Г | #1 / #2 | 4′ 5 ″ | 7′ 6″ | 7′ 9″ | 8′ 10 ″ | 9′ 3″ | 10′ 7″ | 11′ 0″ | 13′ 11″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| +> | - | SPF | #3 | 4′ 2″ | 7′ 1″ | 7′ 9″ | 8′ 9 ″ | 9′ 1″ | 10′ 5 ″ | 10′ 10″ | 13′ 9″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| Ċ | U | HF | Stud | 4′ 2″ | 7′ 1″ | 7′ 6″ | 8′ 9 ″ | 9′ 1″ | 10′ 5 ″ | 10′ 10″ | 13′ 9″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| à | ا م | 1 11 | Standard | 4′ 2″ | 6′ 1″ | 6′ 5″ | 8′ 1″ | 8′ 8″ | 10′ 5 ″ | 10′ 10″ | 12′ 8″ | 13′ 7″ | 14' 0" | 14′ 0″ |
| Ψ, | | | #1 | 4′ 7″ | 7′ 7″ | 7′ 11″ | 9′ 0″ | 9′ 4″ | 10′ 8″ | 11′ 1″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| > | | ISP I | #2 | 4′ 5″ | 7′ 6″ | 7′ 9″ | 8′ 10 ″ | 9′ 3″ | 10′ 7″ | 11′ 0″ | 13′ 11″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| | 9 | | #3 | 4′ 4″ | 6′ 5 ″ | 6′ 10 ″ | 8′ 6 ″ | 9′ 1″ | 10′ 6 ″ | 10′ 11″ | 13′ 4″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| Φ | 1,6 | DFLI | Stud | 4′ 4″ | 6′ 5 ″ | 6′ 10 ″ | 8′ 6 ″ | 9′ 1″ | 10′ 6″ | 10′ 11″ | 13′ 4″ | 14′ 0″ | 14' 0" | 14′ 0″ |
| _ | , , | | Standard | 4′ 2″ | 5′ 8″ | 6′ 0 ″ | 7′ 6″ | 8′ 0 ″ | 10′ 2″ | 10′ 10″ | 11′ 10″ | 12′ 7″ | 14′ 0″ | 14′ 0″ |
| Gαb | | CDL | #1 / #2 | 4′ 10 ″ | 8′ 3″ | 8′ 7″ | 9′ 9″ | 10′ 2″ | 10′ 7″ | 12′ 1″ | 14′ 0″ | 14′ 0″ | 14' 0" | 14′ 0″ |
| Ø | - | SPF | #3 | 4′ 7″ | 8′ 2″ | 8′ 5 ″ | 9′8″ | 10′ 0″ | 11′ 6″ | 12′ 0 ″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| 9 | U | HF | Stud | 4′ 7″ | 8′ 2″ | 8′ 5″ | 9′ 8″ | 10′ 0″ | 11′ 6″ | 12′ 0 ″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| | ا م | | Standard | 4′ 7″ | 7′ 0″ | 7′ 5″ | 9′ 4″ | 10′ 0″ | 11′ 6″ | 12′ 0 ″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| X | | | #1 | 5′ 1 ″ | 8′ 5 ″ | 8′ 8″ | 9′ 11″ | 10′ 3″ | 11′ 9″ | 12′ 3″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| ما X | | SP | #2 | 4′ 10 ″ | 8′ 3″ | 8′ 7 ″ | 9′ 9″ | 10′ 2″ | 11′ 7″ | 12′ 1″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| $i \sim 1$ | ا مُ | - ' | #3 | 4′ 9″ | 7′ 4″ | 7′ 10 ″ | 9′8 ″ | 10′ 1″ | 11′ 7″ | 12′ 0 ″ | 14′ 0″ | 14′ 0″ | 14' 0" | 14' 0" |



1x4 Braces shall be SRB (Stress-Rated Board) **For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards, Group B values may be used with these grades.

Gable Truss Detail Notes: Wind Load deflection criterion is L/240.

Provide uplift connections for 75 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0' overhang, or 12' plywood overhang.

Attach "L" braces with 10d (0.128"x3.0" min) nails. ★ For (1) "L" brace: space nails at 2" o.c. in 18" end zones and 4" o.c. between zones. ₩₩For (2) "L" braces: space nails at 3" o.c. in 18" end zones and 6" o.c. between zones.

"L" bracing must be a minimum of 80% of web member length.

| Gable Vertical Plate | e Sizes |
|---|-----------|
| Vertical Length | No Splice |
| Less than 4' 0" | 2X3 |
| Greater than 4' 0", but less than 11' 6" | 3X4 |
| Greater than 11' 6" | 4X4 |
| + Refer to common truss | |

Refer to the Building Designer for conditions not addressed by this detail.

Gable Truss Diagonal brace option: vertical length may be doubled when diagonal brace is used. Connect diagonal brace for 600# at each end. Max web total length is 14'. 2x6 DF-L #2 or better diagonal brace; single Vertical length shown or double cut in table above. (as shown) at upper end. Connect diagonal at midpoint of vertical web.

Stud

Standard

4' 9″

4′ 7″

7' 4"

6' 6"

7' 10"

6' 11"

9' 8"

8' 8"

10' 1"

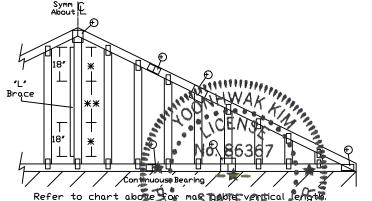
9' 3"

11' 7"

11' 6"

12' 0"

12' 0"



14' 0"

13′ 74

14' 0"

14' 0"

14' 0"

14' 0"

14' 0"

14' 0"

VARNINGI READ AND FOLLOW ALL NOTES ON THIS DRAWING ***IMPORTANT*** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

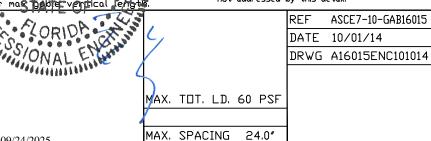
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Refer to drawings 160A-Z for standard plate positions.

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For more information see this job's general notes page and these web sites: ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org



09/24/20258, Yoonhwak Kim, FL PE # Florida Certificate of Product Approval #FL 1999

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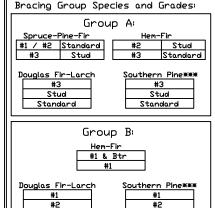
ASCE 7-16: 160 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Dr: 140 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00

Dr: 140 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 1.00

Or: 120 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

| | | 2×4 | Brace | NI- | (1) 1×4 *L | Brace * | (1) 2×4 *L | " Brace * | (2) 2×4 *L | * Brace ** | (1) 2×6 L | * Brace * | (2) 2x6 *L | " Brace * | * |
|--|---------|--------------------|----------|----------------|---------------|----------------|----------------|---------------|----------------|----------------|----------------|-----------|------------|-----------|---|
| \parallel $_{-}$ | Spacing | Vertica Species | Grade | No Braces | Group A | Group B | Group A | Group B | Group A | Group B | Group A | Group B | Group A | Group B | |
| 구 | | CDE | #1 / #2 | 3′ 10 ″ | 6′ 7″ | 6′ 10 ″ | 7′ 9″ | 8′ 1″ | 9′ 3″ | 9′ 7″ | 12′ 2″ | 12′ 8″ | 14′ 0″ | 14′ 0″ |] |
| | 1 | SPF | #3 | 3′ 8″ | 5′ 9 ″ | 6′ 2″ | 7′ 8 ″ | 7′ 11″ | 9′ 1″ | 9′ 6″ | 12′ 0″ | 12′ 6″ | 14′ 0″ | 14′ 0″ |] |
| D | ļΨ | HF | Stud | 3′ 8″ | 5′ 9″ | 6′ 1″ | 7′ 8″ | 7′ 11″ | 9′ 1″ | 9′ 6″ | 12′ 0 ″ | 12′ 6″ | 14′ 0″ | 14′ 0″ |] |
| \ | ΙŌ | 1 11 | Standard | 3′ 8″ | 4′ 11″ | 5′ 3 ″ | 6′ 7 ″ | 7′ 1″ | 8′ 11 ″ | 9′ 6″ | 10′ 4″ | 11′ 1″ | 14′ 0″ | 14′ 0″ | ⅃ |
| ΙΙΨ | | 0.0 | #1 | 4′ 0″ | 6′ 8″ | 6′ 11″ | 7′ 10″ | 8′ 2 ″ | 9′ 4″ | 9′ 8″ | 12′ 4″ | 12′ 9″ | 14′ 0″ | 14′ 0″ |] |
| | * | SP | #2 | 3′ 10 ″ | 6′ 7″ | 6′ 10 ″ | 7′ 9″ | 8′ 1″ | 9′ 3″ | 9′ 7″ | 12′ 2″ | 12′ 8″ | 14′ 0″ | 14′ 0″ | |
| | 4 | l | #3 | 3′ 9″ | 5′ 3″ | 5′ 7 ″ | 6′ 11 ″ | 7′ 5″ | 9′ 2″ | 9′ 7″ | 10′ 11″ | 11′ 8″ | 14′ 0″ | 14′ 0″ | ╛ |
| | N | IDFL | Stud | 3′ 9″ | 5′ 3″ | 5′ 7″ | 6′ 11″ | 7′ 5″ | 9′ 2″ | 9′ 7″ | 10′ 11″ | 11′ 8″ | 14′ 0″ | 14′ 0″ | ╛ |
| č | | | Standard | 3′ 6″ | 4′ 7″ | 4′ 11″ | 6′ 2 ″ | 6′ 7″ | 8′ 4″ | 8′ 11″ | 9′ 8″ | 10′ 4″ | 13′ 1″ | 14′ 0″ | ╛ |
| <u> </u> | | SPF | #1 / #2 | 4′ 5″ | 7′ 6″ | 7′ 9″ | 8′ 10 ″ | 9′ 3″ | 10′ 7″ | 11′ 0″ | 13′ 11″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | ╛ |
| === | l | 12LL | #3 | 4′ 2″ | 7′ 1″ | 7′ 9″ | 8′ 9 ″ | 9′ 1″ | 10′ 5″ | 10′ 10″ | 13′ 9″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | ╛ |
| <u> </u> | O O | HF | Stud | 4′ 2″ | 7′ 1″ | 7′ 6″ | 8′ 9 ″ | 9′ 1″ | 10′ 5″ | 10′ 10″ | 13′ 9″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | ╛ |
| Πā | ا آ | 1 11 | Standard | 4′ 2″ | 6′ 1″ | 6′ 5 ″ | 8′ 1″ | 8′ 8 ″ | 10′ 5″ | 10′ 10″ | 12′ 8″ | 13′ 7″ | 14′ 0″ | 14′ 0″ | ╛ |
| $\mathbb{N}^{\mathbb{Z}}$ | | | #1 | 4′ 7″ | 7′ 7″ | 7′ 11″ | 9′ 0″ | 9′ 4″ | 10′ 8″ | 11′ 1″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | ╛ |
| > | | SP | #2 | 4′ 5″ | 7′ 6″ | 7′ 9″ | 8′ 10″ | 9′ 3″ | 10′ 7″ | 11′ 0″ | 13′ 11″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | _ |
| |) | l | #3 | 4′ 4″ | 6′ 5″ | 6′ 10 ″ | 8′ 6 ″ | 9′ 1″ | 10′ 6″ | 10′ 11″ | 13′ 4″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | ╛ |
| IJω | 1 | DFL | Stud | 4′ 4″ | 6′ 5″ | 6′ 10 ″ | 8′ 6 ″ | 9′ 1″ | 10′ 6″ | 10′ 11″ | 13′ 4″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | ╛ |
| Ⅱਢ | | | Standard | 4′ 2″ | 5′ 8″ | 6′ 0 ″ | 7′ 6 ″ | 8′ 0 ″ | 10′ 2″ | 10′ 10″ | 11′ 10″ | 12′ 7″ | 14′ 0″ | 14′ 0″ | ╛ |
| lda J | | SPF | #1 / #2 | 4′ 10″ | 8′ 3″ | 8′ 7 ″ | 9′ 9″ | 10′ 2″ | 10′ 7″ | 12′ 1″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | _ |
| | l | | #3 | 4′ 7″ | 8′ 2″ | 8′ 5 ″ | 9′ 8″ | 10′ 0″ | 11′ 6″ | 12′ 0 ″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | ╛ |
| ق | o U | HF | Stud | 4′ 7″ | 8′ 2″ | 8′ 5 ″ | 9′ 8 ″ | 10′ 0″ | 11′ 6″ | 12′ 0 ″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | ╛ |
| | ا آ | 1 11 | Standard | 4′ 7″ | 7′ 0″ | 7′ 5″ | 9′ 4″ | 10′ 0″ | 11′ 6″ | 12′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | ╛ |
| $ \times $ | | | #1 | 5′ 1″ | 8′ 5″ | 8′ 8 ″ | 9′ 11″ | 10′ 3″ | 11′ 9″ | 12′ 3″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | ╛ |
| II A | * | SP | #2 | 4′ 10″ | 8′ 3″ | 8′ 7 ″ | 9′ 9″ | 10′ 2″ | 11′ 7″ | 12′ 1″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | ╛ |
| MΩ | ù | L | #3 | 4′ 9″ | 7′ 4″ | 7′ 10″ | 9′ 8″ | 10′ 1″ | 11′ 7″ | 12′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 1 |
| | 15 | DFL | Stud | 4′ 9″ | 7′ 4″ | 7′ 10″ | 9′ 8″ | 10′ 1″ | 11′ 7″ | 12′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 1 |
| | | | Standard | 4′ 7″ | 6′ 6″ | 6′ 11″ | 8′ 8 ″ | 9′ 3″ | 11′ 6″ | 12′ 0″ | 13′ 7″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | ⅃ |
| | | | | | | | | Symr Abou | ļ¢ | | | | | | |
| I | | | - W | | M | | | Abou | ri_ | | | | | | |



1x4 Braces shall be SRB (Stress-Rated Board) **For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards, Group B values may be used with these grades.

Gable Truss Detail Notes: Wind Load deflection criterion is L/240.

Provide uplift connections for 75 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.

Attach "L" braces with 10d (0.128"x3.0" min) nails. ★ For (1) "L" brace: space nails at 2" o.c. in 18" end zones and 4" o.c. between zones. ₩₩For (2) "L" braces: space nails at 3" o.c. in 18" end zones and 6" o.c. between zones.

"L" bracing must be a minimum of 80% of web member length.

| Gable Vertical Plate | Sizes |
|---|-----------|
| Vertical Length | No Splice |
| Less than 4' 0" | 2X3 |
| Greater than 4' 0", but less than 11' 6" | 3X4 |
| Greater than 11' 6" | 4X4 |
| + Refer to common truss | |

Refer to the Building Designer for conditions not addressed by this detail.

Gable Truss Diagonal brace option: vertical length may be doubled when diagonal brace is used. Connect diagonal brace for 600# at each end. Max web "L" Brace End total length is 14'. Zones, typ. 2x6 DF-L #2 or better diagonal brace; single Vertical length shown or double cut in table above. (as shown) at upper end. Continuous Bearing Connect diagonal at Refer to chart about or midpoint of vertical web.

VARNINGI READ AND FOLLOW ALL NOTES ON THIS DRAWING ***IMPORTANT*** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

Trusses require extreme care in fabricating, shaping, installing and installing and follow the latest edition of BCSI (Building Component Safety Information, brill and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid celling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise.

Refer to drawings 160A-Z for standard plate positions.

Alpine, a division of ITV Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.

A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design his drawing, indicates acceptance of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see this Job's general notes page and these web sites: ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org



Florida Certificate of Product Approval #FL 1999

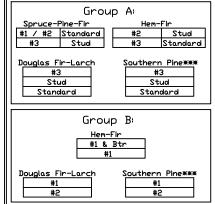
ASCE 7-10: 160 mph Wind Speed, 30' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Dr: 140 MPH Wind Speed, 30' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00

Dr: 140 mph Wind Speed, 30' Mean Height, Enclosed, Exposure D, Kzt = 1.00

Dr: 120 mph Wind Speed, 30' Mean Height, Partially Enclosed, Exposure D, Kzt = 1,00

| | | 2x4 Vertica | Brace | No | (1) 1×4 "L | Brace * | (1) 2×4 *L | " Brace * | (2) 2×4 *L | Brace ** | (1) 2×6 ' L | " Brace * | (2) 2x6 *L | Brace ** |
|-----------------|----------------|----------------|----------|---------------|---------------|---------------|---------------|---------------|----------------|----------------|--------------------|-----------|------------|----------|
| | Spacing | Species | Grade | _ | Group A | Group B | Group A | Group B | Group A | Group B | Group A | Group B | Group A | Group B |
| th | | | #1 / #2 | 3′ 8″ | 6' 3" | 6′ 6″ | 7′ 5″ | 7′ 8″ | 8′ 9″ | 9′ 2″ | 11′ 7″ | 12′ 0″ | 13′ 9″ | 14′ 0″ |
| | ; ; | SPF | #3 | 3′ 6″ | 5′ 5 ″ | 5′ 9 ″ | 7′ 2″ | 7′ 7″ | 8′ 8 ″ | 9′ 1″ | 11′ 3″ | 11' 11" | 13′ 7″ | 14′ 0″ |
| <u>6</u> | Ų | HF | Stud | 3′ 6″ | 5′ 4 ″ | 5′ 8 ″ | 7′ 2″ | 7′ 7* | 8′ 8″ | 9′ 1″ | 11′ 2″ | 11' 11" | 13′ 7″ | 14′ 0″ |
| | | | Standard | 3′ 6″ | 4′ 7″ | 4′ 11″ | 6′ 2″ | 6′ 7 ″ | 8′ 4″ | 8′ 11 ″ | 9′ 8″ | 10′ 4″ | 13′ 1″ | 14′ 0″ |
| ا به اا | | 0.0 | #1 | 3′ 10″ | 6′ 4″ | 6′ 7 ″ | 7′ 6″ | 7′ 9″ | 8′ 11 ″ | 9′ 3″ | 11′ 9″ | 12′ 2″ | 13′ 11″ | 14′ 0″ |
| $ \perp $ | * | SP | #2 | 3′ 8″ | 6′ 1″ | 6′ 6 ″ | 7′ 5″ | 7′ 8 ″ | 8′ 9 ″ | 9′ 2″ | 11′ 7″ | 12′ 0″ | 13′ 9″ | 14′ 0″ |
| | 4 | l | #3 | 3′ 7″ | 4′ 10″ | 5′ 2″ | 6′ 5 ″ | 6′ 11″ | 8′ 9″ | 9′ 1″ | 10′ 1″ | 10′ 10″ | 13′ 8″ | 14′ 0″ |
| | N | IDFLI | Stud | 3′ 7″ | 4' 10" | 5′ 2 ″ | 6′ 5 ″ | 6′ 11″ | 8′ 9 ″ | 9′ 1″ | 10′ 1″ | 10′ 10″ | 13′ 8″ | 14′ 0″ |
| ŭ | | | Standard | 3′ 3″ | 4′ 3″ | 4′ 7″ | 5′ 8 ″ | 6′ 1″ | 7′ 9″ | 8′ 3″ | 8′ 11 ″ | 9′ 7″ | 12′ 2″ | 13′ 0″ |
| II <u>.</u> U I | | | #1 / #2 | 4′ 2″ | 7′ 2″ | 7′ 5″ | 8′ 5 ″ | 8′ 9 ″ | 10′ 1″ | 10′ 6″ | 13′ 3″ | 13′ 9″ | 14′ 0″ | 14′ 0″ |
| 🚎 | . - | SPF | #3 | 4′ 0″ | 6′ 7 ″ | 7′ 5″ | 8′ 4″ | 8′ 8 ″ | 9′ 11″ | 10′ 4″ | 13′ 1″ | 13′ 7″ | 14′ 0″ | 14′ 0″ |
| | U O | HF | Stud | 4′ 0″ | 6′ 7 ″ | 7′ 0″ | 8′ 4″ | 8′ 8 ″ | 9′ 11″ | 10′ 4″ | 13′ 1″ | 13′ 7″ | 14′ 0″ | 14′ 0″ |
| à | l | 1 11 | Standard | 4′ 0″ | 5′ 8 ″ | 6′ 0″ | 7′ 6″ | 8′ 0″ | 9′ 11″ | 10′ 4″ | 11′ 10″ | 12′ 8″ | 14′ 0″ | 14′ 0″ |
| 🗸 | | | #1 | 4′ 5 ″ | 7′ 3″ | 7′ 6″ | 8′ 7 ″ | 8′ 11″ | 10′ 2″ | 10′ 7″ | 13′ 5 ″ | 13′ 11″ | 14′ 0″ | 14′ 0″ |
| / | | SP | #2 | 4′ 2″ | 7′ 2″ | 7′ 5″ | 8′ 5 ″ | 8′ 9 ″ | 10′ 1″ | 10′ 6″ | 13′ 3″ | 13′ 9″ | 14′ 0″ | 14′ 0″ |
| | | l | #3 | 4′ 1″ | 5′ 11″ | 6′ 4″ | 7′ 11″ | 8′ 5 ″ | 10′ 0″ | 10′ 5″ | 12′ 5 ″ | 13′ 3″ | 14′ 0″ | 14′ 0″ |
| IJωI | 16 | IDFLI | Stud | 4′ 1″ | 5′ 11″ | 6′ 4″ | 7′ 11″ | 8′ 5 ″ | 10′ 0″ | 10′ 5″ | 12′ 5 ″ | 13′ 3″ | 14′ 0″ | 14′ 0″ |
| | | | Standard | 4′ 0″ | 5′ 3 ″ | 5′ 7″ | 7′ 0″ | 7′ 6 ″ | 9′ 5″ | 10′ 2″ | 11′ 0″ | 11′ 9″ | 14′ 0″ | 14′ 0″ |
| Gabl | | CDE | #1 / #2 | 4′ 8″ | 7′ 11″ | 8′ 2″ | 9′ 4″ | 9′ 8″ | 10′ 1″ | 11′ 6″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| ď | | SPF | #3 | 4′ 5″ | 7′ 7″ | 8′ 1″ | 9′ 2″ | 9′ 6″ | 10′ 11″ | 11′ 5″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| U U | U | HF | Stud | 4′ 5″ | 7′ 7″ | 8′ 1″ | 9′ 2″ | 9′ 6″ | 10′ 11″ | 11′ 5″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| | O U | 1 11 | Standard | 4′ 5 ″ | 6′ 6″ | 6′ 11″ | 8′ 8″ | 9′ 3″ | 10′ 11″ | 11′ 5 ″ | 13′ 7″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| $ \times $ | | | #1 | 4′ 10″ | 8′ 0″ | 8′ 3″ | 9′ 5″ | 9′ 9″ | 11′ 2″ | 11′ 8″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| [[급] | | SP | #2 | 4′ 8″ | 7′ 11″ | 8′ 2″ | 9′ 4″ | 9′ 8″ | 11′ 1″ | 11′ 6″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| Μα | ù | ا ہے۔ ا | #3 | 4′ 6″ | 6′ 10″ | 7′ 3″ | 9′ 1″ | 9′ 7″ | 11′ 0″ | 11′ 6″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| _ | 1, | DFL | Stud | 4′ 6″ | 6′ 10″ | 7′ 3″ | 9′ 1″ | 9′ 7″ | 11′ 0″ | 11′ 6″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| | | | Standard | 4′ 5 ″ | 6′ 1″ | 6′ 5 ″ | 8′ 1″ | 8′ 7″ | 10′ 11″ | 11′ 5 ″ | 12′ 8 ″ | 13′ 7″ | 14′ 0″ | 14′ 0″ |



Bracing Group Species and Grades:

1x4 Braces shall be SRB (Stress-Rated Board) **For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards, Group B values may be used with these grades.

