

GENERAL STRUCTURAL NOTES:

- ALL WORK SHALL CONFORM TO THE 2023 8th EDITION OF THE FLORIDA (INTERNATIONAL) RESIDENTIAL CODE AND APPLICABLE COUNTY CODE AMENDMENTS. STANDARDS LISTED IN STRUCTURAL NOTE SECTIONS REFER TO THE VERSION AND EFFECTIVE DATE IDENTIFIED IN THE REFERENCED STANDARDS CHAPTER IN THE GOVERNING BUILDING CODE NOTED ABOVE.
- THE STRUCTURAL NOTES ARE INTENDED TO AUGMENT THE DRAWINGS. SHOULD CONFLICTS EXIST BETWEEN THE DRAWINGS AND THE STRUCTURAL NOTES, THE STRICTEST PROVISION SHALL GOVERN.
- THE STRUCTURAL DRAWINGS FORM AN INTEGRAL PART OF CONTRACT DOCUMENTS, WHICH INCLUDE ARCHITECTURAL, STRUCTURAL, MECHANICAL, ELECTRICAL, AND CIVIL/SITE DRAWINGS. COORDINATE THE STRUCTURAL DRAWINGS WITH THE REQUIREMENTS SHOWN IN THE OTHER COMPONENTS OF THE CONTRACT DOCUMENTS.
- SUBMIT DEFERRED SUBMITTAL SHOP DRAWINGS FOR REVIEW BY THE ENGINEER AS INDICATED IN MATERIAL SECTION OF GENERAL NOTES. REVIEW OF SHOP DRAWINGS & OTHER SUBMITTALS BY THE ENGINEER DOES NOT RELIEVE THE CONTRACTOR OF THE RESPONSIBILITY TO CHECK THE SHOP DRAWINGS PRIOR TO SUBMITTAL. ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF SHOP DRAWINGS NOT CONFORMING TO THE CONSTRUCTION DOCUMENTS ARE THE RESPONSIBILITY OF THE SHOP DRAWING PREPARER.
- TYPICAL DETAILS AND OTHER SECTIONS/DETAILS APPLY TO CONDITIONS THAT ARE SIMILAR TO THE CONDITIONS DESCRIBED IN THE SECTIONS/DETAILS, EVEN IF THEY ARE NOT SPECIFICALLY REFERENCED ON THE PLANS.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR MEANS, METHODS SEQUENCES AND PROCEDURES OF CONSTRUCTION, INCLUDING BUT NOT LIMITED TO TEMPORARY SHORING AND BRACING.
- DO NOT SCALE STRUCTURAL DRAWINGS. IF DIMENSIONS ARE OMITTED OR NOT CLEAR, CONTACT THE ENGINEER.
- CONSTRUCTION SHALL COMPLY FULLY WITH THE APPLICABLE PROVISIONS OF OSHA AND THE LOCAL GOVERNING CODES, CURRENT EDITION, AND ALL REQUIREMENTS SPECIFIED IN THE CODES SHALL BE ADHERED TO AS IF THEY WERE CALLED FOR OR SHOWN ON THE DRAWINGS. THIS SHALL NOT BE CONSTRUED TO MEAN THAT REQUIREMENTS SET FORTH ON THE DRAWING MAY BE MODIFIED BECAUSE THEY ARE MORE STRINGENT THAN THE CODE REQUIREMENTS OR BECAUSE THEY ARE NOT SPECIFICALLY REQUIRED BY CODE.
- THE ENGINEER'S SEAL APPLIES ONLY TO THE STRUCTURAL COMPONENTS ON THIS DOCUMENT. ALL DIMENSIONS ARE TO BE VERIFIED WITH THE ARCHITECTURAL DRAWINGS, ANY DEVIATIONS OR DISCREPANCIES ON THESE PLANS MUST BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE ENGINEER OF RECORD. BACK CHARGES WILL NOT BE ACCEPTED, REGARDLESS OF FAULT, WITHOUT PRIOR NOTIFICATION BY BUILDER WITHIN 48-HOURS AND INVESTIGATION BY FIELDSTONE.
- THE TERMS 'CONCRETE SCREW' AND 'MASONRY SCREW' SHALL BE USED INTERCHANGEABLY WITHIN THESE DRAWINGS. CONCRETE/MASONRY SCREWS AS GENERICALLY SPECIFIED WITHIN THESE DRAWINGS SHALL REFER TO ONE OF THE FOLLOWING ACCEPTED MANUFACTURER PRODUCTS: SIMPSON STRONG TIE TITEN TURBO, GENUINE TAPCON, OR HILTI KWIK-CON II.
- CONTENT LINKED WITHIN QR CODES THAT MAY BE SHOWN WITH THE DRAWING SET ARE FOR INFORMATIONAL PURPOSES ONLY AND THE PROFESSIONAL SEAL ON THIS DRAWING DOES NOT APPLY TO CONTENT CONTAINED WITHIN ANY QR CODES.
- POST INSTALLED EPOXY ANCHORAGE GENERICALLY NOTED WITHIN THESE DRAWINGS SHALL BE EITHER SIMPSON STRONG TIE SET-3G, OR HILTI HIT-HY-150

DESIGN CRITERIA:

- BUILDING RISK CATEGORY _____=II
- VERTICAL LOADING:

ROOF TOP CHORD LIVE LOAD _____	=20 PSF (REDUCIBLE)
ROOF TOP CHORD DEAD LOAD _____	=7 PSF
ROOF TOP CHORD DEAD LOAD (TILE) _____	=15 PSF
ROOF BOTTOM CHORD LIVE LOAD (w/ STORAGE) _____	=20 PSF
ROOF BOTTOM CHORD LIVE LOAD (w/o STORAGE) _____	=10 PSF
ROOF BOTTOM CHORD DEAD LOAD _____	=10 PSF
FLOOR LIVE LOAD _____	=40 PSF
FLOOR LIVE LOAD (SLEEPING ROOMS) _____	=30 PSF
FLOOR DEAD LOAD _____	=15 PSF
DECK/BALCONY LIVE LOAD _____	=40 PSF

NOTE TO TRUSS & I-JOIST MANUFACTURERS - ROOF & FLOOR SYSTEM DELEGATED DESIGN SHALL CONFORM TO THE FOLLOWING DESIGN STANDARDS:

LOCATION	TOTAL LOAD	LIVE LOAD
ROOF TRUSSES	L/240	L/240
FLOOR JOISTS/TRUSS	L/360	L/480
- NOTES:
 - MAXIMUM TOTAL LOAD DIFFERENTIAL DEFLECTION BETWEEN ADJACENT PARALLEL MEMBERS SHALL BE 5/8"
 - ANY MEMBER SUPPORTING BRICK/MASONRY SHALL BE LIMITED TO A TOTAL LOAD DEFLECTION OF L/600
 - FIELDSTONE AE CANNOT BE HELD RESPONSIBLE FOR ANY STRUCTURAL ISSUES RELATED TO ANY BUILDING COMPONENT IF COMPONENT SHOP DRAWINGS ARE NOT SUBMITTED TO FAE FOR REVIEW PRIOR TO FABRICATION, DELIVERY, OR INSTALLATION
- LATERAL LOADING

TORNADO WIND SPEED _____	=N/A
BASIC WIND SPEED _____	=160 MPH
EXPOSURE CATEGORY _____	=C
INTERNAL PRESSURE COEFFICIENT _____	= +/- 0.18 (ENCLOSED)

SEISMIC CRITERIA

SITE CLASS _____	=D
SEISMIC DESIGN CATEGORY _____	=A
- EARTH PRESSURE LOADING:

ALLOWABLE SOIL BEARING CAPACITY _____	
TOTAL LOADS (INCLUDING WIND & SEISMIC) _____	=2000 PSF

LATERAL EARTH EQUIVALENT FLUID PRESSURE

WALLS UNBRACED AT TOP _____	=40 PCF
WALL BRACED AT TOP _____	=55 PCF
- STAIRS, GUARDRAIL/HANDRAIL LOADING:

STAIRS, IF THEY OCCUR, SHALL MEET UNIFORM LOADING OF 40 PSF OR 300 LBS CONCENTRATED LOAD. ALL GUARDRAIL/HANDRAILS SHALL MEET 200 LBS (MINIMUM) CONCENTRATED FORCE APPLIED IN ANY DIRECTION. IN-FILL COMPONENTS OF GUARDRAIL SHALL MEET 50 PLF. THESE LOADS ARE TO BE EVALUATED INDEPENDENTLY AND ARE NOT TO ACT CONCURRENTLY WITH ANY OTHER LIVE LOAD. DEFLECTION OF GUARDRAIL SHALL MEET THE CRITERIA SET FORTH BY THE INTERNATIONAL CODE COUNCIL EVALUATION SERVICES (ICC-ES) AC 273 - 'ACCEPTANCE CRITERIA FOR HANDRAILS AND GUARDS.'	
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FOUNDATION NOTES:

- CONTRACTOR SHALL VERIFY ALL CONDITIONS, INCLUDING UNDERGROUND UTILITIES AND FIELD MEASUREMENTS AT JOB SITE AND REPORT ANY DISCREPANCIES TO OWNERS REPRESENTATIVE.
- TOP OF SPREAD FOOTING ELEVATIONS NOTED ON PLAN ARE MINIMUM ELEVATIONS. IN ALL CASES FOOTINGS ARE TO BEAR ON UNDISTURBED NATURAL SOILS OR ENGINEERED FILL HAVING A MINIMUM NET ALLOWABLE BEARING CAPACITY OF 2000 PSF.
- SIDES OF FOUNDATIONS SHALL BE FORMED UNLESS CONDITIONS PERMIT EARTH FORMING. FOUNDATIONS POURED AGAINST THE EARTH REQUIRED THE FOLLOWING PRECAUTIONS: SLOPE SIDES OF EXCAVATIONS AS APPROVED BY GEOTECHNICAL ENGINEER AND CLEAN UP SLOUGHING BEFORE AND DURING CONCRETE PLACEMENT. PROVIDE POLYETHYLENE VAPOR RETARDER COMPLYING WITH GOVERNING BUILDING CODE LISTED ON THIS SHEET AGAINST SOILS WHERE CONCRETE IS TO BE PLACED; LAP ALL EDGES 6 INCH (MINIMUM) AND SEAL WITH ADHESIVE.
- WHERE FOOTING STEPS ARE NECESSARY, THEY SHALL BE NO STEEPER THAN ONE VERTICAL TO TWO HORIZONTAL U.N.O.
- FOOTINGS SHALL BE CENTERED UNDER COLUMNS AND WALLS UNLESS SPECIFICALLY DETAILED OTHERWISE ON THE DRAWINGS.
- NO FOOTINGS OR SLABS SHALL BE PLACED ON OR AGAINST SUB-GRADE CONTAINING FREE WATER, FROST OR ICE. SHOULD WATER OR FROST, HOWEVER SLIGHT, ENTER A FOOTING EXCAVATION AFTER SUB-GRADE APPROVAL, THE SUB-GRADE SHALL BE RE-INSPECTED BY THE GEOTECHNICAL ENGINEER/ TESTING LABORATORY AFTER REMOVAL OF WATER OR FROST.
- CONTRACTOR SHALL FURNISH ALL REQUIRED DE-WATERING EQUIPMENT TO MAINTAIN A DRY EXCAVATION UNTIL BACKFILL IS COMPLETE.

FOUNDATION NOTES (CONTINUED):

- A GEOTECHNICAL ENGINEER REGISTERED IN THE STATE OF WORK PERFORMED SHALL INSPECT THE CONDITION AND ASSURE THE ADEQUACY OF ALL SITE PREPARATION, SUBGRADES, FILLS, BACKFILLS BEFORE PLACEMENT OF FOUNDATIONS, FOOTINGS, SLABS AND WALLS. HE SHALL SUBMIT REPORT TO THE ARCHITECT/ENGINEER DESCRIBING HIS INVESTIGATIONS OF ANY NON-CONFORMING WORK.