Gable Truss Detail Notes: Wind Load deflection criterion is L/240.

Provide uplift connections for 135 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.

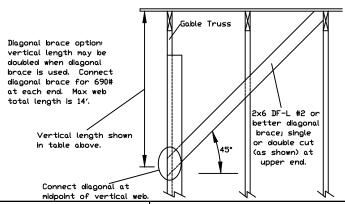
Attach "L" braces with 10d (0.128"x3.0" min) nails. ★ For (1) "L" brace: space nails at 2" o.c. in 18" end zones and 4" o.c. between zones. ₩ ¥For (2) "L" braces: space nails at 3" o.c. in 18" end zones and 6" o.c. between zones.

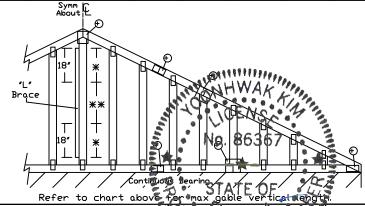
"L" bracing must be a minimum of 80% of web member length.

| Vertical Length | No Splice |
|---|-----------|
| Less than 4' 0' | 2X4 |
| Greater than 4' 0", but less than 12' 0" | 4X4 |

peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.





VARNINGI READ AND FOLLOW ALL NOTES ON THIS DRAWING ***IMPORTANT*** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and macing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI unless noted otherwise, top chord shall have properly attached structural sheathing and botton chord shall have a properly attached rigid celling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. Applicable to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions.

Alpine, a division of ITV Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.

A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see this job's general notes page and these web sites: ALPINE: www.alpineitw.com, TPI: www.tpinst.org, SBCA: www.sbcacomponents.com, ICC: www.lccsafe.org

MAX. TOT. LD. 60 PSF 2024#30258, Yoonhwak Kim, FUZE #8696CING 24.0"

Florida Certificate of Product Approval #FI

ASCE7-10-GAB16030 DATE 10/01/14 DRWG A16030ENC101014

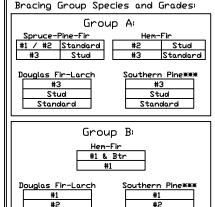
ASCE 7-16: 160 mph Wind Speed, 30' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Dr: 140 MPH Wind Speed, 30' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00

Dr: 140 mph Wind Speed, 30' Mean Height, Enclosed, Exposure D, Kzt = 1.00

Dr: 120 mph Wind Speed, 30' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

| | | 24 | | | | | | | l | | | | 1 | |
|--------------|----------------|------------------|----------|---------------|---------------|---------------|---------------|----------------|----------------|----------------|----------------|-----------|-----------|----------|
| | | 2x4 · Vertica | Brace | No | (1) 1×4 *L | " Brace * | (1) 2×4 *L | ." Brace * | (2) 2×4 L | Brace ** | (1) 2×6 L | " Brace * | (2) 2x6 L | Brace ** |
| _ | Spacing | Species | | Braces | Group A | Group B | Group A | Group B | Group A | Group B | Group A | Group B | Group A | Group B |
| 누 | | CDE | #1 / #2 | 3′ 8″ | 6′ 3″ | 6′ 6″ | 7′ 5″ | 7′ 8 ″ | 8′ 9″ | 9′ 2″ | 11′ 7″ | 12′ 0″ | 13′ 9″ | 14′ 0″ |
| | 1.7 | SPF | #3 | 3′ 6″ | 5′ 5 ″ | 5′ 9″ | 7′ 2″ | 7′ 7″ | 8′ 8 ″ | 9′ 1″ | 11′ 3″ | 11′ 11″ | 13′ 7″ | 14′ 0″ |
| 2 | Ų | HF | Stud | 3′ 6″ | 5′ 4″ | 5′ 8 ″ | 7′ 2″ | 7′ 7″ | 8′ 8″ | 9′ 1″ | 11′ 2″ | 11′ 11″ | 13′ 7″ | 14′ 0″ |
| > | ō | | Standard | 3′ 6″ | 4′ 7″ | 4′ 11″ | 6′ 2″ | 6′ 7 ″ | 8′ 4″ | 8′ 11 ″ | 9′ 8″ | 10′ 4″ | 13′ 1″ | 14′ 0″ |
| ا به ا | | | #1 | 3′ 10″ | 6′ 4″ | 6′ 7″ | 7′ 6″ | 7′ 9″ | 8′ 11 ″ | 9′ 3″ | 11′ 9″ | 12′ 2″ | 13′ 11″ | 14′ 0″ |
| | * | LSP | #2 | 3′ 8″ | 6′ 1″ | 6′ 6″ | 7′ 5″ | 7′ 8 ″ | 8′ 9″ | 9′ 2″ | 11′ 7″ | 12′ 0″ | 13′ 9″ | 14′ 0″ |
| | 4 | l | #3 | 3′ 7″ | 4′ 10″ | 5′ 2″ | 6′ 5 ″ | 6′ 11″ | 8′ 9″ | 9′ 1″ | 10′ 1″ | 10′ 10″ | 13′ 8″ | 14′ 0″ |
| d | N | IDFL | Stud | 3′ 7 ″ | 4' 10" | 5′ 2″ | 6′ 5 ″ | 6′ 11″ | 8′ 9 ″ | 9′ 1″ | 10′ 1″ | 10′ 10″ | 13′ 8″ | 14′ 0″ |
| | | | Standard | 3′ 3″ | 4′ 3″ | 4′ 7″ | 5′ 8″ | 6′ 1″ | 7′ 9″ | 8′ 3″ | 8′ 11 ″ | 9′ 7″ | 12′ 2″ | 13′ 0″ |
| 1.9 | | | #1 / #2 | 4′ 2″ | 7′ 2″ | 7′ 5″ | 8′ 5 ″ | 8′ 9 ″ | 10′ 1″ | 10′ 6″ | 13′ 3 ″ | 13′ 9″ | 14′ 0″ | 14′ 0″ |
| +> | - | SPF | #3 | 4′ 0″ | 6′ 7 ″ | 7′ 5″ | 8′ 4″ | 8′ 8 ″ | 9′ 11″ | 10′ 4″ | 13′ 1″ | 13′ 7″ | 14′ 0″ | 14′ 0″ |
| ; | O, O | HF | Stud | 4′ 0″ | 6′ 7 ″ | 7′ 0″ | 8′ 4″ | 8′ 8 ″ | 9′ 11″ | 10′ 4″ | 13′ 1″ | 13′ 7″ | 14′ 0″ | 14′ 0″ |
| l à | Ō | 1 11 | Standard | 4′ 0″ | 5′ 8 ″ | 6′ 0″ | 7′ 6″ | 8′ 0 ″ | 9′ 11″ | 10′ 4″ | 11′ 10″ | 12′ 8″ | 14′ 0″ | 14′ 0″ |
| 1 ~ | | | #1 | 4′ 5 ″ | 7′ 3″ | 7′ 6″ | 8′ 7 ″ | 8′ 11 ″ | 10′ 2″ | 10′ 7″ | 13′ 5 ″ | 13′ 11″ | 14′ 0″ | 14′ 0″ |
| > | * | ISP | #2 | 4′ 2″ | 7′ 2″ | 7′ 5″ | 8′ 5 ″ | 8′ 9 ″ | 10′ 1″ | 10′ 6″ | 13′ 3″ | 13′ 9″ | 14′ 0″ | 14′ 0″ |
| | 9 | | #3 | 4′ 1″ | 5′ 11″ | 6′ 4″ | 7′ 11″ | 8′ 5 ″ | 10′ 0″ | 10′ 5″ | 12′ 5 ″ | 13′ 3″ | 14′ 0″ | 14′ 0″ |
| ا س | 16 | IDFL | Stud | 4′ 1″ | 5′ 11″ | 6′ 4″ | 7′ 11″ | 8′ 5 ″ | 10′ 0″ | 10′ 5″ | 12′ 5 ″ | 13′ 3″ | 14′ 0″ | 14′ 0″ |
| I — | | | Standard | 4′ 0″ | 5′ 3 ″ | 5′ 7 ″ | 7′ 0″ | 7′ 6″ | 9′ 5″ | 10′ 2″ | 11′ 0″ | 11′ 9″ | 14′ 0″ | 14′ 0″ |
| | | CDE | #1 / #2 | 4′ 8″ | 7′ 11″ | 8′ 2″ | 9′ 4″ | 9′ 8″ | 10′ 1″ | 11′ 6″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| ෮ | . . | SPF | #3 | 4′ 5″ | 7′ 7″ | 8′ 1″ | 9′ 2″ | 9′ 6″ | 10′ 11″ | 11′ 5″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| 0 | U | HF | Stud | 4′ 5 ″ | 7′ 7″ | 8′ 1 ″ | 9′ 2″ | 9′ 6″ | 10′ 11″ | 11′ 5 ″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| | Ō | 1 11 | Standard | 4′ 5 ″ | 6′ 6″ | 6′ 11″ | 8′ 8″ | 9′ 3″ | 10′ 11″ | 11′ 5 ″ | 13′ 7″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| $ \times $ | | | #1 | 4′ 10″ | 8′ 0 ″ | 8′ 3″ | 9′ 5″ | 9′ 9″ | 11′ 2″ | 11′ 8″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| | * | SP | #2 | 4′ 8″ | 7′ 11″ | 8′ 2″ | 9′ 4″ | 9′ 8″ | 11′ 1″ | 11′ 6″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| M | ù | L. | #3 | 4′ 6″ | 6′ 10″ | 7′ 3″ | 9′ 1″ | 9′ 7″ | 11′ 0″ | 11′ 6″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| | 10 | IDFL | Stud | 4′ 6″ | 6′ 10″ | 7′ 3″ | 9′ 1″ | 9′ 7″ | 11′ 0″ | 11′ 6″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| | · | | Standard | 4′ 5″ | 6′ 1″ | 6′ 5″ | 8′ 1″ | 8′ 7 ″ | 10′ 11″ | 11′ 5″ | 12′ 8 ″ | 13′ 7″ | 14′ 0″ | 14′ 0″ |



1x4 Braces shall be SRB (Stress-Rated Board) **For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards, Group B values may be used with these grades.

Gable Truss Detail Notes: Wind Load deflection criterion is L/240.

Provide uplift connections for 135 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.

Attach "L" braces with 10d (0.128"x3.0" min) nails. ★ For (1) "L" brace: space nails at 2" o.c. in 18" end zones and 4" o.c. between zones. ₩ ¥For (2) "L" braces: space nails at 3" o.c. in 18" end zones and 6" o.c. between zones.

"L" bracing must be a minimum of 80% of web member length.

| Vertical Length | No Splice |
|---|-----------|
| Less than 4' 0" | 2X4 |
| Greater than 4' 0", but less than 12' 0" | 4X4 |

Refer to the Building Designer for conditions not addressed by this detail.

Gable Truss Diagonal brace option: vertical length may be doubled when diagonal brace is used. Connect diagonal brace for 690# at each end. Max web "L" Brace End total length is 14'. Zones, typ. 2x6 DF-L #2 or better diagonal brace; single Vertical length shown or double cut in table above. (as shown) at upper end. Connect diagonal at Refer to chart above midpoint of vertical web.

Symm C

VARNINGI READ AND FOLLOW ALL NOTES ON THIS DRAWING ***IMPORTANT*** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Bullding Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and botton chord shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise.

Refer th drawing 16/64-7 for standard risk needling. Refer to drawings 160A-Z for standard plate positions.

Alpine, a division of ITV Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.

A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

155 Harlem Ave North Building, 4th Floor

Glenview, IL 60025

For more information see this Job's general notes page and these web sites: ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org

ASCE7-16-GAB16030 01/26/2018 MAX, TOT, LD, 60 PSF 09/24/2025 COA#0-278, Yoonhwak Kim, FLIMA #86**367**ACING 24.0" Florida Certificate of Product Approval #FL 1999

ASCE 7-10: 180 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Dr: 160 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00

Dr: 160 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 1.00

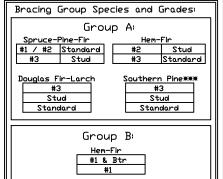
Or: 140 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

| | | 2x4 Vertica | Brace | No | (1) 1×4 "L | Brace * | (1) 2×4 "L | " Brace * | (2) 2×4 L | Brace ** | (1) 2×6 " L | " Brace * | (2) 2x6 " L | "Brace * | * |
|--------------|------------------|----------------|----------|----------------|---------------|----------------|----------------|----------------|----------------|----------------|--------------------|-----------|--------------------|----------|---|
| _ | Spacing | Species | Grade | | Group A | Group B | Group A | Group B | Group A | Group B | Group A | Group B | Group A | Group B | |
| 구 | | CDE | #1 / #2 | 3′ 6″ | 6′ 0″ | 6′ 3″ | 7′ 1″ | 7′ 4″ | 8′ 5 ″ | 8′ 9 ″ | 11′ 1″ | 11′ 7″ | 13′ 3″ | 13′ 9″ | 7 |
| ˈm | . ; | SPF | #3 | 3′ 4″ | 5′ 1 ″ | 5′ 5 ″ | 6′ 9 ″ | 7′ 3″ | 8′ 4 ″ | 8′ 8 ″ | 10′ 7″ | 11' 4" | 13′ 1″ | 13′ 7″ | _ |
| | Ų | HF | Stud | 3′ 4″ | 5′ 0 ″ | 5′ 4 ″ | 6′ 9″ | 7′ 2″ | 8′ 4 ″ | 8′ 8″ | 10′ 6″ | 11′ 3″ | 13′ 1″ | 13′ 7″ | |
| Ç | | 1 11 | Standard | 3′ 4″ | 4′ 4″ | 4′ 7″ | 5′ 9 ″ | 6′ 2″ | 7′ 10 ″ | 8′ 5 ″ | 9′ 1″ | 9′ 8″ | 12′ 4″ | 13′ 2″ | |
| به | | | #1 | 3′ 8″ | 6′ 1″ | 6′ 4″ | 7′ 2″ | 7′ 5″ | 8′ 6 ″ | 8′ 10 ″ | 11′ 3″ | 11′ 8″ | 13′ 4″ | 13′ 11″ | |
| \sqcup | * | SP | #2 | 3′ 6″ | 5′ 9″ | 6′ 2″ | 7′ 1″ | 7′ 4″ | 8′ 5 ″ | 8′ 9 ″ | 11′ 1″ | 11′ 7″ | 13′ 3″ | 13′ 9″ | |
| | 4 | l | #3 | 3′ 5 ″ | 4′ 7″ | 4′ 10″ | 6′ 1″ | 6′ 6″ | 8′ 2 ″ | 8′ 9 ″ | 9′ 6″ | 10′ 2″ | 12′ 11″ | 13′ 8″ | _ |
| d | N | DFL | Stud | 3′ 5 ″ | 4′ 7″ | 4′ 10″ | 6′ 1″ | 6′ 6″ | 8′ 2″ | 8′ 9 ″ | 9′ 6″ | 10′ 2″ | 12′ 11″ | 13′ 8″ | _ |
| | | | Standard | 3′ 1″ | 4′ 0″ | 4′ 3″ | 5′ 4″ | 5′ 9″ | 7′ 3″ | 7′ 9″ | 8′ 5 ″ | 9′ 0″ | 11′ 5″ | 12′ 3″ | _ |
| <u> </u> | | SPF | #1 / #2 | 4′ 0″ | 6′ 10″ | 7′ 2″ | 8′ 1″ | 8′ 5 ″ | 9′ 8″ | 10′ 1″ | 12′ 9″ | 13′ 3″ | 14′ 0″ | 14′ 0″ | 4 |
| | l . . | | #3 | 3′ 10″ | 6′ 2″ | 7′ 2″ | 8′ 0″ | 8′ 4″ | 9′ 7″ | 9′ 11″ | 12′ 7″ | 13′ 1″ | 14′ 0″ | 14′ 0″ | _ |
| | o V | HF | Stud | 3′ 10 ″ | 6′ 2″ | 6′ 7″ | 8′ 0 ″ | 8′ 4″ | 9′ 7″ | 9′ 11″ | 12′ 7″ | 13′ 1″ | 14′ 0″ | 14′ 0″ | _ |
| Πā | | 1 11 | Standard | 3′ 10″ | 5′ 4″ | 5′ 8″ | 7′ 1″ | 7′ 7″ | 9′ 7″ | 9′ 11″ | 11′ 1″ | 11′ 11″ | 14' 0" | 14′ 0″ | _ |
| \mathbb{I} | ~ | | #1 | 4′ 3″ | 7′ 0″ | 7′ 3″ | 8′ 3″ | 8′ 6″ | 9′ 9″ | 10′ 2″ | 12′ 11″ | 13′ 5″ | 14' 0" | 14′ 0″ | _ |
| / | | SP | #2 | 4′ 0″ | 6′ 10″ | 7′ 2″ | 8′ 1″ | 8′ 5″ | 9′ 8″ | 10′ 1″ | 12′ 9″ | 13′ 3″ | 14' 0" | 14′ 0″ | _ |
| | Ġ. | ъ | #3 | 3′ 11″ | 5′ 7 ″ | 5′ 11″ | 7′ 5″ | 7′ 11″ | 9′ 7″ | 10′ 0″ | 11′ 8″ | 12′ 6″ | 14' 0" | 14′ 0″ | _ |
| llω | <u> </u> | DFL | Stud | 3′ 11″ | 5′ 7 ″ | 5′ 11″ | 7′ 5″ | 7′ 11″ | 9′ 7″ | 10′ 0″ | 11′ 8″ | 12′ 6″ | 14′ 0″ | 14′ 0″ | _ |
| Gabl | | | Standard | 3′ 9″ | 4′ 11″ | 5′ 3″ | 6′ 7″ | 7′ 0″ | 8′ 11″ | 9′ 6″ | 10′ 4″ | 11′ 1″ | 14′ 0″ | 14′ 0″ | _ |
| | | SPF | #1 / #2 | 4′ 5 ″ | 7′ 7″ | 7′ 10″ | 8′ 11 ″ | 9′ 3″ | 9′ 8″ | 11′ 1″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | _ |
| II .₫ | l . . | | #3 | 4′ 3″ | 7′ 2″ | 7′ 7″ | 8′ 10″ | 9′ 2″ | 10′ 6″ | 10′ 11″ | 13′ 10″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | _ |
| | Ų | HF | Stud | 4′ 3″ | 7′ 1″ | 7′ 7″ | 8′ 10 ″ | 9′ 2″ | 10′ 6″ | 10′ 11″ | 13′ 10″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | _ |
| | Ιō | 1 11 | Standard | 4′ 3″ | 6′ 2″ | 6′ 6″ | 8′ 2″ | 8′ 9″ | 10′ 6″ | 10′ 11″ | 12′ 10 ″ | 13′ 9″ | 14′ 0″ | 14′ 0″ | _ |
| × | | | #1 | 4′ 8″ | 7′ 8″ | 7′ 11″ | 9′ 0″ | 9′ 5″ | 10′ 9″ | 11′ 2″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 4 |
| | * | SP | #2 | 4′ 5″ | 7′ 7″ | 7′ 10″ | 8′ 11″ | 9′ 3″ | 10′ 8″ | 11′ 1″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | _ |
| M | ù | | #3 | 4′ 4″ | 6′ 5″ | 6′ 10″ | 8′ 7″ | 9′ 12″ | 10′ 7″ | 11′ 0″ | 13′ 5″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 4 |
| | ;; | DFL | Stud | 4′ 4″ | 6′ 5 ″ | 6′ 10 ″ | 8′ 7″ | 9′ 12 ″ | 10′ 7″ | 11′ 0″ | 13′ 5″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 1 |
| | | | Standard | 4′ 3″ | 5′ 8″ | 6′ 1″ | 7′ 7″ | 8′ 1″ | 10′ 3″ | 10′ 11″ | 11′ 11″ | 12′ 9″ | 14′ 0″ | 14′ 0″ | |

Symm C

Refer to chart above

Brace



1x4 Braces shall be SRB (Stress-Rated Board) **For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards, Group B values may be used with these grades.

Southern Pine***

#1 #2

Gable Truss Detail Notes: Wind Load deflection criterion is L/240.

Douglas Fir-Larch

#1

Provide uplift connections for 100 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0' outlookers with 2' 0" overhang, or 12" plywood overhang.

Attach "L" braces with 10d (0.128"x3.0" min) nails. * For (1) "L" brace: space nails at 2" o.c. in 18" end zones and 4" o.c. between zones. ₩¥For (2) "L" braces: space nails at 3" o.c. in 18" end zones and 6" o.c. between zones.

"L" bracing must be a minimum of 80% of web member length.

| Gable Vertical Plate | No Splice |
|--|-----------|
| Less than 4' 0" | 2X3 |
| Greater than 4' 0", but less than 10' 0" | 4X4 |
| + Refer to common truss peak, splice, and heel pi | |

Refer to the Building Designer for conditions not addressed by this detail.

VARNINGI READ AND FOLLOW ALL NOTES ON THIS DRAWING ***IMPORTANT*** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

2x6 DF-L #2 or better diagonal brace; single

or double cut

(as shown) at upper end.

Gable Truss

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, br PI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid celling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise.

Refer to drawings 160A-Z for standard plate positions.

Alpine, a division of ITV Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.

A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see this job's general notes page and these web sites: ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org

MAX. TOT. LD. 60 PSF

DATE 10/01/14 DRWG A18015ENC101014

ASCE7-10-GAB18015

29624/20258, Yoonhwak Kim, HMRK,#85P67CING 24.0"

155 Harlem Ave North Building, 4th Floor Glenview, IL 60025

Diagonal brace option:

vertical length may be doubled when diagonal

brace is used. Connect diagonal brace for 775# at each end. Max web

Vertical length shown

Connect diagonal at

midpoint of vertical web.

in table above.

total length is 14'.

Florida Certificate of Product Approval #FL 19

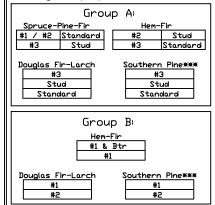
ASCE 7-16: 180 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Dr: 160 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00

Dr: 160 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 1.00

Or: 140 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

| | | 2x4 Vertica | Brace | No | (1) 1×4 "L | Brace * | I | | (2) 2×4 *L | | (1) 2×6 (L | " Brace * | (2) 2x6 1 L | Brace ** |
|-----------------|---------|----------------|----------|----------------|----------------|----------------|----------------|-------------------|---------------|----------------|----------------|-----------|--------------------|----------|
| | Spacing | Species | Grade | - | Group A | Group B | Group A | Group B | Group A | Group B | Group A | Group B | Group A | Group B |
| t | | CDE | #1 / #2 | 3′ 6″ | 6′ 0″ | 6′ 3″ | 7′ 1″ | 7′ 4″ | 8′ 5 ″ | 8′ 9″ | 11′ 1″ | 11′ 7″ | 13′ 3″ | 13′ 9″ |
| | 1 7 | SPF | #3 | 3′ 4″ | 5′ 1 ″ | 5′ 5 ″ | 6′ 9″ | 7′ 3″ | 8′ 4″ | 8′ 8″ | 10′ 7″ | 11' 4" | 13′ 1″ | 13′ 7″ |
| E | 0,0 | HF | Stud | 3′ 4″ | 5′ 0 ″ | 5′ 4 ″ | 6′ 9″ | 7′ 2″ | 8′ 4″ | 8′ 8 ″ | 10′ 6″ | 11′ 3″ | 13′ 1″ | 13′ 7″ |
| < | 0 | 1 11 | Standard | 3′ 4″ | 4′ 4″ | 4′ 7″ | 5′ 9″ | 6′ 2″ | 7′ 10″ | 8′ 5 ″ | 9′ 1″ | 9′ 8″ | 12′ 4″ | 13′ 2″ |
| a. | | | #1 | 3′ 8″ | 6′ 1″ | 6′ 4″ | 7′ 2″ | 7′ 5″ | 8′ 6″ | 8′ 10 ″ | 11′ 3″ | 11′ 8″ | 13′ 4″ | 13′ 11″ |
| | * | SP | #2 | 3′ 6″ | 5′ 9″ | 6′ 2″ | 7′ 1″ | 7′ 4″ | 8′ 5 ″ | 8′ 9″ | 11′ 1″ | 11′ 7″ | 13′ 3″ | 13′ 9″ |
| | 4 | l | #3 | 3′ 5 ″ | 4′ 7″ | 4′ 10″ | 6′ 1″ | 6′ 6″ | 8′ 2″ | 8′ 9″ | 9′ 6″ | 10′ 2″ | 12′ 11″ | 13′ 8″ |
| | N | IDFLI | Stud | 3′ 5 ″ | 4′ 7″ | 4′ 10″ | 6′ 1 ″ | 6′ 6″ | 8′ 2 ″ | 8′ 9″ | 9′ 6″ | 10′ 2″ | 12′ 11″ | 13′ 8″ |
| 전 | . – | | Standard | 3′ 1″ | 4′ 0″ | 4′ 3″ | 5′ 4″ | 5′ 9″ | 7′ 3″ | 7′ 9″ | 8′ 5 ″ | 9′ 0″ | 11′ 5″ | 12′ 3″ |
| II <u>.</u> U I | | | #1 / #2 | 4′ 0″ | 6′ 10 ″ | 7′ 2″ | 8′ 1 ″ | 8′ 5 ″ | 9′ 8″ | 10′ 1″ | 12′ 9 ″ | 13′ 3″ | 14′ 0″ | 14′ 0″ |
| 🚎 | | SPF | #3 | 3′ 10 ″ | 6′ 2″ | 7′ 2″ | 8′ 0 ″ | 8′ 4″ | 9′ 7″ | 9′ 11″ | 12′ 7″ | 13′ 1″ | 14' 0" | 14′ 0″ |
| | Ų | HF | Stud | 3′ 10 ″ | 6′ 2″ | 6′ 7″ | 8′ 0 ″ | 8′ 4″ | 9′ 7″ | 9′ 11″ | 12′ 7″ | 13′ 1″ | 14′ 0″ | 14′ 0″ |
| | Ō | 1 11 | Standard | 3′ 10″ | 5′ 4″ | 5′ 8″ | 7′ 1″ | 7′ 7″ | 9′ 7″ | 9′ 11″ | 11′ 1″ | 11′ 11″ | 14′ 0″ | 14′ 0″ |
| 🗸 | | | #1 | 4′ 3″ | 7′ 0″ | 7′ 3″ | 8′ 3″ | 8′ 6″ | 9′ 9″ | 10′ 2″ | 12′ 11″ | 13′ 5″ | 14' 0" | 14′ 0″ |
| / | * | SP | #2 | 4′ 0″ | 6′ 10″ | 7′ 2″ | 8′ 1 ″ | 8′ 5″ | 9′ 8″ | 10′ 1″ | 12′ 9″ | 13′ 3″ | 14′ 0″ | 14′ 0″ |
| | | l | #3 | 3′ 11″ | 5′ 7″ | 5′ 11″ | 7′ 5″ | 7′ 11″ | 9′ 7″ | 10′ 0″ | 11′ 8″ | 12′ 6″ | 14' 0" | 14′ 0″ |
| IJωl | 16 | IDFLI | Stud | 3′ 11″ | 5′ 7″ | 5′ 11 ″ | 7′ 5″ | 7′ 11″ | 9′ 7″ | 10′ 0″ | 11′ 8″ | 12′ 6″ | 14' 0" | 14′ 0″ |
| | | | Standard | 3′ 9″ | 4′ 11″ | 5′ 3″ | 6′ 7″ | 7′ 0″ | 8′ 11″ | 9′ 6″ | 10′ 4″ | 11′ 1″ | 14′ 0″ | 14′ 0″ |
| Gabl | | SPF | #1 / #2 | 4′ 5″ | 7′ 7″ | 7′ 10″ | 8′ 11 ″ | 9′ 3″ | 9′ 8″ | 11′ 1″ | 14′ 0″ | 14′ 0″ | 14' 0" | 14′ 0″ |
| d | | 12LL | #3 | 4′ 3″ | 7′ 2″ | 7′ 7″ | 8′ 10 ″ | 9′ 2″ | 10′ 6″ | 10′ 11″ | 13′ 10″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| U | U | HF | Stud | 4′ 3″ | 7′ 1″ | 7′ 7″ | 8′ 10″ | 9′ 2″ | 10′ 6″ | 10′ 11″ | 13′ 10″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| | סיכ | 1 11 | Standard | 4′ 3″ | 6′ 2″ | 6′ 6″ | 8′ 2 ″ | 8′ 9 ″ | 10′ 6″ | 10′ 11″ | 12′ 10″ | 13′ 9″ | 14' 0" | 14′ 0″ |
| $ \times $ | | | #1 | 4′ 8″ | 7′ 8″ | 7′ 11″ | 9′ 0″ | 9′ 5″ | 10′ 9″ | 11′ 2″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| 미국 | * | SP | #2 | 4′ 5″ | 7′ 7″ | 7′ 10″ | 8′ 11 ″ | 9′ 3″ | 10′ 8″ | 11′ 1″ | 14′ 0″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| Μα | ù | ا ہے۔ ا | #3 | 4′ 4″ | 6′ 5″ | 6′ 10 ″ | 8′ 7 ″ | 9′ 12″ | 10′ 7″ | 11′ 0″ | 13′ 5″ | 14′ 0″ | 14′ 0″ | 14′ 0″ |
| | 10 | DFL | Stud | 4′ 4″ | 6′ 5″ | 6′ 10″ | 8′ 7″ | 9′ 12 ″ | 10′ 7″ | 11′ 0″ | 13′ 5″ | 14′ 0″ | 14' 0" | 14′ 0″ |
| | | | Standard | 4′ 3″ | 5′ 8″ | 6′ 1″ | 7′ 7″ | 8′ 1″ | 10′ 3″ | 10′ 11″ | 11′ 11″ | 12′ 9″ | 14′ 0″ | 14′ 0″ |



Bracing Group Species and Grades:

1x4 Braces shall be SRB (Stress-Rated Board) **For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards, Group B values may be used with these grades.

Gable Truss Detail Notes: Wind Load deflection criterion is L/240.

Provide uplift connections for 100 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0' outlookers with 2' 0" overhang, or 12" plywood overhang.

Attach "L" braces with 10d (0.128"x3.0" min) nails. ¥ For (1) "L" brace: space nalls at 2" o.c. in 18" end zones and 4" o.c. between zones. ₩¥For (2) "L" braces: space nails at 3" o.c. in 18" end zones and 6" o.c. between zones.

"L" bracing must be a minimum of 80% of web member length.

| Vertical Length | No Splice |
|---|-----------|
| Less than 4' 0" | 2X3 |
| Greater than 4' 0", but less than 10' 0" | 4X4 |

Refer to the Building Designer for conditions not addressed by this detail.

Gable Truss Diagonal brace option: vertical length may be doubled when diagonal brace is used. Connect diagonal brace for 775# at each end. Max web "L" Brace End total length is 14'. Zones, typ. 2x6 DF-L #2 or better diagonal brace; single Vertical length shown or double cut in table above. (as shown) at upper end. Continuous Bearing Connect diagonal at Refer to chart above midpoint of vertical web.

VARNINGI READ AND FOLLOW ALL NOTES ON THIS DRAWING ***IMPORTANT*** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

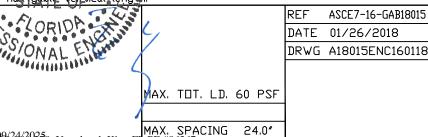
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and botton chord shall have a properly attached ripid celling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise.

Refer to drawings IGOA-Z for standard plate positions.

Alpine, a division of ITV Building Components Group Inc. shall not be responsible for any deviation from this drawing, any fallure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.

A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see this job's general notes page and these web sites: ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org



Florida Certificate of Product Approval #FL 1999



CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

Notes:

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforecement or scab reinforcement.

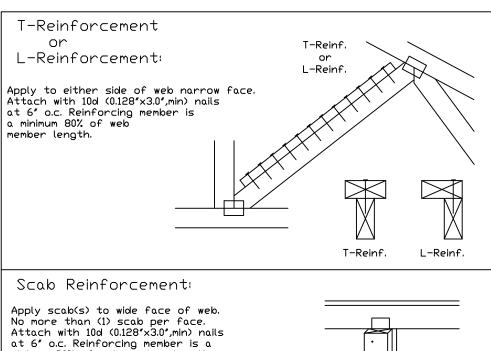
Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

Use scabs instead of L- or T- reinforcement on webs with intersecting truss joints, such as K-web joints, that may interfere with proper application along the narrow face of the web.

| Web Member | Specified CLR | Alternative Reir | |
|------------|---------------|------------------|-------------------|
| Size | Restraint | T- or L- Reinf. | |
| 2x3 or 2x4 | 1 row | 2×4 | 1-2×4 |
| 2x3 or 2x4 | 2 rows | 2×6 | 2-2×4 |
| 2×6 | 1 row | 2×4 | 1-2×6 |
| 2×6 | 2 rows | 2×6 | 2-2×4(*) |
| 2×8 | 1 row | 2x6 | 1-2×8 |
| 2×8 | 2 rows | 2x6 | 2-2×6(*) |

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

Center scab on wide face of web. Apply (1) scab to each face of web.





Scab Reinf.

VARNINGI READ AND FOLLOW ALL NOTES ON THIS DRAVINGI ****IMPORTANT*** FURNISH THIS DRAVING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

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ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org

IREF CLR Subst. TC DL DATE 01/02/19 BC DI DRWG BRCLBSUB0119 BC II PSF NTOT. LD. PSF DUR. FAC. SPACING

Florida Certificate of Product Approval #FL 1999

North Building, 4th Floor Glenview, IL 60025

155 Harlem Ave

CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

Notes:

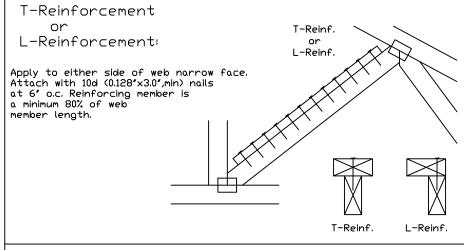
This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforecement or scab reinforcement.

Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

| Web Member | Specified CLR | Alternative Reir | |
|------------|---------------|------------------|-------------------|
| Size | Restraint | T- or L- Reinf. | |
| 2x3 or 2x4 | 1 row | 2×4 | 1-2×4 |
| 2x3 or 2x4 | 2 rows | 2×6 | 2-2×4 |
| 2×6 | 1 row | 2×4 | 1-2×6 |
| 2×6 | 2 rows | 2×6 | 2-2×4(米) |
| 2×8 | 1 row | 2×6 | 1-2×8 |
| 2×8 | 2 rows | 2×6 | 2-2×6(*) |

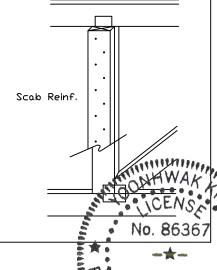
T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

(*) Center scab on wide face of web. Apply (1) scab to each face of web.



Scab Reinforcement:

Apply scab(s) to wide face of web. No more than (1) scab per face. Attach with 10d (0.128"x3.0", min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.





13723 Riverport Drive Maryland Heights, MO 63043

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| | 100 | • | rates Wester | | |
|----------------|------------|-------------|--------------|---|------|
| TC LL TC DL | PSF PSF | REF BATE | SCATEUOST. | 1 | 43°6 |
| BC DL | PSF | DIS VIC | BRCLBSUB101 | | 8 |
| BC LL | PSF | 101 | ONAL | F | |
| TOT. LD. | PSF | | | | - |
| DUR. FAC. | | | | | |
| SPACING | | | | | |

NAIL SPACING DETAIL

MINIMUM SPACING FOR SINGLE BLOCK IS SHOWN. DOUBLE NAIL SPACINGS AND STAGGER NAILING FOR TWO BLOCKS. GREATER SPACING MAY BE REQUIRED TO AVOID SPLITTING.

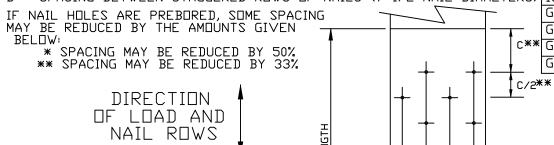
BLOCK LOCATION, SIZE, LENGTH, GRADE AND TOTAL NUMBER AND TYPE OF NAILS ARE TO BE SPECIFIED ON SEALED DESIGN REFERENCING THIS DETAIL.

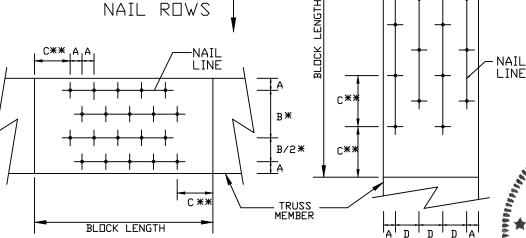
LOAD PERPENDICULAR TO GRAIN

- A EDGE DISTANCE AND SPACING BETWEEN STAGGERED ROWS OF NAILS (6 NAIL DIAMETERS)
- B SPACING OF NAILS IN A ROW (12 NAIL DIAMETERS)
- C END DISTANCE (15 NAIL DIAMETERS)

LOAD PARALLEL TO GRAIN

- A EDGE DISTANCE (6 NAIL DIAMETERS)
- C SPACING OF NAILS IN A ROW AND END DISTANCE (15 NAIL DIAMETERS)
- D SPACING BETWEEN STAGGERED ROWS OF NAILS (7 1/2 NAIL DIAMETERS)





LOAD APPLIED PERPENDICULAR TO GRAIN

LOAD APPLIED PARALLEL TE GRAI

VARNING READ AND FOLLOW ALL NOTES ON THIS DRAVING ***IMPORTANT*** FURNISH THIS DRAVING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

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Refer to drawings 160A-Z for standard plate positions.