BACKFILLING:

- DO NOT PLACE BACKFILL AGAINST FOUNDATION WALLS - DESIGNED AS SUPPORTED AT TOP AND BOTTOM - UNTIL BASEMENT LEVEL AND FIRST FLOOR SYSTEM ARE IN PLACE. SHORE AND/OR BRACE WALLS AS REQUIRED IF BACKFILLING OPERATIONS ARE TO BE CARRIED OUT PRIOR TO PLACEMENT OF FLOOR SYSTEM.
- WHERE BACKFILL IS TO BE PLACED ON BOTH SIDES OF FOUNDATION WALLS, PROVIDE A BALANCED BACKFILL AGAINST FOUNDATION WALLS TO ELIMINATE LATERAL LOAD EFFECTS, OR PROVIDE NECESSARY TEMPORARY LATERAL SUPPORT TO THE TOP OF THE WALL UNTIL PERMANENT SUPPORT IS INSTALLED.
- BACKFILL MATERIAL SHALL CONSIST OF CLEAN, WELL GRADE GRANULAR SOILS, FREE OF ORGANIC MATERIAL AND BE COMPACTED TO 95% OF MAXIMUM DENSITY, AS DETERMINED BY THE MODIFIED PROCTOR METHOD (ASTM D1557), IN LIFTS NOT EXCEEDING 8 INCHES.

CONCRETE NOTES:

- STRUCTURAL CONCRETE HAS BEEN DESIGNED IN ACCORDANCE PER ACI 332 "RESIDENTIAL CODE REQUIREMENTS FOR STRUCTURAL CONCRETE".
- CONCRETE PRODUCTION AND PLACEMENT SHALL CONFORM TO THE REQUIREMENTS OF ACI 332 "RESIDENTIAL CODE REQUIREMENTS FOR STRUCTURAL CONCRETE" EXCEPT AS MODIFIED BY STRUCTURAL REQUIREMENTS NOTED ON THE DRAWINGS.
- CEMENT SHALL CONFORM TO ASTM C150 "SPECIFICATION FOR PORTLAND CEMENT." CONCRETE SHALL BE NORMAL WEIGHT, UNLESS INDICATED OTHERWISE.
- CONCRETE AGGREGATES SHALL CONFORM TO ASTM C33 "SPECIFICATION FOR CONCRETE AGGREGATES."
- REINFORCING SHALL CONFORM TO ASTM A-615 GRADE 60.
- REINFORCEMENT SHALL BE FABRICATED AND ERECTED ACCORDING TO THE ACI STANDARDS: 'DETAILS AND DETAILING OF CONCRETE REINFORCEMENT,' ACI 315 - AND 'MANUAL OF ENGINEERING AND PLACING DRAWINGS FOR REINFORCED CONCRETE STRUCTURES,' ACI 315R.
- REINFORCING STEEL AND WELDED WIRE FABRIC SHALL BE PLACED WITH MINIMUM CONCRETE COVER AND TOLERANCES AS FOLLOWS:

SLAB-ON-GRADE: WITHIN UPPER 2/3 OF SLAB AND 3/4" MINIMUM COVER AT INTERIOR CONDITIONS; 1-1/2" MINIMUM COVER AT EXTERIOR CONDITIONS	
CONCRETE (FOUNDATION) AGAINST EARTH: 3"	
FORMED CONCRETE: 1-1/2" (#5 REBAR OR SMALLER) +/- 3/8"	2" (#6 REBAR OR LARGER)
- REINFORCING STEEL SHALL NOT BE CUT, BENT OR STRAIGHTENED IN THE FIELD UNLESS APPROVED BY THE STRUCTURAL ENGINEER OR AS INDICATED ON THE DRAWINGS.
- WELDING OF REINFORCING STEEL IS PROHIBITED UNLESS SPECIFICALLY DETAILED. WELDING WHERE DETAILED SHALL CONFORM TO AWS D1.4 SPECIFICATION.
- CONCRETE SHALL HAVE A MINIMUM 28-DAY COMPRESSIVE STRENGTH AS FOLLOWS:

FOUNDATIONS & FOUNDATION WALLS:	2500 PSI
SLAB-ON-GRADE & CONCRETE BEAMS:	2500 PSI
- CONTRACTOR SHALL TIE REINFORCING STEEL SECURELY IN PLACE PRIOR TO PLACING CONCRETE AND PROVIDE SUFFICIENT SUPPORTS TO MAINTAIN THEIR POSITION WITHIN SPECIFIED TOLERANCES DURING ALL CONSTRUCTION ACTIVITIES. INSERTING DOWELS INTO WET CONCRETE IS NOT PERMITTED. ANCHOR RODS AND STEEL INSERTS SHALL BE SET BY TEMPLATE TO WITHIN A 1/8" TOLERANCE IN ANY DIRECTION WITH MINIMUM EMBEDMENT AND EXACT PROJECTION INDICATED ON THE DRAWINGS. PRIOR TO PLACING CONCRETE.
- HOOKEB BARS SHALL BE STANDARD 90 DEGREE HOOKS PER ACI UNLESS NOTED OTHERWISE ON THE DRAWINGS. MINIMUM LAP SPlice SHALL BE CLASS B PER ACI 318. LOCATION OF LAP SPICES SHALL BE AS INDICATED ON CONSTRUCTION DOCUMENTS.
- DOWELS INTO FOUNDATION SHALL MATCH SIZE AND SPACING OF VERTICAL REINFORCEMENT AT ALL COLUMNS, PIERS AND WALLS, UNLESS OTHERWISE NOTED.
- CONTRACTOR SHALL OBTAIN APPROVAL PRIOR TO PLACING OPENINGS OR SLEEVES NOT SHOWN ON THE DRAWINGS, THROUGH ANY STRUCTURAL MEMBER.
- ONE OF THE FOLLOWING CONCRETE SLAB ON GRADE CRACK CONTROL METHODS SHALL BE USED (CONTRACTOR PREFERENCE):
 - CONTROL JOINTS: SHALL BE FORMED, SAWED, OR TOOLED TO A 1" MINIMUM DEPTH WITHIN 24 HOURS OF CONCRETE PLACEMENT. MAXIMUM JOINT SPACING FOR UNREINFORCED SLAB IS AS FOLLOWS:

3-1/2" SLAB:	8 FEET O.C. EACH DIRECTION
4" SLAB:	10 FEET O.C. EACH DIRECTION
 - WELDED WIRE FABRIC (W.W.F.) 6x6-W1.4XW1.4. WELDED WIRE FABRIC SHALL BE FURNISHED IN FLAT SHEETS (ROLLS NOT PERMITTED) AND SHALL CONFORM TO ASTM A-1064 AND HAVE A MINIMUM SIDE AND END LAP OF 6 INCHES.
 - MICRO- OR MACRO- SYNTHETIC FIBER REINFORCEMENT FIBER LENGTHS SHALL BE 1/2" TO 2-1/4" IN LENGTH. DOSAGE AMOUNT SHALL BE FROM 3/4 TO 3.0 POUNDS IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS. SYNTHETIC FIBERS SHALL COMPLY WITH ASTM C1116. THE MANUFACTURER/SUPPLIER SHALL PROVIDE CERTIFICATION OF COMPLIANCE WITH ASTM C1116 WHEN REQUESTED BY THE BUILDING OFFICIAL.
- CONTROL JOINTS SHALL BE FURNISHED WITH A FULL-LENGTH KEYWAY CENTERED ON MEMBERS. WHERE THE SIZE OF KEY IS NOT SHOWN ON THE DRAWINGS, THE KEY SHALL BE 25% OF THE CROSS-SECTION DIMENSION OF THE MEMBER AND MINIMUM 1-1/2 INCHES INTO THE FIRST POUR OF CONCRETE.
- PROVIDE POCKETS OR RECESSES IN CONCRETE WORK FOR COLUMNS AND BEAMS AS REQUIRED EVEN IF NOT SHOWN ON THE DRAWINGS. PROVIDE CONCRETE FILL AFTER BEAM ERECTION.
- CURING OF CONCRETE SURFACES SHALL CONFORM TO ACI 308.1 "STANDARD SPECIFICATION FOR CONCRETE CURING" AND ACI 308R "GUIDE TO CURING CONCRETE".

MASONRY NOTES:

- CONCRETE MASONRY HAS BEEN DESIGNED IN ACCORDANCE WITH ACI 530, "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES" AND SHALL BE CONSTRUCTED IN ACCORDANCE WITH ACI 530.1, "SPECIFICATIONS FOR MASONRY STRUCTURES."
- CONCRETE MASONRY SHALL CONFORM TO ASTM C90 AND HAVE A MINIMUM COMPRESSIVE STRENGTH F'm = 1,500 PSI U.O.N. MASONRY SHALL HAVE A TEXTURE SUITABLE FOR STUCCO APPLICATIONS WHERE ARCHITECTURAL DRAWINGS INDICATE STUCCO APPLICATION.
- CONCRETE MASONRY UNITS SHALL BE LAID IN RUNNING BOND.
- MORTAR FOR ALL MASONRY SHALL CONFORM TO ASTM C270 WITH MINIMUM COMPRESSIVE STRENGTH OF 2,000 PSI. MORTAR BELOW GRADE SHALL BE TYPE M. ELSEWHERE MORTAR MAY BE EITHER TYPE M OR S UNLESS SPECIFICALLY INDICATED OTHERWISE. USE EITHER PORTLAND CEMENT/LIME OR MASONRY CEMENT FOR MORTAR.
- GROUT SHALL CONFORM TO ASTM C476 WITH MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 2,500 PSI.
- STEEL BAR REINFORCEMENT SHALL CONFORM TO ASTM A615, GRADE 60.
- VERTICAL CELLS CONTAINING REINFORCING AND GROUT SHALL FORM A CONTINUOUS CAVITY, FREE OF MORTAR DROPPINGS.
- VERTICAL CMU WALL REINFORCING SHALL BE AS NOTED ON THE DRAWINGS. DOWELS TO CONCRETE FOUNDATION TO MATCH SIZE AND SPACING OF VERTICAL REINFORCING UNLESS NOTED OTHERWISE. REINFORCE CMU CORE AT CORNERS, EACH SIDE OF CONTROL JOINTS AND EACH SIDE OF WALL OPENINGS WITH (1)-#5 CONTINUOUS VERTICAL REINFORCING BARS.
- HORIZONTAL BOND BEAM AND VERTICAL REINFORCING SHALL BE CONTINUOUS U.O.N. LAP SPlice REINFORCING PER THE SCHEDULE OR USE MECHANICAL SPICES ADEQUATE FOR 125% OF SPECIFIED YIELD STRENGTH OF THE BAR. LAP VERTICAL REINFORCEMENT WITH MINIMUM DOWELS OF SAME SIZE AND SPACING THAT HAVE BEEN PREVIOUSLY INSTALLED IN THE FOUNDATIONS. DOWEL EMBEDMENT IN CONCRETE SHALL CONFORM TO THE REQUIREMENTS OF THE CONCRETE NOTES.

BAR SIZE	LAP SPICE LENGTH	BEND DIAMETER	ACI HOOK
#4	20"	3"	10"
#5	25"	3-3/4"	10"
#6	34"	4-1/2"	11"
#7	42"	5-1/4"	13"

MASONRY NOTES (CONTINUED):

- REINFORCING BARS SHALL BE HELD IN POSITION BY WIRE TIES OR OTHER APPROVED MEANS TO ENSURE DESIGN LOCATION AND LAP. PLACE BARS AND LAP PRIOR TO GROUTING.
- GROUTING OF MASONRY WALLS SHALL CONFORM TO RECOMMENDED PROCEDURE FOR "LOW LIFT GROUTING" OR "HIGH LIFT GROUTING" AS OUTLINED IN THE NCMA-TEK 3-2A - GROUTING FOR CONCRETE MASONRY WALLS AND ACI 530/ASCE 5 SPECIFICATION FOR MASONRY STRUCTURES. GROUT LIFTS SHALL NOT EXCEED 5 FEET WITHOUT MECHANICALLY CONSOLIDATED (VIBRATED) GROUT POURS.
- LIFTS OF GROUT SHALL BE KEYED 4 INCHES INTO THE PREVIOUS COURSE OF MASONRY BELOW.
- MASONRY BELOW GRADE SHALL BE GROUTED SOLID.
- CONTRACTOR SHALL BRACE MASONRY WALLS TO RESIST WIND LOADS UNTIL FLOORS AND ROOFS ARE IN PLACE, AND THE MASONRY HAS REACHED 75% OF THE REQUIRED STRENGTH F.M. BRACING SHALL BE PROVIDED IN ACCORDANCE WITH OSHA - CONSTRUCTION SAFETY STANDARDS FOR MASONRY WALL BRACING AND NCMA TEK 3-4B. "BRACING CONCRETE MASONRY WALLS DURING CONSTRUCTION."