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MINIMUM NAIL SPACING DISTANCES

| | DISTANCES | | | |
|---------------------------------|-----------|--------|--------|--------|
| NAIL TYPE | Α | B* | C** | D |
| 8d BOX (0.113"X 2.5",MIN) | 3/4" | 1 3/8" | 1 3/4" | 7/8" |
| 10d BOX (0.128"X 3.",MIN) | 7/8" | 1 5/8" | 2" | 1" |
| 12d BOX (0.128"X 3.25",MIN) | 7/8" | 1 5/8" | 2" | 1" |
| 16d BOX (0.135"X 3.5",MIN) | 7/8" | 1 5/8" | 2 1/8" | 1 1/8" |
| 20d BOX (0.148"X 4.",MIN) | 1" | 1 7/8" | 2 1/4" | 1 1/8" |
| 8d COMMON (0.131"X 2.5",MIN) | 7/8" | 1 5/8" | 2" | 1" |
| 10d CDMMDN (0.148"X 3.",MIN) | 1" | 1 7/8" | 2 1/4" | 1 1/8" |
| 12d COMMON (0.148"X 3.25",MIN) | 1" | 1 7/8" | 2 1/4" | 1 1/8" |
|) 16d COMMON (0.162"X 3.5",MIN) | 1' | 2" | 2 1/2" | 1 1/4" |
| GUN (0.120"X 2.5",MIN) | 3/4" | 1 1/2" | 1 7/8" | 1" |
| GUN (0.131"X 2.5",MIN) | 7/8" | 1 5/8" | 2" | 1" |
| * GUN (0.120"X 3.",MIN) | 3/4" | 1 1/2" | 1 7/8" | 1" |
| GUN (0.131"X 3.",MIN) | 7/8″ | 1 5/8" | 2" | 1″ |

No. 86367

REF NAIL SPACE
DATE 10/01/14
DRWG CNNAILSP1014

PO24/20258 Yoonhwak Kim, FLIPE #86367 Florida Certificate of Product Approval #FL 1999



System 42 Ply to Ply Connection Detail

Using GRK (RSS) JTS 1/4x6-3/4 or Simpson SDS25600 or SDW22634 Strong Drive Screws or Equal.

Refer to Alpine sealed drawing for individual truss design.

Max. Concentrated Load per Chart Below Top Chord Screw Location dditional Web Screw Location

Apply screws to top chord within 12" of the concentrated load location @ 4" o.c., min, evenly distributing them to each side of the concentrated load. A maximum of 6 screws may be applied to the top chord for each concentrated load.

For double top chords, evenly distribute the screws over both top chords, using same spacing guidelines specified above. The max number of top chord screws is 6 per chord member for a total maximum of 12 screws.

If the concentrated load connection requires more screws than 6 per top chord member and the load is located at a panel point where webs intersect the top chord, the remainder of required screws may be applied to those webs below the concentrated load location evenly spaced @ 4" o.c., min, keeping the 3" min end distances. Each additional screw is worth 474 lb for SP webs, 442 lb for DFL webs, and 400 lb for SPF webs.

Load Applied to This Ply 3 1/2"---Truss Member (Chord Shown) 1 1/2"

Screw Position in the Truss

JTS or Strong-Drive Screw

Max. Uniform Load per Chart Below

For single top chord, see chart below for screw spacing. For double top chord the screw spacing may be doubled (but may not exceed 24" o.c. per chord). Screw spacing shall be offset by 1/2 the o.c. spacing in each chord

Screws need only apply to the extents of that load.

For chord sections supporting less than 100 plf apply one screw at each top chord joint location.

| # of Screws | Maximum Concentrated Load (lbs) (1.00 DF) | | | |
|----------------|--|------|------|--|
| 30,043 | SP | DFL | SPF | |
| 1 | 474 | 442 | 400 | |
| 2 | 984 | 884 | 800 | |
| 3 | 1422 | 1326 | 1200 | |
| 4 | 1896 | 1768 | 1600 | |
| 5 | 2370 | 2210 | 2000 | |
| 6 | 2844 | 2652 | 2400 | |
| 7 | 3318 | 3094 | 2800 | |
| 8 | 3792 | 3536 | 3200 | |
| 9 | 4266 | 3978 | 3600 | |
| 10 | 4740 | 4420 | 4000 | |
| 11 | 5214 | 4862 | 4400 | |
| 12 | 5688 | 5304 | 4800 | |

General Notes:

- 1. Screws centered along the 1.5" dimension of the 4x2 member.
- 2. Minimum end distance of 3".
- 3. Screws installed with head in loaded member.
- 4. Gap between plies not to exceed 1/8".
- 5. Screw location may be adjusted up to 1' to avoid partition, with other hardware or to avoid lumber defeats.
- with other hardware or to avoid lumber defeats.

 6. Do not install screws in areas where lumber was exceeds 1/4/.

 7. Equal loads from both faces or loads that are eventy.

 distributed to each ply do not require connections per this detail.
- 8. For 3x2 members use GRK (RSS) JTS 1/\$x5 strew10 or 85m3 507's SDS25412 or SDW22500 screws or eque.
- 9. Contact Alpine for special connections not covered by this detail.

| Top Chord Screw | Maximum Uniform Load (plf) Along Top Chord (1.00 DF) | | |
|-----------------|---|------|------|
| | SP | DFL | SPF |
| 4 | 1422 | 1326 | 1200 |
| 6 | 948 | 884 | 800 |
| 8 | 711 | 663 | 600 |
| 10 | 568 | 530 | 480 |
| 12 | 474 | 442 | 400 |
| 14 | 406 | 378 | 342 |
| 16 | 355 | 331 | 300 |
| 18 | 316 | 294 | 266 |
| 20 | 284 | 265 | 240 |
| 22 | 258 | 241 | 218 |
| 24 | 237 | 221 | 200 |
| ** | | | |

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Refer to drawings 160A-Z for standard plate positions.

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ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcacomponents.com; ICC: www.lccsafe.org

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|--|-------------------------|-----|---------------------|
| S. ADDIOR S. | TC LL | PSF | REF SY42 Connection |
| C.C. | DL DL | PSF | DATE 01/19/2018 |
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| 09/24/2025 COA#0-278. Yoonhwak Kim. F | SBAGING7 | · | |

Florida Certificate of Product Approval #FL 1999



Camber may be built into trusses to compensate for the vertical deflection that results from the application of loads. Providing camber has the following advantages:

- Helps to ensure level ceilings and floors after dead loads are applied.
- Facilitates drainage to avoid ponding on flat or low slope roofs.
- Compensates for different deflection characteristics between adjacent trusses.
- Improves appearance of garage door headers and other long spans that can appear to "sag."
- Avoids "dips" in roof ridgelines at the transition from the gable to adjacent clear span trusses.

In accordance with ANSI/TPI 1 the Building Designer, through the Construction Documents, shall provide the location, direction, and magnitude of all loads attributable to ponding that may occur due to the design of the roof drainage system. The Building Designer shall also specify any dead load, live load, and in-service creep deflection criteria for flat or low-slope roofs subject to ponding loads.

The amount of camber is dependent on the truss type, span, loading, application, etceteras.

More restrictive limits for allowable deflection and slenderness ratio (L/D) may be required to help control vibration.

The following tables are provided as guidelines for limiting deflection and estimating camber. Conditions or codes may exist that require exceeding these recommendations, or past experience may warrant using more stringent limitations.

Commentary: Deflection and Camber

L = Span of Truss (inches)

D = Depth of Truss at Deflection Point (inches)

Recommended Truss Deflection Limits

| <u>Truss Type</u> | <u>L/D</u> | <u>Deflection Limits</u> | | | |
|---|------------|--------------------------|--------------------|--|--|
| | | <u>Live Load</u> | <u>Total Load</u> | | |
| Pitched Roof Trusses | 24 | L/240 (vertical) | L/180 (vertical) | | |
| Floor of Room-In-Attic Trusses | 24 | L/360 (vertical) | L/240 (vertical) | | |
| Flat or Shallow Pitched Roof Trusses | 24 | L/360 (vertical) | L/240 (vertical) | | |
| Residential Floor Trusses | 24 | L/360 (vertical) | L/240 (vertical) | | |
| Commercial Floor Trusses | 20 | L/480 (vertical) | L/240 (vertical) | | |
| Scissors Trusses | 24 | 0.75" (horizontal) | 1.25" (horizontal) | | |

Truss Type Recommended Camber

Pitched Trusses 1.00 x Deflection from Actual Dead Load

Sloping Parallel 1.5 x Vertical Deflection from

Chord Trusses Actual Dead Load

(0.25 x Deflection from Live Load) + Floor Trusses

Actual Dead Load

Flat Roof . Th 125 x Deflection from Live Load) + Mor Design Dead Load Deflection)

dead load may be considerably less than

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Alpine, a division of ITV Building Components Group Icc.

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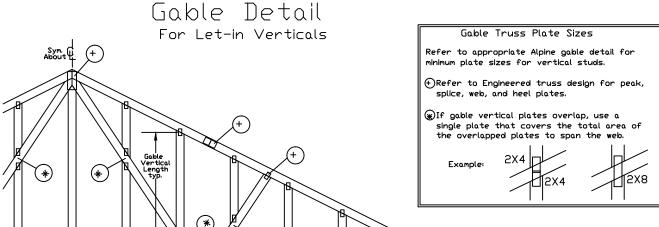


IREF DEFLEC/CAMB DATE 10/01/14

DRWG DEFLCAMB1014

09/24/2025 COA#0-278. Yoon<u>hwak Kim. FUPE #86367</u> Florida Certificate of Product Approval #FL 1999





Provide connections for uplift specified on the engineered truss design.

Attach each "T" reinforcing member with

End Driven Nails:

10d Common (0.148"x 3.",min) Nails at 4" o.c. plus

(4) nails in the top and bottom chords.

10d Common (0.148"x3".min) Toenails at 4" o.c. plus

(4) toenalls in the top and bottom chords.

This detail to be used with the appropriate Alpine gable detail for ASCE wind load.

ASCE 7-05 Gable Detail Drawings

A13015051014, A12015051014, A11015051014, A10015051014, A14015051014, A13030051014, A12030051014, A11030051014, A10030051014, A14030051014

ASCE 7-10 & ASCE 7-16 Gable Detail Drawings

A11515ENC100118, A12015ENC100118, A14015ENC100118, A14015ENC100118, A18015ENC100118, A20015ENC100118, A20015ENC100118, A20015ENC100118, A20015ENC100118, A1530ENC100118, A1530ENC100118, A1530ENC100118, A1530ENC100118, A20030ENC100118, A20030ENC1

\$18015ENC100118, \$20015ENC100118, \$20015END100110, \$20015PED100118

\$11530ENC100118, \$12030ENC100118, \$14030ENC100118, \$14000ENC100118 \$18030ENC100118, \$20030ENC100118, \$20030ENT100118, \$20030PED100118

See appropriate Alpine gable detail for maximum unneinforced gable vertical

"T" Reinforcement Attachment Detail



To convert from "L" to "T" reinforcing members, multiply "T" increase by length (based on appropriate Alpine gable detail).

Maximum allowable "T" reinforced gable vertical length is 14' from top to bottom chord.

"T" reinforcing member material must match size, specie, and grade of the "L" reinforcing member.

Web Length Increase w/ "T" Brace

| "T" Reinf. | "T" |
|------------|----------|
| Mbr. Size | Increase |
| 2×4 | 30 % |
| 2x6 | 20 % |

Example:

ASCE 7-10 Wind Speed = 120 mph Mean Roof Height = 30 ft, Kzt = 1.00 Gable Vertical = 24°o.c. SP #3

"T" Reinforcing Member Size = 2x4

"T" Brace Increase (From Above) = 30% = 1.30 (1) 2x4 "L" Brace Length = 8' 7"

Maximum "T" Reinforced Gable Vertical Length $1.30 \times 8' \ 7'' = 11' \ 2''$

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IREF LET-IN VERT DATE 01/02/2018 DRWG GBLLETIN0118

MAX. TOT. LD. 60 PSF

DUR. FAC. ANY 0024#0-278. Yoonhwak Kim, F**IMA***#8&**B**ACING 24.0"



Rigid Sheathing

Ceiling

4 Nails

Nails

Spaced At

4 Nails

Reinforcing

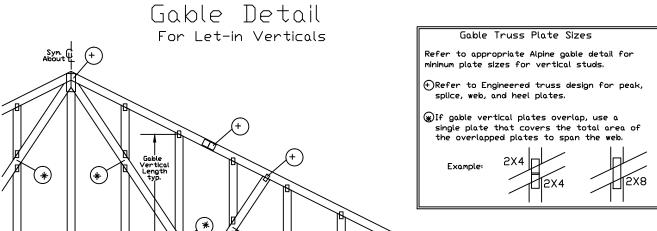
Member

Gable

Truss

155 Harlem Ave North Building, 4th Floor Glenview, IL 60025

Florida Certificate of Product Approval #FL 1999



Provide connections for uplift specified on the engineered truss design.

Attach each "T" reinforcing member with

End Driven Nails:

10d Common (0.148"x 3.", min) Nails at 4" o.c. plus

(4) nails in the top and bottom chords.

10d Common (0.148"x3".min) Toenails at 4" o.c. plus

(4) toenails in the top and bottom chords.

This detail to be used with the appropriate Alpine gable detail for ASCE wind load.

ASCE 7-05 Gable Detail Drawings

A13015051014, A12015051014, A11015051014, A10015051014, A14015051014, A13030051014, A12030051014, A11030051014, A10030051014, A14030051014

ASCE 7-10 Gable Detail Drawings

A11515ENC101014, A12015ENC101014, A14015ENC101014, A16015ENC101014, A18015ENC101014, A20015ENC101014, A20015END101014, A20015PED101014, A11530ENC101014, A12030ENC101014, A14030ENC101014, A16030ENC101014, A18030ENC101014, A20030ENC101014, A20030END101014, A20030PED101014 S11515ENC100815, S12015ENC100815, S14015ENC100815, S16015ENC100815, \$18015ENC100815, \$20015ENC100815, \$20015END100815, \$20015PED100815, \$11530ENC100815, \$12030ENC100815, \$14030ENC100815, \$16030ENC100815, \$18030ENC100815, \$20030ENC100815, \$20030END100815, \$20030PED100815

See appropriate Alpine gable detail for maximum unreinforced gable vertical length.

"T" Reinforcement Attachment Detail



To convert from "L" to "T" reinforcing members, multiply "T" increase by length (based on appropriate Alpine gable detail).

Maximum allowable "T" reinforced gable vertical length is 14' from top to bottom chord.

"T" reinforcing member material must match size, specie, and grade of the "L" reinforcing member.

Web Length Increase w/ "T" Brace

| "T" Reinf. | "T" |
|------------|----------|
| Mbr. Size | Increase |
| 2×4 | 30 % |
| 2x6 | 20 % |

Example:

ASCE 7-10 Wind Speed = 120 mph Mean Roof Height = 30 ft, Kzt = 1.00 Gable Vertical = 24°o.c. SP #3

'T' Reinforcing Member Size = 200111111

"T" Brace Increase (From Abo (1) 2x4 "L" Brace Length Maximum 'T' Reinforce Galle 1.30 x 8' 7'

MAX. TOT. LD. 60 PSF

DUR. FAC. ANY MAX. SPACING 24.0"



Rigid Sheathing

Ceiling

4 Nails

Nails

Spaced At

4 Nails

Reinforcing Member

Gable

Truss

13723 Riverport Drive Suite 200 Maryland Heights, MO 63043

VARNINGI READ AND FOLLOW ALL NOTES ON THIS DRAWING ***IMPORTANT*** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

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Alpine, a division of ITV Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.

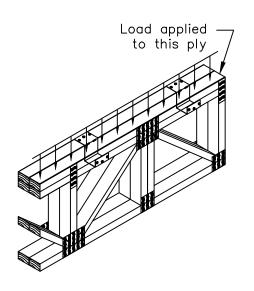
A seal on this drawing or cover page listing this drawing, indicates acceptance of professional

engineering responsibility solely for the design shown. The suitability and use of this for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see this job's general notes page and these web sites: ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcindustry.org; ICC: www.iccsafe.org

SY32/SY42 PLY TO PLY LSC CONNECTION DETAIL FOR DOWNWARD LOADS ONLY

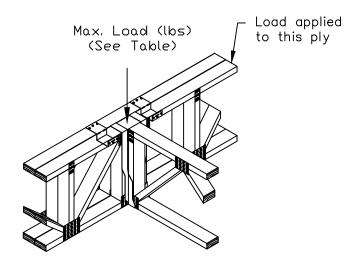
Uniform Load Application



| | T.C. Uni oad (pli | | Clip Spacing Along |
|-----|----------------------|--------|-----------------------|
| SP | DF | SPF/HF | Top Chord |
| 935 | 810 | 585 | 12 " o.c. |
| 625 | 540 | 390 | 18 " o.c. |
| 470 | 405 | 295 | 24 " o.c. |
| 375 | 325 | 235 | 30 ″ o.c. |

Maximum LSC spacing is 30" o.c.

Concentrated Load Application



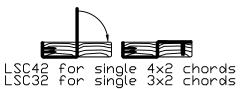
| | Max Load (lbs) | |
|------|-------------------|--------|
| SP | DF | SPF/HF |
| 1870 | 1620 | 1170 |

Note:

Install LSC adjacent, equidistant, CEA and not more than 6" on each side of concentrated locations

Installation Instructions:

- 1. Position and attach LSC to loaded ply with (3) 0.131"x1.5" nails into narrow face.
- 2. Bend clip over adjacent ply and attach with (3) $0.131' \times 1.5''$ nails into wide face.





LSC42-2 for stacked 4x2 chords LSC32-2 for stacked 3x2 chords

ALL

Refer to Alpine sealed drawing fan individual

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Refer to drawings 160A-2 for standard plate positions.

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AN ITW COMPANY

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ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org

REF SY42 Connection DATE 10/01/14 DRWG LSCSYX2A1014

DUR. FAC.

09/24/2025 COA#0-278, Yoonhwak Kim, FL PE #86 Florida Certificate of Product Approval #FL 1999



Piggyback Detail - ASCE 7-22: 180 mph, 30' Mean Hgt, Partially Enclosed, Exp. C, Kzt=1.00

180 mph Wind, 30.00 ft Mean Hgt, ASCE 7-22, Part. Enclosed Bldg. located anywhere in roof, Exp C, Wind DL= 5.0 psf (min), Kzt=1.0. Or 160 mph wind, 30.00 ft Mean Hgt, ASCE 7-22, Part Enclosed Bldg located anywhere in roof, Exp D, wind DL= 5.0 psf (min), Kzt=1.0.

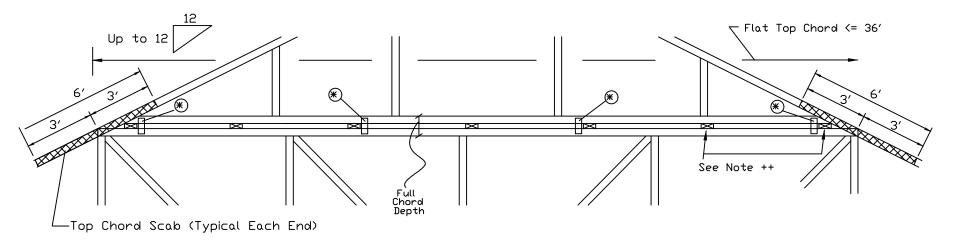
Note: Top chords of trusses supporting piggyback cap trusses must be adequately braced by sheathing or purlins. The building designer shall provide diagonal bracing or any other suitable anchorage to permanently restrain purlins, and lateral bracing for out of plane loads over gable ends.

Maximum truss spacing is 24" o.c. Detail is not applicable if cap supports additional loads such as cupola, steeple, chimney or drag strut loads.

** Refer to Engineer's sealed truss design drawing for piggyback and base truss specifications.

Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4" o.c.

++ Flat top chord purlins required at both ends and at a maximum of 24" intervals unless otherwise noted on base truss design drawing. Attach purlin bracing to the flat top chord using a minimum of (2) 16d box nails (0.135"x3.5").



* In addition, provide connection with one of the following methods: 28PB Wave Piggyback Plate

Use 3X8 Trulox plates for 2x4 chord member, and 3X10 Trulox plates for 2x6 and larger chord members. Attach to each face @ 8' o.c. with (4) 0.120"x1.375" nails into cap bottom chord and (4) in base truss top chord. Trulox plates may be staggered 4' o.c. front to back faces.

One 28PB wave piggyback plate to each face

APA Rated Gusset

8'x8'x7'16' (min) APA rated sheathing gussets (each face). Attach @ 8' o.c. with (8) 6d common (0.113'x2') nalls per gusset, (4) in cap bottom chord and (4) in base truss top chord. Gussets may be staggered 4' o.c. front to back faces.

One 28PB wave piggyback plate to each face 8 % o.c. Attach teeth to piggyback at time of fabrication. Attach to supporting truss with (4) 0.120°x1.375° nalls per face per ply. Piggyback plates may be staggered 4' o.c. front to back faces

2x4 Ver eal 3aas

2x4 Ver eal 3aas

2x4 SP #8, Pul. Plur d depth scale, (each face). Attach 8 % o.c. with (8) (0d box nalls (0.128°x3') per acab, (3) in cap bottom chord and (3) in base truss top chord. Scales may be staggered 4' o.c. front workand faces.

mainportantma furnish this draving to all contractors including the installers.

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Alpine, a division of ITW Building Components Group less chair.

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IREF **PIGGYBACK** DATE 07/03/2023 DRWG PB180220723 2024/2025/8, Yoonhwak Kim, FI SPE#861965 24.0"

Florida Certificate of Product Approval #FL 1999

Scabbed Piggyback Detail

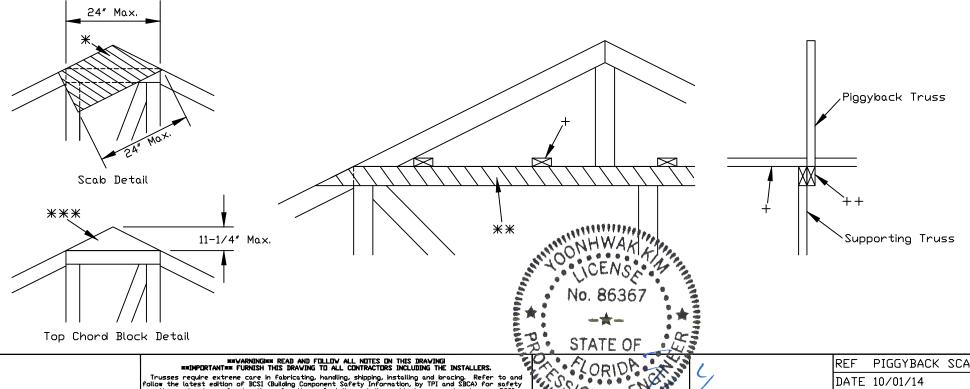
Note: Top chords of trusses supporting piggyback cap trusses must be adequately braced by sheathing or purlins. The building Engineer of Record shall provide diagonal bracing or any other suitable anchorage to permanently restrain purlins, and lateral bracing for out of plane loads over gable ends.

Refer to Engineer's sealed truss design drawing for piggyback and base truss specifications.

Maximum truss spacing is 24" o.c. Detail is not applicable if cap supports additional loads such as cupola, steeple, chimney or drag strut loads.

- * 2x4,2x6,2x8,2x10 or 2x12, #2 or #2 PT, SPF, Hem-Fir, or SP as needed. Attach scab to one face of truss with 16d (0.162"x3.5") common nails or 0.128"x3" gun nails @ 3" oc. throughout top chord.
- ** Piggyback as per ITWBCG drawing. Attach to top chord of supporting truss using either the attachment specified on ITWBCG drawing or with gun nails (0.131"x3"gun nails) at: 8" oc. for ASCE 7-10, 160 mph, Enc. Bldg, Exp B & C, 30" MH 6" oc. for ASCE 7-10, 180 mph, Part. Enc. Bldg, Exp B, 30" MH 6" oc. for ASCE 7-10, 180 mph, Enc. Bldg, Exp C, 30" MH 4" oc. for ASCE 7-10, 180 mph, Part. Enc. Bldg, Exp C, 30" MH
- *** Toenail block to top chord with 0.128"x3" gun nails @ 4" oc from each face throughout top chord.

- ++ Detail valid only for full overlap of piggyback chord as shown.
- + Continuous Lateral Bracing (CLB) as specified on the supporting truss ITWBGC drawing or at 24" o.c. if not specified. Attach to each supporting truss with (2) 16d (0.162"x3.5") common nails. Bracing material to be supplied and attached at both ends to a suitable support by erection contractor.
- + CLB lumber: 2x4 or 2x6 #2 or #2 PT, SPF, Hem-Fir, or SP. CLB may be applied to top edge of supporting truss top chord (as shown) or bottom edge of supporting truss top chord.



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Refer to drawings 160A-Z for standard plate positions.

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& 278, Yoonhwak Kim, ₽₽₽₽₩\$67 Florida Certificate of Product Approval #FL 1999 PIGGYBACK SCAB

DRWG PBSCAB101014

MAX, TOT, LD, 55 PSF DUR, FAC, 1.25

24.0"

Scabbed Piggyback Detail

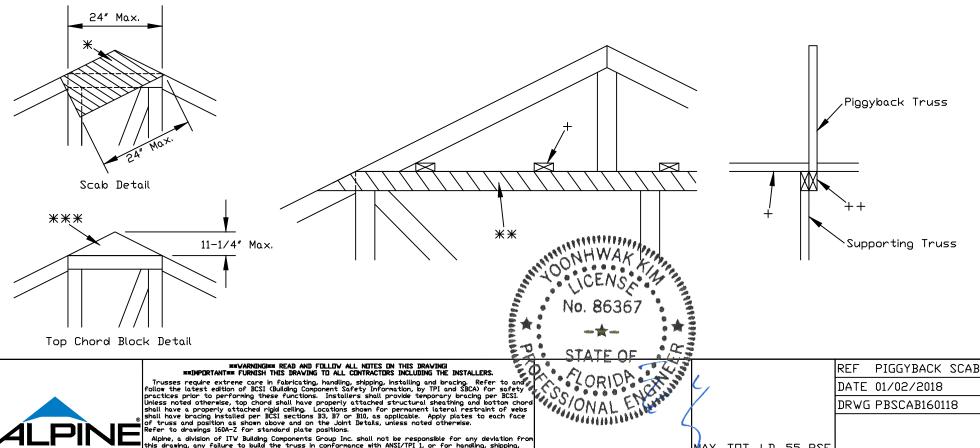
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Refer to Engineer's sealed truss design drawing for piggyback and base truss specifications.

Maximum truss spacing is 24" o.c. Detail is not applicable if cap supports additional loads such as cupola, steeple, chimney or drag strut loads.

- * 2x4,2x6,2x8,2x10 or 2x12, #2 or #2 PT, SPF, Hem-Fir, or SP as needed. Attach scab to one face of truss with 16d (0.162"x3.5") common nails or 0.128"x3" gun nails @ 3" oc. throughout top chord.
- ** Piggyback as per ITWBCG drawing. Attach to top chord of supporting truss using either the attachment specified on Alpine drawing or with gun nails (0.131'x3'gun nails) ati 8' oc. for ASCE 7-16, 160 mph, Enc. Bldg, Exp B & C, 30' MH 6° oc. for ASCE 7-16, 180 mph, Part. Enc. Bldg, Exp B, 30° MH 6° oc. for ASCE 7-16, 180 mph, Enc. Bldg, Exp C, 30° MH 4" oc. for ASCE 7-16, 180 mph, Part. Enc. Bldg, Exp C, 30' MH
- *** Toenail block to top chord with 0.128"x3" gun nails @ 4" oc from each face throughout top chord.

- ++ Detail valid only for full overlap of piggyback chord as shown.
- + Continuous Lateral Bracing (CLB) as specified on the supporting truss ITWBGC drawing or at 24" o.c. if not specified. Attach to each supporting truss with (2) 16d (0.162"x3.5") common nails. Bracing material to be supplied and attached at both ends to a suitable support by erection contractor.
- + CLB lumber: 2x4 or 2x6 #2 or #2 PT, SPF, Hem-Fir, or SP. CLB may be applied to top edge of supporting truss top chord (as shown) or bottom edge of supporting truss top chord.



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SPACING 24.0" Florida Certificate of Product Approval #FL 1999

MAX. TOT. LD. 55 PSF

1.25

DUR, FAC,

Scabbed Piggyback Detail

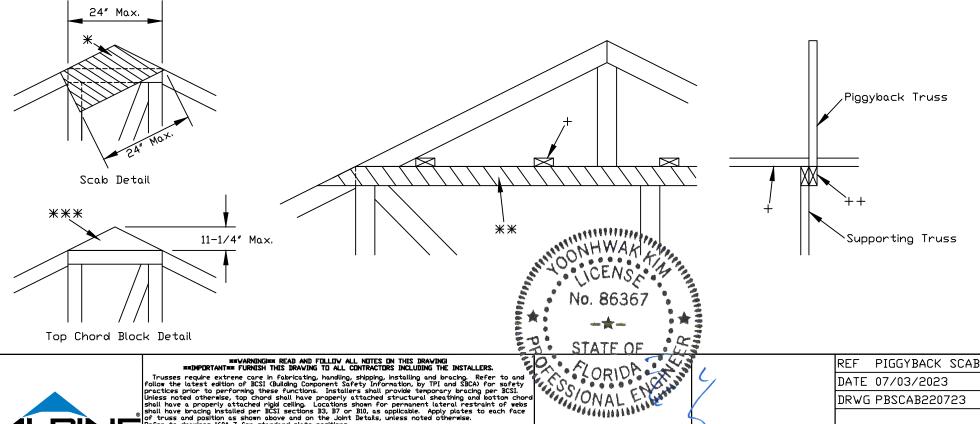
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Refer to Engineer's sealed truss design drawing for piggyback and base truss specifications.

Maximum truss spacing is 24" o.c. Detail is not applicable if cap supports additional loads such as cupola, steeple, chimney or drag strut loads.

- * 2x4,2x6,2x8,2x10 or 2x12, #2 or #2 PT, SPF, Hem-Fir, or SP as needed. Attach scab to one face of truss with 16d (0.162"x3.5") common nails or 0.128"x3" gun nails @ 3" oc. throughout top chord.
- ** Piggyback as per Alpine drawing. Attach to top chord of supporting truss using either the attachment specified on Alpine drawing or with gun nalls (0.131"x3"gun nalls) at: 8" oc. for ASCE 7-22, 160 mph, Enc. Bldg, Exp B & C, 30' MH 6' oc. for ASCE 7-22, 180 mph, Part. Enc. Bldg, Exp B, 30' MH 6' oc. for ASCE 7-22, 180 mph, Enc. Bldg, Exp C, 30' MH 4' oc. for ASCE 7-22, 180 mph, Part. Enc. Bldg, Exp C, 30' MH
- *** Toenail block to top chord with 0.128"x3" gun nails @ 4" oc from each face throughout top chord.

- ++ Detail valid only for full overlap of piggyback chord as shown.
- + Continuous Lateral Restraint (CLR) as specified on the supporting truss Alpine drawing or at 24" o.c. if not specified. Attach to each supporting truss with (2) 16d (0.162"x3.5") common nails. Bracing material to be supplied and attached at both ends to a suitable support by erection contractor.
- + CLR lumber: 2x4 or 2x6 #2 or #2 PT, SPF, Hem-Fir, or SP. CLR may be applied to top edge of supporting truss top chord (as shown) or bottom edge of supporting truss top chord.



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Refer to drawings 160A-Z for standard plate positions.

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Florida Certificate of Product Approval #FL 1999

MAX, TOT, LD, 55 PSF

1.25

24.0"

DUR, FAC,

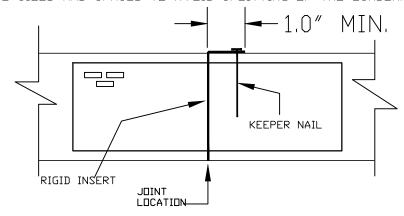
SPACING

RIGID INSERT DETAIL - REINFORCEMENT FOR HIGH STRESS COMPRESSION JOINTS

THIS DETAIL IS TO BE USED WHEN STRESS AT A COMPRESSION SPLICE EXCEEDS 75% OF THE ALLOWABLE COMPRESSION STRESS PER TPI 1 SECTION 7.3.9.2

OPTION #1:

APPLY A 20 GAGE MINIMUM METAL INSERT BETWEEN BUTTED ENDS OF COMPRESSION CHORD MEMBERS TO FULLY COVER THE JOINT BEARING AREA. BEND RIGID INSERT OVER THE TOP OR BOTTOM OF THE COMPRESSION MEMBER A MINIMUM OF 1" AND SECURE IN PLACE WITH A KEEPER NAIL. KEEPER NAIL IS TO BE SIZED AND SPACED TO AVOID SPLITTING OF THE LUMBER.

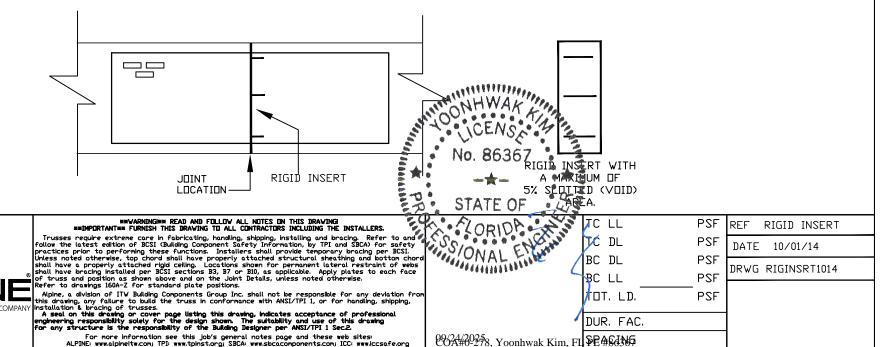


OPTION #2:

155 Harlem Ave North Building, 4th Floor

Glenview, IL 60025

APPLY A 20 GAGE MINIMUM METAL INSERT WITH SLOTTED TEETH BETWEEN BUTTED ENDS OF COMPRESSION CHORD MEMBERS TO FULLY COVER THE JOINT BEARING AREA. HAMMER RIGID INSERT SECURELY IN PLACE AND FLUSH WITH BUTTED END.



2624460278, Yoonhwak Kim, FI \$2461365 Florida Certificate of Product Approval #FL 1999

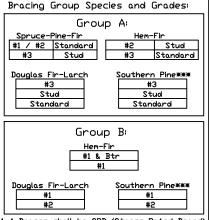
ASCE 7-10: 160 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Dr: 140 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00

Dr: 140 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 1.00

Or: 120 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

| 2x4 Brace No Braces Group A Group B Group A Gr | 13' 2" 13' 0" 13' 0" 13' 0" 13' 3" 13' 2" 13' 1" 13' 1" 13' 1" |
|--|--|
| Spacing Species Grade Braces Group A Group B Group B Group A Group B Group B Group A Group B Group B Group B Group A Group B | 13' 2" 13' 0" 13' 0" 13' 0" 13' 3" 13' 2" 13' 1" 13' 1" 13' 0" |
| TO C O THE H3 3' 2" 5' 8" 5' 10" 6' 8" 6' 11" 8' 0" 8' 4" 10' 6" 10' 11" 12' 6" | 13' 0" 13' 0" 13' 0" 13' 3" 13' 2" 13' 1" 13' 1" 13' 0" |
| Stud 3' 2" 5' 8" 5' 10" 6' 8" 6' 11" 8' 0" 8' 4" 10' 6" 10' 11" 12' 12' 12' 12' 12' 12' 12' 12' 12' 12' | 13' 0" 13' 0" 13' 3" 13' 2" 13' 1" 13' 1" 13' 0" |
| Study 3 2 3 8 3 10 6 8 6 11 8 0 8 4 10 6 10 11 12 6 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 | 13' 0" 13' 3" 13' 2" 13' 1" 13' 1" 13' 0" |
| 0 | 13' 3" 13' 2" 13' 1" 13' 1" 13' 0" |
| | 13' 2" 13' 1" 13' 1" 13' 0" |
| _ ` | 13′ 1″ 13′ 1″ 13′ 0″ |
| | 13′ 1″ 13′ 0″ |
| #3 3' 3" 5' 3" 5' 7" 6' 9" 7' 0" 8' 0" 8' 4" 10' 7" 11' 0" 12' 7" | 13′ 0″ |
| Stud 3' 3" 5' 3" 5' 7" 6' 9" 7' 0" 8' 0" 8' 4" 10' 7" 11' 0" 12' 7" | |
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| | 14′ 0″ 14′ 0″ |
| | 14′ 0″ 14′ 0″ |
| Standard 2' 9" 6' 1" 6' 5" 7' 9" 7' 11" 9' 1" 9' 6" 12' 0" 12' 6" 14' 0" | 14' 0" |
| 01 0 | |
| | 14′ 0″ 14′ 0″ |
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| $ \begin{array}{c ccccccccccccccccccccccccccccccccccc$ | 14' 0" |
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| Stud 4' 0" 7' 1" 7' 5" 8' 5" 8' 9" 10' 0" 10' 5" 13' 2" 13' 9" 14' 0" 5 tandard 4' 0" 7' 0" 7' 5" 8' 5" 8' 9" 10' 0" 10' 5" 13' 2" 13' 9" 14' 0" 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 | 14′ 0″ |
| 11 🗸 - | 14′ 0″ |
| | 14′ 0″ |
| $ \; \; \; \; \; \; \; \; \; \; \; \; \; \; \; \; \; \;$ | 14′ 0″ |
| [] | 14' 0" |
| Standard 4' 0" 6' 6" 6' 11" 8' 5" 8' 9" 10' 0" 10' 5" 13' 2" 13' 9" 14' 0" | 14′ 0″ |



1x4 Braces shall be SRB (Stress-Rated Board) **For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards, Group B values may be used with these grades.

Gable Truss Detail Notes: Wind Load deflection criterion is L/360.

Provide uplift connections for 75 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.

Attach "L" braces with 10d (0.128"x3.0" min) nails. ★ For (1) "L" brace: space nails at 2" o.c. in 18" end zones and 4" o.c. between zones. ₩₩For (2) "L" braces: space nails at 3" o.c. in 18" end zones and 6" o.c. between zones.

"L" bracing must be a minimum of 80% of web member length.

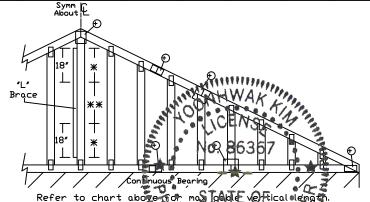
| Gable Vertical Plate | Sizes |
|--|-----------|
| Vertical Length | No Splice |
| Less than 4' 0" | 2X3 |
| Greater than 4' 0", but less than 11' 6" | 3X4 |
| Greater than 11' 6' | 4X4 |
| + Refer to common truss peak, splice, and heel pl | |

Refer to the Building Designer for conditions not addressed by this detail.

Gable Truss Diagonal brace option: vertical length may be doubled when diagonal brace is used. Connect diagonal brace for 600# at each end. Max web total length is 14'. 2x6 DF-L #2 or better diagonal brace; single Vertical length shown or double cut in table above. (as shown) at upper end. Connect diagonal at midpoint of vertical web.