WOOD FRAMING NOTES:

- STRUCTURAL SAWN LUMBER, STRUCTURAL COMPOSITE LUMBER, AND CONNECTIONS HAVE BEEN DESIGNED IN ACCORDANCE WITH THE "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION." (NDS)
- PLYWOOD HAS BEEN DESIGNED IN ACCORDANCE WITH "PANEL DESIGN SPECIFICATION." AND SHALL CONFORM TO "VOLUNTARY PRODUCT STANDARD PS1 CONSTRUCTION AND INDUSTRIAL PLYWOOD."
- ORIENTED STRAND BOARD (OSB) SHALL CONFORM TO "VOLUNTARY PRODUCT STANDARD PS2 PERFORMANCE STANDARD FOR WOOD-BASED STRUCTURAL -USE PANELS".
- ALL BEAMS AND HEADERS SHALL BE (2)-PLY U.N.O. A 1/2" RIGID INSULATION BOARD SHALL BE ADDED BETWEEN HEADER PLIES**
- WOOD SILL PLATES OF ALL EXTERIOR WALLS, INTERIOR WALLS w/UPLIFT AND SHEAR WALLS WHICH ARE SUPPORTED ON CONTINUOUS CONCRETE OR MASONRY SHALL BE ANCHORED WITH 1/2" DIAMETER CAST-IN-PLACE ANCHOR BOLTS WITH 7" EMBEDMENT, 4" (MINIMUM) AND 12" (MAXIMUM) FROM END OF SILL PLATE(S), LOCATED WITHIN THE MIDDLE THIRD OF THE SILL PLATE, WITH NUT AND 3"x3"x0.229" WASHERS. 3"x3"x0.229" WASHERS ARE REQUIRED FOR 2x6 EXTERIOR WALLS & 2x6 INTERIOR WALLS w/ UPLIFT (I.E. W10 DETAIL). OTHERWISE 2"x2"x3/16" WASHERS MAY BE USED FOR 2x4 WALLS w/ SIMILAR CONDITIONS.
MAXIMUM ANCHOR BOLT SPACING = 2'-0" O.C.
REFER TO "PRE-APPROVED FIELD REPAIR NOTES" SECTION FOR ALTERNATE ANCHORAGE APPROVALS. FOR OTHER ANCHORS NOT LISTED, SUBMIT TO ENGINEER OR RECORD FOR REVIEW.
- ALL LUMBER IN CONTACT WITH EARTH, CONCRETE, OR MASONRY - AND WOOD BEAMS POCKETING INTO CONCRETE OR MASONRY WALLS HAVING CLEARANCES LESS THAN 1/2" ON TOP, SIDES, AND END(S) SHALL BE PRESSURE TREATED OR SEPARATED WITH AN IMPERVIOUS MOISTURE BARRIER.
- STRUCTURAL SAWN LUMBER SHALL BE SPRUCE-PINE-FIR NO.1 /NO.2 OR BETTER U.N.O. WITH BASE DESIGN VALUES:

F _b = 875 PSI	F _c = 425 PSI	F _t = 450 PSI	F _e = 1150 PSI	F _v = 135 PSI	E = 1400 KSI
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WITH THE MOISTURE CONTENT OF THE WOOD IN SERVICE WILL NOT EXCEED 19%.
- BEARING AND EXTERIOR WALL STUDS SHALL BE CAPPED WITH DOUBLE (2X) TOP PLATES INSTALLED TO PROVIDE OVERLAPPING AT CORNERS AND AT INTERSECTIONS. END JOINTS IN DOUBLE (2X) TOP PLATES SHALL BE OFFSET AT LEAST 24 INCHES AND SHALL BE NAILED WITH NOT LESS THAN (8)-0.162"x2-1/2" FACE NAILS ON EACH SIDE THE JOINT. DOUBLE (2X) TOP PLATES OF EXTERIOR WALLS AND INTERIOR LOAD BEARING WALLS SHOWN ON DRAWINGS SHALL BE SOUTHERN YELLOW PINE NO.2 OR BETTER WITH BASE DESIGN VALUES:

F _b = 1100 PSI	F _c = 565 PSI	F _t = 675 PSI	F _e = 1450 PSI	F _v = 175 PSI	E = 1400 KSI
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WITH THE MOISTURE CONTENT OF THE WOOD IN SERVICE WILL NOT EXCEED 19%.
- STRUCTURAL COMPOSITE LUMBER SHALL CONFORM TO ASTM D 5456 WITH THE FOLLOWING ALLOWABLE DESIGN STRESSES

MICROLAM LVL	PARALLAM PSL (*)	TIMBERSTRAND LSL
E = 2000 KSI	E = 1800 KSI	E = 1300 KSI
F _b = 2600 PSI	F _b = 2400 PSI	F _b = 1700 PSI
(*) - BOOZER BEAM 2.1'E ARE AN ACCEPTED ALTERNATE TO PSL COLUMNS SPECIFIED ON PLANS PROVIDED THE 'BOOZER BEAM 2.1'E IS OF EQUAL OR GREATER SECTION DIMENSION TO THE PSL COLUMN AND OF EQUAL OR LESSER HEIGHT		
- FLOOR SHEATHING SHALL BE 23/32 INCH, APA RATED SHEATHING, 48/24 MIN., EXPOSURE 1. INSTALL WITH THE LONG DIMENSION OR STRENGTH AXIS OF THE PANEL ACROSS SUPPORTS AND WITH PANEL CONTINUOUS OVER TWO OR MORE SPANS. PANEL EDGES SHALL BE TONGUE-AND-GROOVE. FLOOR SHEATHING SHALL BE GLUED AND NAILED TO SUPPORTING MEMBERS. ADHESIVE SHALL MEET APA SPECIFICATION AFG.01, APPLIED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. PROVIDE 0.131"x2-1/2" COMMON NAILS AT 6 INCH O.C ALONG SUPPORTED PANEL EDGES AND 12 INCH O.C AT INTERMEDIATE SUPPORTS.
- FOR INTERIOR WALL HEADERS PROVIDE (1)-2X JACK STUD AND (1)-2X KING STUD U.N.O. ON DRAWINGS.
- FOR EXTERIOR WALL HEADERS PROVIDE (1)-2X JACK STUD AND (1)-2X KING STUD, MINIMUM, REFER TO CHART BELOW U.N.O ON DRAWINGS:

		WINDSPEED OF ≤ 160 EXP. C		
	WALL HEIGHT STUD SIZE HEADER SPAN (FT)	8FT	9FT	10FT
2x4	4	1J/1K	1J/1K	1J/1K
	6	1J/1K	1J/1K	1J/2K
	8	1J/1K	1J/2K	1J/3K
	10	1J/2K	1J/3K	1J/4K
	12	1J/2K	1J/3K	ENG
	14	1J/3K	1J/4K	ENG
	16	1J/4K	ENG	ENG
	18	1J/4K	ENG	ENG

		WINDSPEED OF ≤ 160 EXP. C		
	WALL HEIGHT STUD SIZE HEADER SPAN (FT)	8FT	9FT	10FT
2x6	4	1J/1K	1J/1K	1J/1K
	6	1J/1K	1J/1K	1J/1K
	8	1J/1K	1J/1K	1J/1K
	10	1J/1K	1J/1K	1J/1K
	12	1J/1K	1J/1K	1J/1K
	14	1J/1K	1J/1K	1J/1K
	16	1J/1K	1J/1K	1J/2K
	18	1J/1K	1J/1K	1J/2K

- WALLS SHOWN ON THE FRAMING PLANS ARE LOAD BEARING WALLS; WALLS NOT SHOWN ON STRUCTURAL FRAMING PLANS ARE INTERIOR NON-LOADING BEARING PARTITION WALLS.
- LOAD BEARING WALLS SHALL BE CONSTRUCTED WITH 2X STUDS AT 16" O.C. MAXIMUM U.N.O. NON-LOAD BEARING PARTITION WALLS SHALL BE CONSTRUCTED WITH 2X OR METAL STUDS (18 GA. MIN.) @ 24" O.C. MAXIMUM.
- PLY COLUMNS UNDER LOAD BEARING POINTS OF GIRDER/BEAMS SHALL MATCH THE GIRDER/BEAM PLU CUT WITHIN 1/2" TOLERANCE (2-PLY STUD COLUMN MINIMUM). SOLID BLOCK ALL GIRDER/BEAMS THRU FLOOR SYSTEMS AND TO THE FOUNDATION.
- WHEN NOT SHOWN ON DRAWINGS OR DETAILS, THE NAILING SCHEDULE FOR WOOD FRAMING ELEMENTS SHALL COMPLY WITH THE IRC TABLE R602.3(1).
- FLOOR SYSTEMS ARE DESIGNED BY THE TRUSS AND/OR WOOD I - JOIST MANUFACTURER'S DELEGATED CRITERIA NATION AND BE IN CONFORMANCE WITH THE LOADS & DEFLECTIONS IDENTIFIED IN THE DESIGN CRITERIA NATION SECTION. INSTALL PER MANUFACTURER'S RECOMMENDATIONS, NOTES, AND DETAILS. FLOOR FRAMING CONCEPT LAYOUTS SHOWN WITHIN THE DRAWINGS ARE FOR ILLUSTRATIVE PURPOSES ONLY TO DEPICT FLOOR SYSTEM-TO-WALL CONNECTIONS. CHANGES TO THE CONCEPTUAL FLOOR LAYOUT SHOWN IS NOT PERMITTED WITHOUT A LETTER AND REVIEW FROM THE ENGINEER OF RECORD. PREFABRICATED WOOD I-JOIST SHALL CONFORM TO ASTM D 5065.

JURISDICTION SPECIFIC NOTES:

- FOR PARTIALLY ENCLOSED AREAS, INCLUDING BUT NOT LIMITED TO LANAIS, PORCHES, AND BALCONIES A 1/2" (MINIMUM) EXTERIOR GRADE SOFFIT BOARD WITH A CEMENTITIOUS KNOCKDOWN FINISH (ONLY REQUIRED WHEN EXTERIOR CEILING IS 12 INCHES OR LESS ABOVE BULKHEAD) IS ACCEPTABLE PROVIDED THAT IT IS PAINTED MEETING THE REQUIREMENTS OF ASTM C932 TO GIVE THE CEILING WATER RESISTANCE. THE SOFFIT BOARD PRODUCT SHALL BE ATTACHED PER MANUFACTURER SPECIFICATIONS TO BE IN COMPLIANCE WITH R703.1.
- THE SOFFIT DESIGN PRESSURE IS AS NOTED IN THESE DOCUMENTS. WITH KAYCAN VINYL SOFFIT INSTALLED IN ACCORDANCE WITH MANUFACTURER'S REQUIRMENTS AND ICC-ES EVALUATION REPORT ESR-1495 THE SOFFIT WILL COMPLY WITH R703.1.

PRE-ENGINEERED ROOF TRUSS NOTES:

- ROOF TRUSS SHOWN ON STRUCTURAL DRAWINGS IN CONCEPTUAL ONLY TO ILLUSTRATE TRUSS-TO WALL CONNECTIONS AND DOES NOT SERVE AS A ROOF TRUSS DESIGN. ROOF TRUSSES SHALL BE DESIGNED AND MANUFACTURED IN CONFORMANCE FOR THE INDICATED LOADS WITH ANSI/TPI 1 "NATIONAL DESIGN STANDARD FOR METAL PLATE CONNECTED WOOD TRUSS CONSTRUCTION." FINAL PRE-ENGINEERED ROOF TRUSS FRAMING LAYOUT AND DESIGNS TO BE BY ROOF MANUFACTURER'S DELEGATED ENGINEER.
- ROOF TRUSSES SHALL BE DESIGNED FOR THE SUPERIMPOSED LOADS IDENTIFIED IN THE DESIGN CRITERIA NOTES SECTION.
- THE CONTRACTOR AND/OR TRUSS MANUFACTURER SHALL SUBMIT DEFERRED SUBMITTAL SHOP DRAWINGS SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF THE PROJECT, FOR REVIEW AND APPROVAL BY THE ENGINEER PRIOR TO FABRICATION. TRUSS SUBMITTAL SHALL INCLUDE TRUSS PLACEMENT PLAN, PERMANENT BRACING REQ'D FOR LATERAL SUPPORT OF INDIVIDUAL MEMBERS, EACH TRUSS ENGINEERING PROFILE WITH REACTION FORCE AND DIRECTION, LUMBER SIZ SPECIES AND GRADE, AND TRUSS HANGERS AND CONNECTIONS AT ALL TRUSS-TO-TRUSS, TRUSS-TO-BEAM, AND BEAM-TO-TRUSS CONNECTIONS ALSO WITH FASTENING REQUIREMENTS OF TRUSS PLY-TO-PLY AND FIELD SPICES.
- REFER TO ARCHITECTURAL DRAWINGS FOR TRUSS SHAPE, OVERHANG, DIMENSIONS, SLOPES, SPANS DRAINAGE, BEARING WALL HEIGHTS, ETC.
- THE POSITIONS, WEIGHTS, AND METHODS OF ATTACHMENTS OF ALL MECHANICAL UNITS, ELECTRICA FIXTURES, PLUMBING, FIRE SPRINKLERS, ETC SHALL BE INCLUDED IN THE DESIGN OF THE TRUSSES B THE TRUSS MANUFACTURER. SEE ARCHITECTURAL DRAWINGS FOR ATTIC ACCESS SIZE AND LOCATIONS.
- SUPERIMPOSED LOADS FROM SECONDARY FRAMING (I.E. SOFFITS, PARAPETS, VALLEY FRAMING, UPPER TRUSSES, ETC) SHALL BE INCLUDED IN THE DESIGN OF SUPPORTING TRUSSES.
- CONTRACTOR TO PROVIDE TRUSS BRACING PER BCSI-B3 IN ADDITION TO PERMANENT BRACING SHOWN ON THE TRUSS DESIGN PROVIDED BY THE DELEGATED TRUSS ENGINEER.

PRE-APPROVED FIELD REPAIR NOTES:

- MISPLACED DOWEL REPAIR: THIS NOTE DESCRIBES ALTERNATES TO MITIGATE THE CASTING OF REBA VERTICAL REINFORCEMENT AT AN INCORRECT LOCATION.
IF THE WALL HAS NOT BEEN BUILT: DRILL AND EPOXY #5 REBAR (36" LG.) INTO THE THICKENED EDGE OF SLAB AT THE CORRECT LOCATION. AS WALL IN CONSTRUCTED, LAP REBAR IN ACCORDANCE WITH MASONRY NOTES LAP SCHEDULE, PLACING A HOOKED BAR WITH 12" LAP AT THE TOP OF WALL WITH THE CONTINUOUS REBAR IN THE TIE BEAM/BOND BEAM. COMPLETE GROUTING OF WALL.
IF THE WALL HAS ALREADY BEEN BUILT: PRIOR TO GROUT PLACEMENT, OPEN THE WALL AT THE CORRECT LOCATION APPROXIMATELY 16" (H) x 4" (W) AT FLOOR LEVEL. DRILL AND EPOXY #5 REBAR (32" LG.) INTO THE THICKENED EDGE OF SLAB. LAP VERTICAL REINFORCEMENT IN ACCORDANCE WITH MASONRY NOTES LAP SCHEDULE, PLACING A HOOKED BAR WITH 12" LAP AT THE TOP OF WALL WITH THE CONTINUOUS REBAR IN THE TIE BEAM/BOND BEAM. COMPLETE GROUTING OF WALL.
- MISSING/MISPLACED ANCHOR BOLT: ALTERNATE SILL PLATE ANCHORAGE SOLUTIONS
INTO CONCRETE:
A 1/2"x6" TITEN HD WITH 1-3/4" MINIMUM EDGE DISTANCE AND 6" MINIMUM END DISTANCE IS A DIRECT 1:1 REPLACEMENT FOR A 1/2" DIAMETER ANCHOR BOLT WITH 7" EMBEDMENT INTO CONCRETE.
A 1/2" THREADED ROD SET-3G EPOXY 7" EMBEDMENT WITH 1-3/4" MINIMUM EDGE DISTANCE AND 7-1/2 MINIMUM END DISTANCE IS A DIRECT 1:1 REPLACEMENT FOR A 1/2" DIAMETER ANCHOR BOLT WITH 7" EMBEDMENT INTO CONCRETE.
INTO GROUT FILLED MASONRY (GFCMU):
A 1/2"x6" TITEN HD WITH 1-3/4" MINIMUM EDGE DISTANCE AND 8" MINIMUM END DISTANCE AT 4'-8" O.C REPLACES A 1/2" DIAMETER ANCHOR BOLT WITH 7" EMBEDMENT AT 6'-0" O.C.; LIKEWISE A 1/2"x6" TITEN HD WITH 1-3/4" MINIMUM EDGE DISTANCE AND 8" MINIMUM END DISTANCE AT 2'-8" O.C. REPLACES A 1/2" DIAMETER ANCHOR BOLT WITH 7" EMBEDMENT AT 4'-0" O.C.
- SIMPSON MASA SILL ANCHORS MAY BE USED AS AN ALTERNATE TO THE ANCHOR BOLTS AND SPACIN NOTED HERE-IN ACCORDING TO THE FOLLOWING TABLE:

BASIC WIND SPEED (MPH)	MAXIMUM SPACING
<140	6'-0"
140	1'-6"
150	1'-3"