155 Harlem Ave North Building, 4th Floor

Glenview, IL 60025



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engineering responsibility solely for the design shown. The sultability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see this job's general notes page and these web sites: ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org



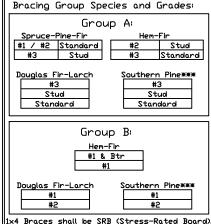
ASCE 7-16: 160 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Dr: 140 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00

Dr: 140 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 1.00

Or: 120 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

| | | | | | | | | | I | | | | 1 | |
|-------------------------|--------------|------------------|-----------------|----------------|----------------|----------------|--------------------------------|---|------------------|------------------|------------------|------------------------------------|------------------|------------------|
| | | 2x4 · Vertica | Brace | No | (1) 1×4 "L | " Brace * | (1) 2×4 *L | ." Brace * | (2) 2×4 *L | " Brace ** | (1) 2×6 *L | " Brace * | (5) 5×6 L | Brace ** |
| \parallel $_{<}$ | | Species | | Braces | Group A | Group B | Group A | Group B | Group A | Group B | Group A | Group B | Group A | Group B |
| | | CDE | #1 / #2 | 3′ 4″ | 5′ 9″ | 5′ 11″ | 6′ 9 ″ | 7′ 0″ | 8′ 1″ | 8′ 5 ″ | 10′ 7″ | 11′ 0″ | 12′ 8″ | 13′ 2″ |
| | 1.7 | SPF | #3 | 3′ 2″ | 5′ 8″ | 5′ 10 ″ | 6′ 8 ″ | 6′ 11″ | 8′ 0 ″ | 8′ 4″ | 10′ 6″ | 10′ 11″ | 12′ 6″ | 13′ 0″ |
| $\square \cong \square$ | Ų | HF | Stud | 3′ 2″ | 5′ 8″ | 5′ 10 ″ | 6′ 8 ″ | 6′ 11″ | 8′ 0 ″ | 8′ 4″ | 10′ 6″ | 10′ 11″ | 12′ 6″ | 13′ 0″ |
| > | 0 | 1 11 | Standard | 3′ 2″ | 4′ 11″ | 5′ 3″ | 6′ 7″ | 6′ 11″ | 8′ 0″ | 8′ 4″ | 10′ 4″ | 10′ 11″ | 12′ 6″ | 13′ 0″ |
| ا به اا | | | #1 | 3′ 6″ | 5′ 10″ | 6′ 0″ | 6′ 10 ″ | 7′ 1″ | 8′ 2″ | 8′ 6″ | 10′ 9″ | 11′ 2″ | 12′ 9″ | 13′ 3″ |
| - | * | SP | #2 | 3′ 4″ | 5′ 9″ | 5′ 11″ | 6′ 9 ″ | 7′ 0″ | 8′ 1″ | 8′ 5 ″ | 10′ 7″ | 11′ 0″ | 12′ 8″ | 13′ 2″ |
| | 4 | L | #3 | 3′ 3″ | 5′ 3″ | 5′ 7″ | 6′ 9″ | 7′ 0″ | 8′ 0 ″ | 8′ 4″ | 10′ 7″ | 11′ 0″ | 12′ 7″ | 13′ 1″ |
| ll d | N | IDFL | Stud | 3′ 3″ | 5′ 3″ | 5′ 7″ | 6′ 9 ″ | 7′ 0″ | 8′ 0 ″ | 8′ 4″ | 10′ 7″ | 11′ 0″ | 12′ 7″ | 13′ 1″ |
| $\Pi \cong \Pi$ | | | Standard | 3′ 2″ | 4′ 7″ | 4′ 11″ | 6′ 2″ | 6′ 7″ | 8′ 0″ | 8′ 4″ | 9′ 8″ | 10′ 4″ | 12′ 6″ | 13′ 0″ |
| ∏-≌ | | SPF | #1 / #2 | 3′ 10″ | 6′ 7″ | 6′ 10″ | 7′ 9″ | 8′ 1″ | 9′ 3″ | 9′ 7″ | 12′ 2″ | 12′ 8″ | 14′ 0″ | 14′ 0″ |
| $ +\rangle $ | | 12LL | #3 | 3′ 8″ | 6′ 6″ | 6′ 10 ″ | 7′ 8″ | 7′ 11″ | 9′ 1″ | 9′ 6″ | 12′ 0″ | 12′ 6″ | 14′ 0″ | 14′ 0″ |
| | Ų | l HF | Stud | 3′ 8″ | 6′ 6″ | 6′ 8″ | 7′ 8″ | 7′ 11″ | 9′ 1″ | 9′ 6″ | 12′ 0″ | 12′ 6″ | 14′ 0″ | 14′ 0″ |
| ΠŌ | Ō | 1 11 | Standard | 3′ 8″ | 6′ 1″ | 6′ 5″ | 7′ 8″ | 7′ 11″ | 9′ 1″ | 9′ 6″ | 12′ 0″ | 12′ 6″ | 14′ 0″ | 14′ 0″ |
| 🔍 | | | #1 | 4′ 0″ | 6′ 8″ | 6′ 11″ | 7′ 10″ | 8′ 2″ | 9′ 4″ | 9′ 8″ | 12′ 4″ | 12′ 9″ | 14′ 0″ | 14′ 0″ |
| / | * | SP | #2 | 3′ 10″ | 6′ 7″ | 6′ 10 ″ | 7′ 9″ | 8′ 1″ | 9′ 3″ | 9′ 7″ | 12′ 2″ | 12′ 8″ | 14′ 0″ | 14′ 0″ |
| | 9 | | #3 | 3′ 9″ | 6′ 5″ | 6′ 9″ | 7′ 8″ | 8′ 0″ | 9′ 2″ | 9′ 7″ | 12′ 1″ | 12′ 7″ | 14′ 0″ | 14′ 0″ |
| O | \vdash | DFL | Stud | 3′ 9″ | 6′ 5″ | 6′ 9″ | 7′ 8″ | 8′ 0″ | 9' 2" | 9′ 7″ | 12′ 1″ | 12′ 7″ | 14′ 0″ | 14′ 0″ |
| | | | Standard | 3′ 8″ | 5′ 8″ | 6′ 0″ | 7′ 6″ | 7′ 11″ | 9′ 1″ | 9′ 6″ | 11′ 10″ | 12′ 6″ | 14′ 0″ | 14′ 0″ |
| | | SPF | #1 / #2 | 4′ 3″ | 7′ 3″ | 7′ 6″ | 8′ 6″ | 8′ 10 ″ | 9′ 3″ | 10′ 7″ | 13′ 5″ | 13′ 11″ | · · | 14′ 0″ |
| ˈd | | | #3 | 4′ 0″ | 7′ 1″ | 7′ 5″ | 8′ 5 ″ | 8′ 9″ | 10′ 0″ | 10′ 5″ | 13′ 2″ | 13′ 9″ | 14′ 0″ | 14′ 0″ |
| [] U | Ų | l HF | Stud | 4′ 0″ | / 1 | <u> </u> | 8′ 5 ″ | 8′ 9 ″ 8′ 9 ″ | 10′ 0″ | 10′ 5″ | 13′ 2″ | 13′ 9″ | 14′ 0″ | 14′ 0″ |
| II I | 0 | <u> </u> | Standard #1 | 4′ 0″ | <u> </u> | 7′ 5″ | 8′ 5″ | 8′ 9 ″ 9′ 0 ″ | 10′ 0″ 10′ 3″ | 10′ 5″ 10′ 8″ | 13′ 2″ 13′ 6″ | 13′ 9 ″ 14′ 0 ″ | 14′ 0″ 14′ 0″ | 14′ 0″ 14′ 0″ |
| × | | SP | #2 | 4′ 5″ | | _ / / | 8′ 8″ | , , <u>, , , , , , , , , , , , , , , , , </u> | | 10′ 8″ 10′ 7″ | | | - · · | |
| M Q | * |) ST | #3 | 4′ 3″ 4′ 2″ | 7′ 3″ 7′ 2″ | 7′ 6″ 7′ 5″ | 8′ 6 ″ 8′ 6 ″ | 8′ 10″ 8′ 10″ | 10′ 2″ 10′ 1″ | 10' 6" | 13′ 5″ 13′ 4″ | 13′ 11 ″ 13′ 10 ″ | | 14′ 0″ 14′ 0″ |
| $ \Sigma $ | N | DFL | Stud | 4' 2" | 7' 2" | 7' 5" | 8' 6" | 8' 10" 8' 10" | 10' 1" | 10' 6" | 13' 4" | 13' 10" | 14' 0" | 14' 0" |
| | \leftarrow | שר ר | Standard | 4' 0" | 6' 6" | 6' 11" | 8' 5" | 8' 9" | 10' 1" | 10' 5" | 13' 4" | 13' 9" | 14' 0" | 14' 0" |
| | | 1 | Standard | <u> 4 U"</u> | 1 0 0. | p 11" | 0 J | | | 10 2 | 13 2" | 13 9 | 14 U" | 14 0" |
| | | | 1 11 | | M | | | Symr Abou | ŧĽ | | | | | |



**For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards, Group B values may be used with these grades.

Gable Truss Detail Notes: Wind Load deflection criterion is L/360.

Provide uplift connections for 75 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.

Attach "L" braces with 10d (0.128"x3.0" min) nails. ★ For (1) "L" brace: space nails at 2" o.c. in 18" end zones and 4" o.c. between zones. ₩₩For (2) "L" braces: space nails at 3" o.c. in 18" end zones and 6" o.c. between zones.

"L" bracing must be a minimum of 80% of web member length.

| Gable Vertical Plate | Sizes |
|--|-----------|
| Vertical Length | No Splice |
| Less than 4' 0" | 2X3 |
| Greater than 4' 0", but less than 11' 6" | 3X4 |
| Greater than 11' 6" | 4X4 |
| + Refer to common truss peak, splice, and heel pl | |

Refer to the Building Designer for conditions not addressed by this detail.

Gable Truss Diagonal brace option: vertical length may be doubled when diagonal brace is used. Connect diagonal brace for 600# at each end. Max web "L" Brace End total length is 14'. Zones, typ. 2x6 DF-L #2 or better diagonal brace; single Vertical length shown or double cut in table above. (as shown) at upper end. Connect diagonal at Refer to chart above for midpoint of vertical web.

m=IMPORTANT== FURNISH THIS DRAVING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by Tracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural structural sheating and botton chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, 87 or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise.

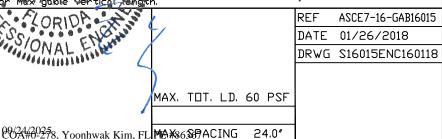
Alpine, a division of ITV Building Components Group Inc.

Alpine, a division of ITV Building Components Group Inc. shall not be responsible for any deviation from this drawing, any fallure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.

A seal on this drawing or cover page listing this drawing, indicates acceptance of professional

engineering responsibility solely for the design shown. The sultability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see this Job's general notes page and these web sites: ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org



Florida Certificate of Product Approval #FL 1999

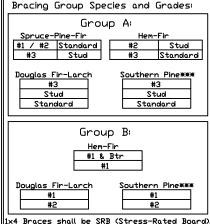
ASCE 7-10: 160 mph Wind Speed, 30' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Dr: 140 MPH Wind Speed, 30' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00

Dr: 140 mph Wind Speed, 30' Mean Height, Enclosed, Exposure D, Kzt = 1.00

Dr: 120 mph Wind Speed, 30' Mean Height, Partially Enclosed, Exposure D, Kzt = 1,00

| | | | | | | - 120 mpm | ,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,, | | i iig.i.v, i | u. v.u, _ | 1 | AP0341 C 2 | 1 112 0 | | г |
|------------|---------|------------------|----------|--------|----------------|---------------|--|---------------|---------------|---------------|--------------------|----------------|-----------|----------|---|
| | | 2x4 · Vertica | Brace | No | (1) 1×4 *L | " Brace * | (1) 2×4 *L | ." Brace * | (2) 2×4 L | " Brace ** | (1) 2x6 " L | " Brace * | (2) 2×6 L | Brace ** | |
| _ ا | Spacing | Species | Grade | Braces | Group A | Group B | Group A | Group B | Group A | Group B | Group A | Group B | Group A | Group B | |
| 1 | | CDE | #1 / #2 | 3′ 2″ | 5′ 6″ | 5′ 8 ″ | 6′ 5″ | 6′ 8 ″ | 7′ 8″ | 8′ 0 ″ | 10′ 1″ | 10′ 6″ | 12′ 0″ | 12′ 6″ | |
| | | SPF | #3 | 3′ 1″ | 5′ 5″ | 5′ 7″ | 6′ 4″ | 6′ 7″ | 7' 7" | 7′ 11″ | 10′ 0″ | 10′ 4″ | 11' 11" | 12′ 5″ | |
| D | Ų | HF | Stud | 3′ 1″ | 5′ 4″ | 5′ 7″ | 6′ 4″ | 6′ 7″ | 7' 7" | 7′ 11″ | 10′ 0″ | 10′ 4″ | 11' 11" | 12′ 5″ | |
| l a | 1 0 | | Standard | 3′ 1″ | 4′ 7″ | 4′ 11″ | 6′ 2″ | 6′ 7″ | 7' 7" | 7′ 11″ | 9′ 8″ | 10′ 4″ | 11′ 11″ | 12′ 5″ | |
| ן ש | _ | | #1 | 3′ 4″ | 5′ 6″ | 5′ 9″ | 6′ 6″ | 6′ 9″ | 7′ 9″ | 8′ 1 ″ | 10′ 3″ | 10′ 8″ | 12′ 2″ | 12′ 8″ | |
| $ \bot $ | * | SP | #2 | 3′ 2″ | 5′ 6 ″ | 5′ 8″ | 6′ 5″ | 6′ 8″ | 7′ 8″ | 8′ 0 ″ | 10′ 1″ | 10′ 6″ | 12′ 0″ | 12′ 6″ | |
| | 4 | | #3 | 3′ 2″ | 4′ 10″ | 5′ 2″ | 6′ 5″ | 6′ 8″ | 7′ 8″ | 7′ 11″ | 10′ 1″ | 10′ 5″ | 12′ 0″ | 12′ 5″ | |
| | N | IDFI | Stud | 3′ 2″ | 4′ 10″ | 5′ 2″ | 6′ 5 ″ | 6′ 8″ | 7′ 8″ | 7′ 11″ | 10′ 1″ | 10′ 5″ | 12′ 0″ | 12′ 5″ | |
| d | _ ` - | | Standard | 3′ 1″ | 4′ 3″ | 4′ 7″ | 5′ 8″ | 6′ 1″ | 7' 7" | 7′ 11″ | 8′ 11″ | 9′ 7″ | 11′ 11″ | 12′ 5″ | |
| \perp | | CDE | #1 / #2 | 3′ 8″ | 6′ 3″ | 6′ 6″ | 7′ 5″ | 7′ 8″ | 8′ 9 ″ | 9′ 2″ | 11′ 7″ | 12′ 0 ″ | 13′ 9″ | 14′ 0″ | |
| == | - | SPF | #3 | 3′ 6″ | 6′ 2″ | 6′ 6″ | 7′ 3″ | 7' 7" | 8′ 8 ″ | 9′ 1″ | 11′ 5″ | 11′ 11″ | 13′ 7″ | 14′ 0″ | |
| 1 ? | U | HF | Stud | 3′ 6″ | 6′ 2″ | 6′ 5″ | 7′ 3″ | 7′ 7″ | 8′ 8 ″ | 9′ 1″ | 11′ 5″ | 11′ 11″ | 13′ 7″ | 14′ 0″ | |
| 1 2 | Ιō | 1 11 | Standard | 3′ 6″ | 5′ 8 ″ | 6′ 0″ | 7′ 3″ | 7' 7" | 8′ 8 ″ | 9′ 1″ | 11′ 5″ | 11′ 11″ | 13′ 7″ | 14′ 0″ | |
| 1 🖑 | | | #1 | 3′ 10″ | 6′ 4″ | 6′ 7″ | 7′ 6″ | 7′ 9″ | 8′ 11″ | 9′ 3″ | 11′ 9″ | 12′ 2″ | 13′ 11″ | 14′ 0″ | |
| > | | l SP | #2 | 3′ 8″ | 6′ 3″ | 6′ 6″ | 7′ 5″ | 7′ 8″ | 8′ 9 ″ | 9′ 2″ | 11′ 7″ | 12′ 0″ | 13′ 9″ | 14′ 0″ | |
| | 9 | | #3 | 3′ 7″ | 5′ 11″ | 6′ 4″ | 7′ 4″ | 7' 7" | 8′ 9 ″ | 9′ 1″ | 11′ 6″ | 11' 11" | 13′ 8″ | 14′ 0″ | |
| lω | 16 | IDFL | Stud | 3′ 7″ | 5′ 11″ | 6′ 4″ | 7′ 4″ | 7' 7" | 8′ 9 ″ | 9′ 1″ | 11′ 6″ | 11′ 11″ | 13′ 8″ | 14′ 0″ | |
| | | | Standard | 3′ 6″ | 5′ 3″ | 5′ 7″ | 7′ 0″ | 7′ 6″ | 8′ 8 ″ | 9′ 1″ | 11′ 0″ | 11′ 9″ | 13′ 7″ | 14′ 0″ | |
| Gabl | | CDE | #1 / #2 | 4′ 1″ | 6′ 11″ | 7′ 2″ | 8′ 2″ | 8′ 5 ″ | 8′ 9 ″ | 10′ 1″ | 12′ 9″ | 13′ 3″ | 14′ 0″ | 14′ 0″ | |
| O | - | SPF | #3 | 3′ 10″ | 6′ 9″ | 7′ 0″ | 8′ 0 ″ | 8′ 4″ | 9′ 7″ | 9′ 11″ | 12′ 7″ | 13′ 1″ | 14′ 0″ | 14′ 0″ | |
| 0 | Ų | HF | Stud | 3′ 10″ | 6′ 9″ | 7′ 0″ | 8′ 0 ″ | 8′ 4″ | 9′ 7″ | 9′ 11″ | 12′ 7″ | 13′ 1″ | 14′ 0″ | 14′ 0″ | |
| | lō | 1 11 | Standard | 3′ 10″ | 6′ 6″ | 6′ 11″ | 8′ 0″ | 8′ 4″ | 9′ 7″ | 9′ 11″ | 12′ 7″ | 13′ 1″ | 14′ 0″ | 14′ 0″ | |
| X | | | #1 | 4′ 3″ | 7′ 0″ | 7′ 3″ | 8′ 3″ | 8′ 6 ″ | 9′ 9″ | 10′ 2″ | 12′ 11 ″ | 13′ 5″ | 14′ 0″ | 14′ 0″ | |
| 12 | | SP | #2 | 4′ 1″ | 6′ 11″ | 7′ 2″ | 8′ 2″ | 8′ 5 ″ | 9′ 8″ | 10′ 1″ | 12′ 9″ | 13′ 3″ | 14′ 0″ | 14′ 0″ | |
| Α | ù | l | #3 | 3′ 11″ | 6′ 10 ″ | 7′ 1″ | 8′ 1″ | 8′ 5 ″ | 9′ 7″ | 10′ 0″ | 12′ 8″ | 13′ 2″ | 14′ 0″ | 14′ 0″ | |
| _ | 15 | IDFL | Stud | 3′ 11″ | 6′ 10″ | /′ 1″ | 8′ 1″ | 8′ 5 ″ | 9′ 7″ | 10′ 0″ | 12′ 8″ | 13′ 2″ | 14′ 0″ | 14′ 0″ | |
| | | | Standard | 3′ 10″ | 6′ 1″ | 6′ 5″ | 8′ 0 ″ | 8′ 4 ″ | 9′ 7″ | 9′ 11″ | 12′ 7″ | 13′ 1″ | 14′ 0″ | 14′ 0″ | |



**For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards, Group B values may be used with these grades.

Gable Truss Detail Notes: Wind Load deflection criterion is L/360.

Provide uplift connections for 135 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.

Attach "L" braces with 10d (0.128"x3.0" min) nails. ★ For (1) "L" brace: space nails at 2" o.c. in 18" end zones and 4" o.c. between zones. ₩¥For (2) "L" braces: space nails at 3" o.c. in 18" end zones and 6" o.c. between zones.

"L" bracing must be a minimum of 80% of web member length.

| Vertical Length | No Splice |
|---|-----------|
| Less than 4' 0" | 2X4 |
| Greater than 4' 0", but less than 12' 0" | 4X4 |

Refer to the Building Designer for conditions not addressed by this detail.

Diagonal brace option: vertical length may be doubled when diagonal brace is used. Connect diagonal brace for 690# at each end. Max web total length is 14'. 2x6 DF-L #2 or better diagonal brace; single Vertical length shown or double cut in table above. (as shown) at upper end. Connect diagonal at

Gable Truss

Symm C **″**L**″** Brace Refer to chart above

VARNINGI READ AND FOLLOW ALL NOTES ON THIS DRAWINGI ****IMPORTANT*** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and macing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI unless noted otherwise, top chord shall have properly attached structural sheathing and botton chord shall have a properly attached rigid celling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. Applicable to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions.

Alpine, a division of ITV Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.

A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see this job's general notes page and these web sites: ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org

ASCE7-10-GAB16030 DATE 8/3/15 DRWG S16030ENC100815 MAX. TOT. LD. 60 PSF COA#6-278, Yoonhwak Kim, FIMAX#868BACING 24.0"

Florida Certificate of Product Approval #FL 1999

midpoint of vertical web.

ASCE 7-16: 160 mph Wind Speed, 30' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Dr: 140 MPH Wind Speed, 30' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00

Dr: 140 mph Wind Speed, 30' Mean Height, Enclosed, Exposure D, Kzt = 1.00 Dr: 120 mph Wind Speed, 30' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

| | | | | | | · IEO MPRI | wirid spee | ea, so mea | ri neigrit, i | -ur dutty E | nciosea, E. | xposure D | , KZt - 1.00 | <u>, </u> |
|------------|------|------------------|----------|----------------|---------------|---------------|---------------|---------------|---------------|---------------|-------------|-----------|--------------|--|
| | | 2x4 · Vertica | Brace | No | (1) 1×4 "L | " Brace * | (1) 2×4 *L | " Brace * | (2) 2×4 *L | " Brace ** | (1) 2×6 *L | " Brace * | (2) 2x6 L | Brace ** |
| _ | 1 | Species | Grade | | Group A | Group B | Group A | Group B | Group A | Group B | Group A | Group B | Group A | Group B |
| 7 | | CDE | #1 / #2 | 3′ 2″ | 5′ 6 ″ | 5′ 8″ | 6′ 5 ″ | 6′ 8 ″ | 7′ 8″ | 8′ 0 ″ | 10′ 1″ | 10′ 6″ | 12′ 0″ | 12′ 6″ |
| | 1 | SPF | #3 | 3′ 1″ | 5′ 5″ | 5′ 7″ | 6′ 4″ | 6′ 7″ | 7′ 7″ | 7′ 11″ | 10′ 0″ | 10′ 4″ | 11′ 11″ | 12′ 5″ |
| <u>ا</u> ق | Ų | HF | Stud | 3′ 1″ | 5′ 4″ | 5′ 7″ | 6′ 4″ | 6′ 7″ | 7′ 7″ | 7′ 11″ | 10′ 0″ | 10′ 4″ | 11′ 11″ | 12′ 5″ |
| | Ιo | | Standard | 3′ 1″ | 4′ 7″ | 4′ 11″ | 6′ 2″ | 6′ 7″ | 7' 7" | 7′ 11″ | 9′ 8″ | 10′ 4″ | 11′ 11″ | 12′ 5″ |
| 1 0 | _ | | #1 | 3′ 4″ | 5′ 6″ | 5′ 9″ | 6′ 6″ | 6′ 9″ | 7′ 9″ | 8′ 1″ | 10′ 3″ | 10′ 8″ | 12′ 2″ | 12′ 8″ |
| $ \bot $ | | SP | #2 | 3′ 2″ | 5′ 6″ | 5′ 8″ | 6′ 5 ″ | 6′ 8″ | 7′ 8″ | 8′ 0 ″ | 10′ 1″ | 10′ 6″ | 12′ 0″ | 12′ 6″ |
| | 4 | | #3 | 3′ 2″ | 4′ 10″ | 5′ 2″ | 6′ 5″ | 6′ 8″ | 7′ 8″ | 7′ 11″ | 10′ 1″ | 10′ 5″ | 12′ 0″ | 12′ 5″ |
| | là | IDFI I | Stud | 3′ 2″ | 4′ 10″ | 5′ 2″ | 6′ 5″ | 6′ 8″ | 7′ 8″ | 7′ 11″ | 10′ 1″ | 10′ 5″ | 12′ 0″ | 12′ 5″ |
| d | _ `` |] | Standard | 3′ 1″ | 4′ 3″ | 4′ 7″ | 5′ 8″ | 6′ 1″ | 7' 7" | 7′ 11″ | 8′ 11″ | 9′ 7″ | 11′ 11″ | 12′ 5″ |
| <u> </u> | | | #1 / #2 | 3′ 8″ | 6′ 3″ | 6′ 6″ | 7′ 5″ | 7′ 8″ | 8′ 9″ | 9′ 2″ | 11′ 7″ | 12′ 0″ | 13′ 9″ | 14′ 0″ |
| == | _ | SPF | #3 | 3′ 6″ | 6′ 2″ | 6′ 6″ | 7′ 3″ | 7′ 7″ | 8′ 8″ | 9′ 1″ | 11′ 5″ | 11′ 11″ | 13′ 7″ | 14′ 0″ |
| 1 (| U | HF | Stud | 3′ 6″ | 6′ 2″ | 6′ 5″ | 7′ 3″ | 7' 7" | 8′ 8″ | 9′ 1″ | 11′ 5″ | 11′ 11″ | 13′ 7″ | 14′ 0″ |
| <u>a</u> | lō | ПГ | Standard | 3′ 6″ | 5′ 8 ″ | 6′ 0 ″ | 7′ 3″ | 7' 7" | 8′ 8 ″ | 9′ 1″ | 11′ 5″ | 11′ 11″ | 13′ 7″ | 14′ 0″ |
| ΙΨ | | | #1 | 3′ 10 ″ | 6′ 4″ | 6′ 7″ | 7′ 6″ | 7′ 9″ | 8′ 11″ | 9′ 3″ | 11′ 9″ | 12′ 2″ | 13′ 11″ | 14′ 0″ |
| > | | SP | #2 | 3′ 8″ | 6′ 3″ | 6′ 6″ | 7′ 5″ | 7′ 8″ | 8′ 9 ″ | 9′ 2″ | 11′ 7″ | 12′ 0″ | 13′ 9″ | 14′ 0″ |
| | 9 | | #3 | 3′ 7″ | 5′ 11″ | 6′ 4″ | 7′ 4″ | 7' 7" | 8′ 9 ″ | 9′ 1″ | 11′ 6″ | 11' 11" | 13′ 8″ | 14′ 0″ |
| lω | 1 | DFL | Stud | 3′ 7″ | 5′ 11″ | 6′ 4″ | 7′ 4″ | 7' 7" | 8′ 9 ″ | 9′ 1″ | 11′ 6″ | 11′ 11″ | 13′ 8″ | 14′ 0″ |
| | | | Standard | 3′ 6″ | 5′ 3″ | 5′ 7″ | 7′ 0″ | 7′ 6″ | 8′ 8 ″ | 9′ 1″ | 11′ 0″ | 11′ 9″ | 13′ 7″ | 14′ 0″ |
| abl | | SPF | #1 / #2 | 4′ 1″ | 6′ 11″ | 7′ 2″ | 8′ 2″ | 8′ 5 ″ | 8′ 9 ″ | 10′ 1″ | 12′ 9″ | 13′ 3″ | 14′ 0″ | 14′ 0″ |
| O | l | | #3 | 3′ 10″ | 6′ 9″ | 7′ 0″ | 8′ 0 ″ | 8′ 4″ | 9′ 7″ | 9′ 11″ | 12′ 7″ | 13′ 1″ | 14′ 0″ | 14′ 0″ |
| 0 | U | HF | Stud | 3′ 10″ | 6′ 9″ | 7′ 0″ | 8′ 0 ″ | 8′ 4″ | 9′ 7″ | 9′ 11″ | 12′ 7″ | 13′ 1″ | 14′ 0″ | 14′ 0″ |
| | Ιō | 1 11 | Standard | 3′ 10″ | 6′ 6″ | 6′ 11″ | 8′ 0 ″ | 8′ 4″ | 9′ 7″ | 9′ 11″ | 12′ 7″ | 13′ 1″ | 14′ 0″ | 14′ 0″ |
| × | | | #1 | 4′ 3″ | 7′ 0″ | 7′ 3″ | 8′ 3″ | 8′ 6″ | 9′ 9″ | 10′ 2″ | 12′ 11″ | 13′ 5″ | 14′ 0″ | 14′ 0″ |
| | | SP | #2 | 4′ 1″ | 6′ 11″ | 7′ 2″ | 8′ 2″ | 8′ 5″ | 9′ 8″ | 10′ 1″ | 12′ 9″ | 13′ 3″ | 14′ 0″ | 14′ 0″ |
| Α | ù | L | #3 | 3′ 11″ | 6′ 10″ | / 1" | 8′ 1″ | 8′ 5 ″ | 9′ 7″ | 10′ 0″ | 12′ 8″ | 13′ 2″ | 14′ 0″ | 14′ 0″ |
| 1 | 15 | IDFL | Stud | 3′ 11″ | 6′ 10″ | / 1" | 8′ 1″ | 8′ 5″ | 9′ 7″ | 10′ 0″ | 12′ 8″ | 13′ 2″ | 14′ 0″ | 14′ 0″ |
| | | | Standard | 3′ 10″ | 6′ 1″ | 6′ 5″ | 8′ 0 ″ | 8′ 4″ | 9′ 7″ | 9′ 11″ | 12′ 7″ | 13′ 1″ | 14′ 0″ | 14′ 0″ |

Bracing Group Species and Grades: Group A: Spruce-Pine-Fir Hem-Fir #1 / #2 Standard #2 Stud #3 Stud #3 Standard Douglas Fir-Larch Southern Pine*** #3 Stud Stud Standard Standard Group B: Hem-Fir #1 & Btr Douglas Fir-Larch Southern Pine*** #1 #1 #2 1x4 Braces shall be SRB (Stress-Rated Board)

**For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards, Group B values may be used with these grades.

Gable Truss Detail Notes: Wind Load deflection criterion is L/360.

Provide uplift connections for 135 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.

Attach "L" braces with 10d (0.128"x3.0" min) nails. ★ For (1) "L" brace: space nails at 2" o.c. in 18" end zones and 4" o.c. between zones. ₩¥For (2) "L" braces: space nails at 3" o.c. in 18" end zones and 6" o.c. between zones.

"L" bracing must be a minimum of 80% of web member length.

| Vertical Length | No Splice |
|---|-----------|
| Less than 4' 0" | 2X4 |
| Greater than 4' 0", but less than 12' 0" | 4X4 |

peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.

Symm C Gable Truss Diagonal brace option: vertical length may be doubled when diagonal brace is used. Connect diagonal brace for 690# at each end. Max web "L" Brace End total length is 14'. Zones, typ. 2x6 DF-L #2 or better diagonal brace; single Vertical length shown or double cut in table above. (as shown) at upper end. Continuous Bearing Connect diagonal at Refer to chart above for most akile midpoint of vertical web.

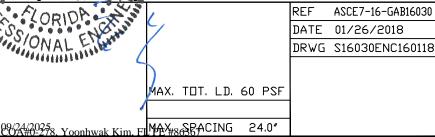
VARNINGI READ AND FOLLOW ALL NOTES ON THIS DRAWING ***IMPORTANT*** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

Trusses require extreme care in Suravina III ALL Cunival Into Intolling Into Refer to drawings 160A-Z for standard plate positions.

Alpine, a division of ITV Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.

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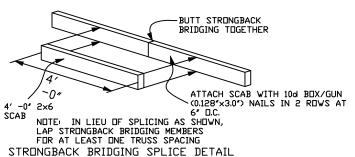
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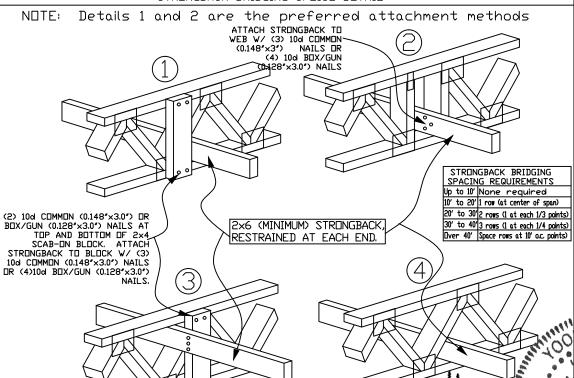


Florida Certificate of Product Approval #FL 1999

STRONGBACK BRIDGING RECOMMENDATIONS

ATTACH SARD TO BOTTO CHORD WITH (2) #10 - 3" SCREWS





- ► All scab-on blocks shall be a minimum 2x4 "stress graded lumber."
- ► All strongback bridging and bracing shall be a minimum 2x6 "stress graded lumber."
- ► The purpose of strongback bridging is to develop load sharing between individual trusses, resulting in an overall increase in the stiffness of the floor system. 2x6 strongback bridging, positioned as shown in details, is recommended at 10' -0" o.c. (max.)
- The terms "bridging" and "bracing" are sometimes mistakenly used interchangeably. "Bracing" is an important structural requirement of any floor or roof system. Refer to the Truss Design Drawing (TDD) for the bracing requirements for each individual truss component. "Bridging," particularly "strongback bridging" is a recommendation for a truss system to help control vibration. In addition to aiding in the distribution of point loads between adjacent truss, strongback bridging serves to reduce "bounce" or residual vibration resulting from moving point loads, such as footsteps.

The performance of all floor systems are enhanced by the installation of strongback bridging and therefore is strongly recommended by Alpine.

For additional information regarding strongback bridging, refer to BCSI (Building Component Safety Information).

STRONGBACK BRIDGING ATTACHMENT ALTERNATIVES

ANITW COMPANY Adpine, a division of ITV Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, in the standard process of the season of the

For more information see this job's general notes page and these web sites: ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org

TC LL **PSF IREF** STRONGBACK 1€/DL **PSF** DATE 10/01/14 ISC DI DRWG STRBRIBR1014 BC LL **PSF** ITØT. LD. **PSF** DUR. FAC. 1.00 SPACING

Florida Certificate of Product Approval #FL 1999



Valley Detail - ASCE 7-10: 160 mph, 30' Mean Height, Enclosed, Exp. C, Kzt=1.00

Top Chord 2x4 SP #2N, SPF #1/#2, DF-L #2 or better.

Bot Chord 2x4 SP #2N or SPF #1/#2 or better.

Webs 2x4 SP #3, SPF #1/#2, DF-L #2 or better.

*** Attach each valley to every supporting truss with:
(2) 16d box (0.135" x 3.5") nails toe-nailed for
ASCE 7-10 160 mph. 30' Mean Height, Enclosed
Building, Exp. C, Wind TC DL=5 psf, Kzt = 1.00
Dr
ASCE 7-10 140 mph. 30' Mean Height, Enclosed
Building, Exp. D, Wind TC DL=5 psf, Kzt = 1.00

Bottom chord may be square or pitched cut as shown.

Valleys short enough to be cut as solid triangular members from a single 2x6, or larger as required, shall be permitted in lieu of fabricating from separate 2x4 members.

All plates shown are ITW BCG Wave Plates.

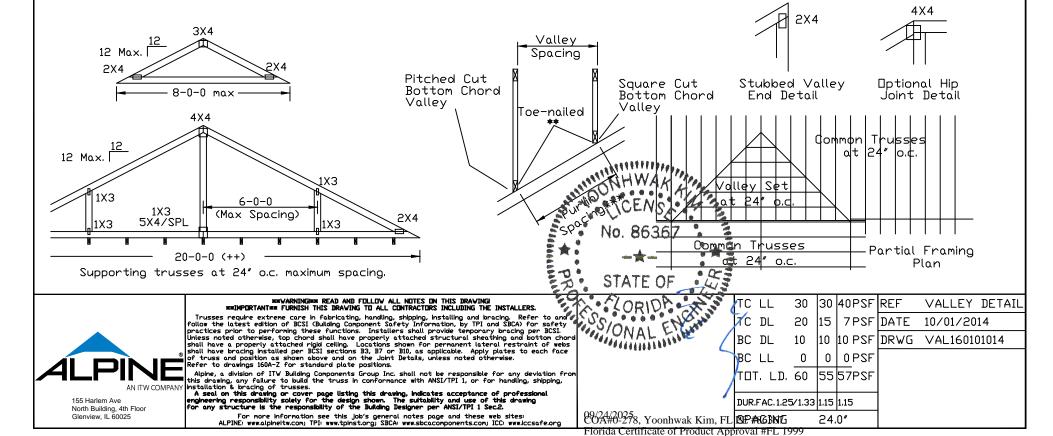
Unless specified otherwise on engineer's sealed design, for vertical valley webs taller than 7-9" apply 2x4 "T" reinforcement, 80% length of web, same species and grade or better, attached with 10d box (0.128" \times 3.0") nails at 6" o.c. In lieu of "T" reinforcement, 2x4 Continuous Lateral Restraint applied at mid-length of web is permitted with diagonal bracing as shown in DRWG BRCLBANC1014.

Top chord of truss beneath valley set must be braced with properly attached, rated sheathing applied prior to valley truss installation.

Purlins at 24" o.c. or as otherwise specified on engineer's sealed design

By valley trusses used in lieu of purlin spacing as specified on Engineer's sealed design.

- *** Note that the purlin spacing for bracing the top chord of the truss beneath the valley is measured along the slope of the top chord.
- ++ Larger spans may be built as long as the vertical height does not exceed 14'-0''.



Valley Detail - ASCE 7-16: 180 mph, 30' Mean Height, Partially Enclosed, Exp. C, Kzt=1.00

Top Chord 2x4 SP #2N, SPF #1/#2, DF-L #2 or better. Bot Chord 2x4 SP #2N or SPF #1/#2 or better. Webs 2x4 SP #3, SPF #1/#2, DF-L #2 or better.

** Attach each valley to every supporting truss with: 535# connection or with (1) Simpson H2.5A or equivalent connector for

ASCE 7-16 180 mph. 30' Mean Height, Part. Enc. Building, Exp. C, Wind TC DL=5 psf, Kzt = 1.00 Πr

ASCE 7-16 160 mph. 30' Mean Height, Part. Enc. Building, Exp. D, Wind TC DL=5 psf, Kzt = 1.00

Bottom chord may be square or pitched cut as shown.

Valleys short enough to be cut as solid triangular members from a single 2x6, or larger as required, shall be permitted in lieu of fabricating from separate 2x4 members.

All plates shown are Alpine Wave Plates.

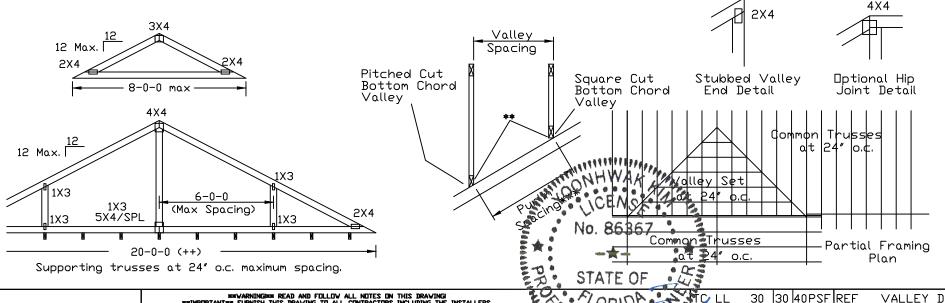
Unless specified otherwise on engineer's sealed design, for vertical valley webs taller than 7-9" apply 2x4 "T" reinforcement, 80% length of web, same species and grade or better, attached with 10d box (0.128" x 3.0") nails at 6" o.c. In lieu of "T" reinforcement, 2x4 Continuous Lateral Restraint applied at mid-length of web is permitted with diagonal bracina as shown in DRWG BRCLBANC1014.

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By valley trusses used in lieu of purlin spacing as specified on Engineer's sealed design.

- *** Note that the purlin spacing for bracing the top chord of the truss beneath the valley is measured along the slope of the top chord.
- ++ Larger spans may be built as long as the vertical height does not exceed 14'-0''.





155 Harlem Ave North Building, 4th Floor Glenview, IL 60025

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Alpine, a division of ITV Building Components Group Inc. shall not be responsible for any deviation from this drawing, any fallure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.

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| SIMILOF | 100 | | | | | | |
|--------------------------------|--------------|--------|------|--------|------|----------|--------|
| SORIDA XX | TC LL | 30 | 30 | 40PSF | REF | VALLEY | DETAIL |
| SS CONTRACTOR | TC DL | 20 | 15 | 7PSF | DATE | 01/26/20 | 018 |
| MAL | BC DL | 10 | 10 | 10 PSF | DRWG | VAL18016 | 50118 |
| | BC LL | 0 | 0 | 0 PSF | | | |
| | TO/T. LD. | 60 | 55 | 57PSF | | | |
| | | | | | | | |
| | DUR.FAC.1.25 | 5/1.33 | 1.15 | 1.15 | | | |
| 09/24/2025 8, Yoonhwak Kim, | SPAE ING | 67 | 24. | 0" | | | |

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Valley Detail - ASCE 7-22: 180 mph, 30' Mean Height, Partially Enclosed, Exp. C, Kzt=1.00

Top Chord 2x4 SP #2N, SPF #1/#2, DF-L #2 or better.

Bot Chord 2x4 SP #2N or SPF #1/#2 or better.

Webs 2x4 SP #3, SPF #1/#2, DF-L #2 or better.

** Attach each valley to every supporting truss with: 535# connection or with (1) Simpson H2.5A or equivalent connector for ASCE 7-22 180 mph. 30' Mean Height, Part. Enc.

ASCE 7-22 180 mph. 30' Mean Height, Part. Enc. Building, Exp. C, Wind TC DL=5 psf, Kzt = 1.00

Or

ASCE 7-22 160 mph. 30' Mean Height, Part. Enc. Building, Exp. D, Wind TC DL=5 psf, Kzt = 1.00

Bottom chord may be square or pitched cut as shown.

Valleys short enough to be cut as solid triangular members from a single 2x6, or larger as required, shall be permitted in lieu of fabricating from separate 2x4 members.

All plates shown are Alpine Wave Plates.

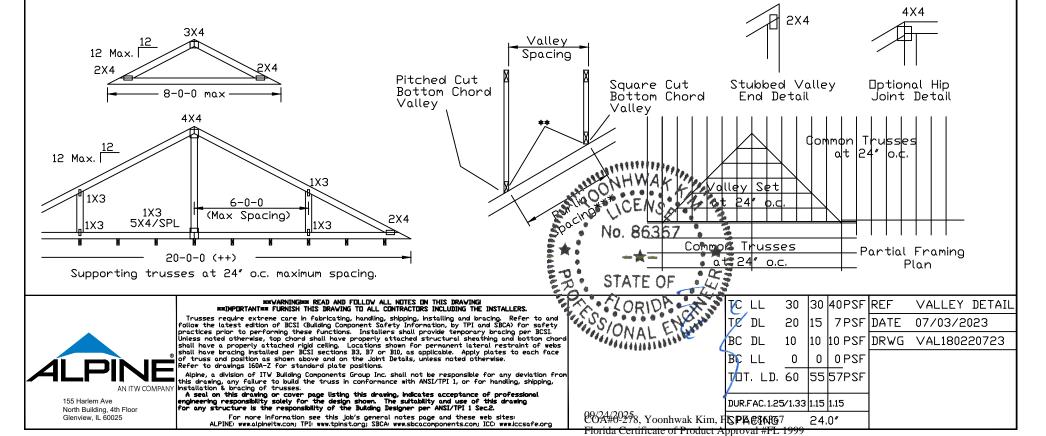
Unless specified otherwise on engineer's sealed design, for vertical valley webs taller than 7-9" apply 2x4 "T" reinforcement, 80% length of web, same species and grade or better, attached with 10d box (0.128" \times 3.0") nails at 6" o.c. In lieu of "T" reinforcement, 2x4 Continuous Lateral Restraint applied at mid-length of web is permitted with diagonal bracing as shown in DRWG BRCLBANC1014.

Top chord of truss beneath valley set must be braced with properly attached, rated sheathing applied prior to valley truss installation.

Purlins at 24" o.c. or as otherwise specified on engineer's sealed design $\ensuremath{\mathsf{\Pi r}}$

By valley trusses used in lieu of purlin spacing as specified on Engineer's sealed design.

- *** Note that the purlin spacing for bracing the top chord of the truss beneath the valley is measured along the slope of the top chord.
- ++ Larger spans may be built as long as the vertical height does not exceed 14'-0''.



Valley Detail - ASCE 7-16: 30' Mean Height, Enclosed, Exp. C, Kzt=1.00

Top Chord 2x4 SP #2N, SPF #1/#2, DF-L #2 or better. Bot Chord 2x4 SP #2N or SPF #1/#2 or better. Webs 2x4 SP #3, SPF #1/#2, DF-L #2 or better.

** Attach each valley to every supporting truss with: (2) 16d box $(0.135" \times 3.5")$ nails toe-nailed for ASCE 7-16, 30' Mean Height, Enclosed Building, Exp. C. Wind TC DL=5 psf, Kzt = 1.00, Max. Wind Speed based on supporting truss material at connection location: 170 mph for SP (G = 0.55, min.), 155 mph for DF-L (G = 0.50, min.), or

120 mph for HF & SPF (G = 0.42, min.).

Maximum top chord pitch is 10/12 for supporting trusses below valley trusses.

Bottom chord of valley trusses may be square or pitched cut as shown.

Valleys short enough to be cut as solid triangular members from a single 2x6, or larger as required, shall be permitted in lieu of fabricating from separate 2x4 members.

All plates shown are Alpine Wave Plates.

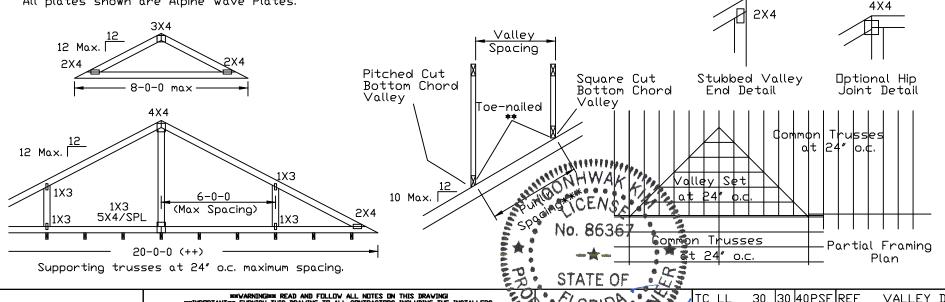
Unless specified otherwise on engineer's sealed design, for vertical valley webs taller than 7-9" apply 2x4 "T" reinforcement, 80% length of web, same species and grade or better, attached with 10d box $(0.128" \times 3.0")$ nails at 6" o.c. In lieu of "T" reinforcement, 2x4 Continuous Lateral Restraint applied at mid-length of web is permitted with diagonal bracing as shown in DRWG BRCLBANC1014.

Top chord of truss beneath valley set must be braced with: properly attached, rated sheathing applied prior to valley truss installation.

Purlins at 24" o.c. or as otherwise specified on engineer's sealed design

By valley trusses used in lieu of purlin spacing as specified on Engineer's sealed design.

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- ++ Larger spans may be built as long as the vertical height does not exceed 14'-0''.





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| 2. MOR | 10A | 3 / | TC | LL | 30 | 30 | 40PSF | REF | VALLEY | DETAIL |
|---------------------------------------|--|--------|-------------|----------|--------|------|--------|------|----------|--------|
| SS | O WELL | , | ₹C | DL | 20 | 15 | 7PSF | DATE | 01/26/20 | 18 |
| UNA | The state of the s | | BC | DL | 10 | 10 | 10 PSF | DRWG | VALTN16 | 0118 |
| - 2004611 | 18800 | 5 | ВC | LL | 0 | 0 | 0 PSF | | | |
| | | | דםד | LD. | 60 | 55 | 57PSF | | | |
| | | 1 | DUR. | FAC.1.25 | 5/1.33 | 1.15 | 1.15 | | | |
| 685 448 6 25 8, | Yoonhwak Kim | , FL I | ₽ ₽₩ | %§¶AC | | 24. | 0" | | | |
| | | | | | | | | | | |

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Valley Detail - ASCE 7-22: 30' Mean Height, Enclosed, Exp. C, Kzt=1.00

Top Chord 2x4 SP #2N, SPF #1/#2, DF-L #2 or better. Bot Chord 2x4 SP #2N or SPF #1/#2 or better. Webs 2x4 SP #3, SPF #1/#2, DF-L #2 or better.

** Attach each valley to every supporting truss with: (2) 16d box (0.135" \times 3.5") nails toe-nailed for ASCE 7-22, 30' Mean Height, Enclosed Building, Exp. C. Wind TC DL=5 psf, Kzt = 1.00, Max. Wind Speed based on supporting truss material at connection location: 140 mph for SP (G = 0.55, min.), 125 mph for DF-L (G = 0.50, min.), or

105 mph for HF & SPF (G = 0.42, min.).

Maximum top chord pitch is 10/12 for supporting trusses below valley trusses.

Bottom chord of valley trusses may be square or pitched cut as shown.

Valleys short enough to be cut as solid triangular members from a single 2x6, or larger as required, shall be permitted in lieu of fabricating from separate 2x4 members.

All plates shown are Alpine Wave Plates.

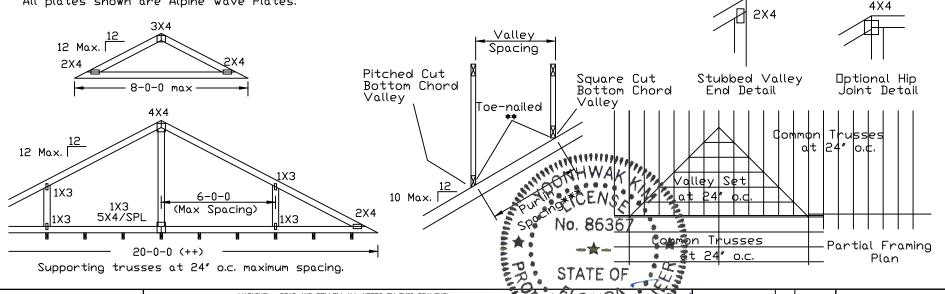
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Top chord of truss beneath valley set must be braced with: properly attached, rated sheathing applied prior to valley truss installation.

Purlins at 24" o.c. or as otherwise specified on engineer's sealed design

By valley trusses used in lieu of purlin spacing as specified on Engineer's sealed design.

- *** Note that the purlin spacing for bracing the top chord of the truss beneath the valley is measured along the slope of the top chord.
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155 Harlem Ave North Building, 4th Floor Glenview, IL 60025

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Alpine, a division of ITV Building Components Group Inc. Shall have the standard plate positions.

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| CORIDA COSTO | TC LL | 30 | 30 | 40PSF | REF | VALLEY | DETAIL |
|--|----------------|--------|------|--------|------|----------|--------|
| SONAL ENLY | TC DL | 20 | 15 | 7PSF | DATE | 07/03/20 | 23 |
| The state of the s | BC DL | 10 | 10 | 10 PSF | DRWG | VALTN22 | 20723 |
| | BC LL | 0 | 0 | 0 PSF | | | · |
| | TOT. LD. | 60 | 55 | 57PSF | | | |
| | DUR.FAC.1.25 | i/1.33 | 1.15 | 1.15 | | | |
| 09/24/2025 COA#6-278, Yoonhwak Kim, FL I | EFACTOG | 10 | 24. | 0" | | | |
| 1 ionaa eerimeate of 1 iodaet Appi | Oval TIL 17. | ,, | | | | | |

NAIL REINFORCEMENT FOR PLATE DETAIL

This detail may be used when the required number of connector teeth in a truss member exceeds the actual number of teeth in the given member. Nails may be driven through the connector plate(s) in truss members using the nail type specified below to increase the lateral resistance of the connector plate(s) having a shortage of required effective teeth.

Nail Type

Duo-Fast CS157 1.5" x 0.105" Smooth Shank Coil Nail

Material Connector Plates Substitution SP, DF-L, HF, SPF Alpine 20-gage Wave 1 Nail replaces 2 teeth (SG >= 0.42)Alpine 20-gage H 1 Nail replaces 2 teeth Alpine 18-gage S 1 Nail replaces 2 teeth

Nail Type

0.131" x 1.5" Box or Gun Nail

Material Connector Plates Substitution SP, DF-L, HF, SPF Alpine 20-gage Wave 1 Nail replaces 3 teeth $(SG \ge 0.42)$ Alpine 20-gage H 1 Nail replaces 3 teeth Alpine 18-gage S 1 Nail replaces 3 teeth

Nails shall be driven through solid steel at a distance no closer that 1 inch to the joint line. Nails may also be driven through the tooth slots, but shall be located at the end of the slot farthest from the joint line.

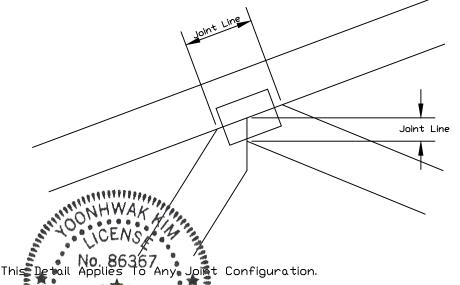
Use a maximum of 2 nails per square inch of plate area.

Nails shall be driven through connector plate having shortage of required effective teeth. Nails may be driven through connector plate on each face of joint if each plate has a shortage of required effective teeth.

Nails shall be fully embedded into wood areas free of defects.

This detail is applicable only on joints where the connector plate has been offset no more than 1/2" from the design position shown on the truss design drawing.

This detail is applicable for trusses with lumber oriented vertically (2x_) and trusses with lumber oriented horizontally (3x2 and 4x2)



Typical Joint Configurations. This Detail Applies

This Detail Is Applicable Only On Joints Where The Connector Plate Has Been tion Shows on The Truss Design Drawing. Offset No More Than 1/2" From The Design Pos

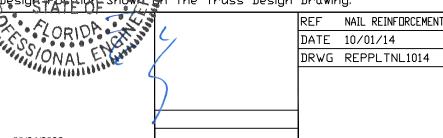
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Refer to drawings 160A-Z for standard plate positions.

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Gable Stud Reinforcement Detail for Stucco Cladding ASCE 7-10: 200 mph Wind Speed, 30' Mean Height, Enclosed, Exposure D, Kzt = 1.00

Dr: 200 mph Wind Speed, 30' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00 Dr: 180 mph Wind Speed, 30' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

| | | 2x4 Vertico | Brace | No | (1) 1×4 "L | " Brace * | (1) 2×4 *L | " Brace * | (2) 2×4 L | Brace ** | (1) 2×6 L | ." Brace * | (2) 2x6 *L | Brace ** |
|---------------------------|----------------|----------------|----------|------------------|---------------|---------------|----------------|---------------|----------------|---------------|---------------|----------------|------------|----------|
| \parallel $_{-}$ | Spacing | Species | Grade | Braces | Group A | Group B | Group A | Group B | Group A | Group B | Group A | Group B | Group A | Group B |
| 구 | | CDE | #1 / #2 | 2' 6" | 4′ 4″ | 4′ 6″ | 5′ 1 ″ | 5′ 3 ″ | 6′ 1″ | 6′ 4″ | 8′ 0 ″ | 8′ 3 ″ | 9′ 6″ | 9′ 10″ |
| ll 'o | 1.7 | SPF | #3 | 5 * 2` | 3′ 9″ | 4′ 0″ | 5′ 0 ″ | 5′ 3 ″ | 6′ 0 ″ | 6′ 3 ″ | 7′ 10″ | 8′ 2 ″ | 9′ 5″ | 9′ 9″ |
| II 27 | Ų | HF | Stud | 5 * | 3′ 9″ | 4′ 0″ | 5′ 0 ″ | 5′ 3 ″ | 6′ 0 ″ | 6′ 3 ″ | 7′ 10″ | 8′ 2″ | 9′ 5″ | 9′ 9″ |
| | | 1 11 | Standard | 5 2` | 3′ 3″ | 3′ 5″ | 4′ 4″ | 4′ 7″ | 5′ 10 ″ | 6′ 3 ″ | 6′ 9″ | 7′ 3″ | 9′ 2″ | 9′ 9″ |
| به | | | #1 | 8 " | 4′ 4″ | 4′ 6″ | 5' 2 " | 5′ 4″ | 6′ 1″ | 6′ 4″ | 8′ 1″ | 8′ 5 ″ | 9′ 7″ | 10′ 0″ |
| \square | * | SP | #2 | 2 6 " | 4′ 3″ | 4′ 6″ | 5′ 1 ″ | 5′ 3″ | 6′ 1″ | 6′ 4 ″ | 8′ 0 ″ | 8′ 3 ″ | 9′ 6″ | 9′ 10″ |
| | 4 | l | #3 | 2′ 6″ | 3′ 5″ | 3′ 7″ | 4′ 6″ | 4′ 10″ | 6′ 0 ″ | 6′ 3 ″ | 7′ 1″ | 7′ 7″ | 9′ 5″ | 9′ 10″ |
| | α | IDFL | Stud | 2′ 6″ | 3′ 5″ | 3′ 7″ | 4′ 6″ | 4′ 10″ | 6′ 0 ″ | 6′ 3″ | 7′ 1″ | 7′ 7″ | 9′ 5″ | 9′ 10″ |
| <u>č</u> | , - | | Standard | 2′ 4″ | 3′ 0″ | 3′ 2″ | 4′ 0″ | 4′ 3″ | 5′ 5″ | 5′ 9″ | 6′ 3″ | 6′ 8″ | 8′ 6″ | 9′ 1″ |
| II <u>.</u> | | CDE | #1 / #2 | 2′ 11″ | 4′ 11″ | 5′ 1 ″ | 5′ 10 ″ | 6′ 1″ | 6′ 11″ | 7′ 3″ | 9′ 2″ | 9′ 6″ | 10′ 10″ | 11′ 4″ |
| +> | - | SPF | #3 | 2′ 9″ | 4′ 7″ | 5′ 1 ″ | 5′ 9 ″ | 6′ 0″ | 6′ 10 ″ | 7′ 2″ | 9′ 0″ | 9′ 4″ | 10′ 9″ | 11′ 2″ |
| | U | HF | Stud | 2′ 9″ | 4′ 7″ | 4′ 11″ | 5′ 9 ″ | 6′ 0″ | 6′ 10 ″ | 7′ 2″ | 9′ 0″ | 9′ 4″ | 10′ 9″ | 11′ 2″ |
| Πà | Ō | 1 11 | Standard | 2′ 9″ | 3′ 11″ | 4′ 2″ | 5′ 3″ | 5′ 7″ | 6′ 10 ″ | 7′ 2″ | 8′ 3 ″ | 8′ 10 ″ | 10′ 9″ | 11′ 2″ |
| $\mathbb{N}^{\mathbb{Z}}$ | | | #1 | 3′ 0 ″ | 5′ 0 ″ | 5′ 2″ | 5′ 11 ″ | 6′ 1″ | 7′ 0″ | 7′ 3″ | 9′ 3″ | 9′ 7″ | 11′ 0″ | 11′ 5″ |
| > | * | SP | #2 | 2′ 11″ | 4′ 11″ | 5′ 1 ″ | 5′ 10 ″ | 6′ 1″ | 6′ 11″ | 7′ 3″ | 9′ 2″ | 9′ 6″ | 10′ 10″ | 11′ 4″ |
| | 9 | l | #3 | 2′ 10″ | 4′ 2″ | 4′ 5″ | 5′ 6 ″ | 5′ 11″ | 6′ 11″ | 7′ 2″ | 8′ 8 ″ | 9′ 3″ | 10′ 10″ | 11′ 3″ |
| IJω | 16 | IDFL | Stud | 2′ 10″ | 4′ 2″ | 4′ 5″ | 5′ 6″ | 5′ 11″ | 6′ 11″ | 7′ 2″ | 8′ 8″ | 9′ 3″ | 10′ 10″ | 11′ 3″ |
| IJ <u></u> | | | Standard | 2′ 9″ | 3′ 8″ | 3′ 11″ | 4′ 11″ | 5′ 3″ | 6′ 7″ | 7′ 1″ | 7′ 8″ | 8′ 3 ″ | 10′ 5″ | 11′ 2″ |
| [] 요 | | SPF | #1 / #2 | 3′ 2 ″ | 5′ 5″ | 5′ 7″ | 6′ 5 ″ | 6′ 8″ | 6′ 11″ | 7′ 11″ | 10′ 1″ | 10′ 5″ | 11′ 11″ | 12′ 5″ |
| ෮ | . . | 722 | #3 | 3′ 0″ | 5′ 4″ | 5′ 7″ | 6′ 4″ | 6′ 7″ | 7′ 6″ | 7′ 10″ | 9′ 11″ | 10′ 4″ | 11′ 10″ | 12′ 4″ |
| 0 | U | HF | Stud | 3′ 0 ″ | 5′ 4″ | 5′ 7″ | 6′ 4″ | 6′ 7″ | 7′ 6″ | 7′ 10″ | 9′ 11″ | 10′ 4″ | 11′ 10″ | 12′ 4″ |
| | Ō | 1 11 | Standard | 3′ 0″ | 4′ 7″ | 4′ 10″ | 6′ 1″ | 6′ 6″ | 7′ 6″ | 7′ 10″ | 9′ 6″ | 10′ 2″ | 11′ 10″ | 12′ 4″ |
| $H \times$ | | | #1 | 3′ 4″ | 5′ 6″ | 5′ 8 ″ | 6′ 6″ | 6′ 9″ | 7′ 8″ | 8′ 0 ″ | 10′ 2″ | 10′ 7″ | 12′ 1″ | 12′ 7″ |
| | * | SP | #2 | 3′ 2″ | 5′ 5″ | 5′ 7 ″ | 6′ 5″ | 6′ 8″ | 7′ 8″ | 7′ 11″ | 10′ 1″ | 10′ 5″ | 11′ 11″ | 12′ 5″ |
| M | ù | n = 1 | #3 | 3′ 1″ | 4′ 10″ | 5′ 1″ | 6′ 4″ | 6′ 7″ | <u> 7′ 7″</u> | 7′ 11″ | 10′ 0″ | 10′ 4″ | 11' 11" | 12′ 4″ |
| | 1, | DFL | Stud | 3′ 1″ | 4′ 10″ | 5′ 1″ | 6′ 4″ | 6′ 7″ | 7′ 7″ | 7′ 11″ | 10′ 0″ | 10′ 4″ | 11′ 11″ | 12′ 4″ |
| | | | Standard | 3′ 0 ″ | 4′ 3″ | 4′ 6″ | 5′ 8″ | 6′ 0″ | 7′ 6″ | 7′ 10″ | 8′ 10″ | 9′ 6″ | 11′ 10″ | 12′ 4″ |

Bracing Group Species and Grades: Group A: Spruce-Pine-Fir Hem-Fir #1 / #2 Standard #2 Stud #3 Stud #3 Standard Douglas Fir-Larch Southern Pine*** #3 Stud Stud Standard Standard Group B: Hem-Fir #1 & Btr Douglas Fir-Larch Southern Pine*** #1 #1 #2

1x4 Braces shall be SRB (Stress-Rated Board). ****For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes: Wind Load deflection criterion is L/360.

Provide uplift connections for 275 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.

Attach "L" braces with 10d (0.128"x3.0" min) nails.

For (1) "L" brace: space nails at 2" o.c.
in 18" end zones and 3" o.c. between zones.

##For (2) "L" braces: space nails at 3" o.c.
in 18" end zones and 6" o.c. between zones.

 $^{\prime}\text{L}^{\prime}$ bracing must be a minimum of 80% of web member length.

| Vertical Length | No Splice |
|--|-----------|
| Less than 3' 0" | 3X4 |
| Greater than 3' 0", but less than 6' 0" | 4X4 |

peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.

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Gable Truss Diagonal brace option: vertical length may be doubled when diagonal brace is used. Connect diagonal brace for 1000# at each end. Max web total length is 10'. 2x6 DF-L #2 or better diagonal brace; single Vertical length shown or double cut in table above. (as shown) at upper end. Connect diagonal at midpoint of vertical web.

Symm Chout Let 18' **

Is' **

Considerable Bearing Refer to chart above to max gable vertical temps.

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING ***IMPORTANT*** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, birstalling and BSAC) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid celling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise.

Refer to drawings 160A-Z for standard plate positions.

Alpine, a division of ITV Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.

A seal on this drawing or cover page listing this drawing, indicates acceptance of professional

A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see this job's general notes page and these web sites: ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org

MAX. TOT. LD. 60 PSF

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