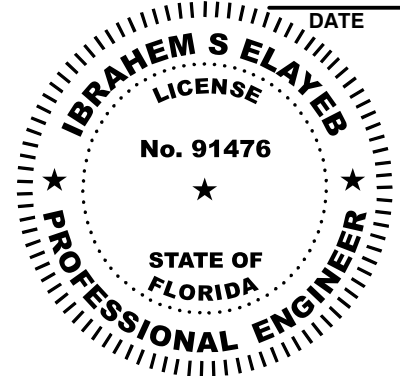
- NOTES:
- MASA SPACING BASED ON NO MORE THAN ONE IN THREE INSTALLED WITH ONE LEG INSTALLED VERTICALLY ATTACHED TO STUD
 - MISSING/MISPLACED TRUSS STRAP ANCHOR: REFER TO "MISSED TRUSS STRAP REPAIR" DETAIL WITHI DRAWINGS
 - FOR MISLOCATED TRUSS ANCHORS WHICH ARE GREATER THAN 1/8" BUT LESS THAN 1-1/2" FROM THE FACE OF THE TRUSS, A SHIM SHALL BE PROVIDED AND NAIL LENGTH INCREASED TO PENETRATE THROUGH BOTH PLYS OF MATERIAL (I.E. 3" LG. NAILS). WHEN GAP IS GREATER THAN 1-1/2", INSTALL NEW ANCHORS.
 - MASONRY WALL OVERHANGING SLAB

BLOCK OVERHANG	REPAIR
< 5/8"	NO REPAIR REQ'D
5/8" - OVERHANG < 1-1/2"	GROUT SOLID BOTTOM 3 COURSES OF AFFECTED WALL LENGTH CONTACT ENGINEER OF RECORD FOR REPAIR
 - NOTCHED TOP PLATES & STUDS REQUIRING REPAIR
AT STUDS: STRUCTURAL REPAIR IS REQUIRED AT STUDS WHEN A STUD IS NOTCHED TO A DEPTH GREATER THAN 25% OF THE WIDTH IN BEARING WALLS, OR 40% IN NON-LOAD BEARING WALLS; OR WHEN A HOLE IS DRILLED/BORED TO A DIAMETER EXCEEDING 40% OF THE STUD WIDTH IN BEARING WALLS, OR 60% OF THE STUD WIDTH IN NON-BEARING WALLS. IF THE MAXIMUM HOLE/NOTCH SPECIFICATIONS ARE EXCEEDED THEN A STUD SHOE MUST BE INSTALLED. STUD SHOE REPAIRS ARE AS FOLLOWS: FOR 2X STUD USE HSS2-SDS1.5 WITH (12)-1/4"x1-1/2" SDS SCREWS. FOR MULTI-PLY 2X STUDS USE HSS2-#1-SDS3 WITH (12)-1/4"x3" SDS SCREWS, WHERE "#1" CORRESPONDS TO THE NUMBER OF STUDS IN THE MULTI-PLY.
AT TOP PLATES: HOLE, CUT, OR NOTCH THAT IS GREATER THAN 50% OF THE PLATE WIDTH IS REMOVED FOR PIPING (EXCEPT WHEN THE SIDE OF THE WALL WITH THE NOTCH OR CUT IS COVERED BY WOOD STRUCTURAL PANEL SHEATHING), OR THE PLATES ARE CUT THROUGH. THE REQUIRED REPAIR IS A GALVANIZED 16-GAUGE METAL TIE THAT IS AT LEAST 1-1/2" WIDE AND MUST BE FASTENED WITH (8)-16D NAILS ON EACH SIDE OF THE OPENING.
 - INTERIOR BEARING WALLS WITHOUT UPLIFT (I.e. DETAIL W11) & OTHER INTERIOR NON-BEARING PARTITION WALLS SHALL HAVE SILL PLATES ATTACHED w/0.162"x3-1/2" NAILS @ 16" O.C. TO JOIST/RIM/BLOCKING AT WOOD FLOOR SUBSTRATE AND 3/16"x3-3/4" CONCRETE SCREWS OR HILTI X-C OR X-CP 2-7/8" POWDER ACTUATED FASTENERS @ 24" O.C. AT CONCRETE SUBSTRATE U.N.O. PER PLA



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C.O.A.# ARCHITECTURE: AA260002802
C.O.E.# ENGINEERING: 29349

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P.E. #91476
□ IAN MCCULLOUGH, PE
P.E.# 80956 10/23/2025



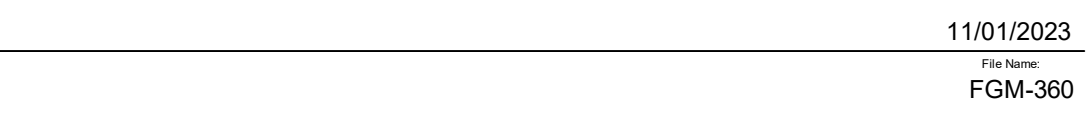
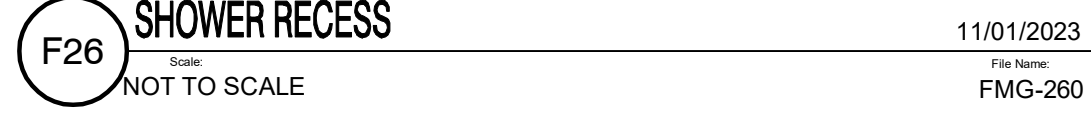
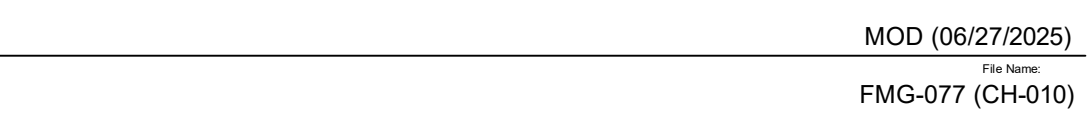
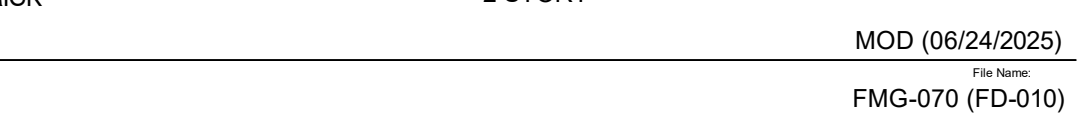
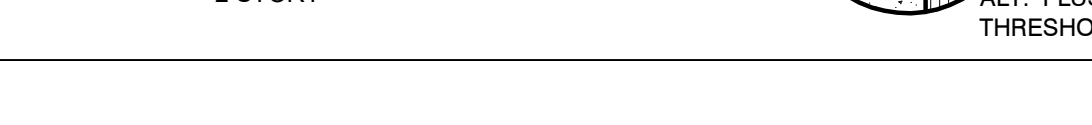
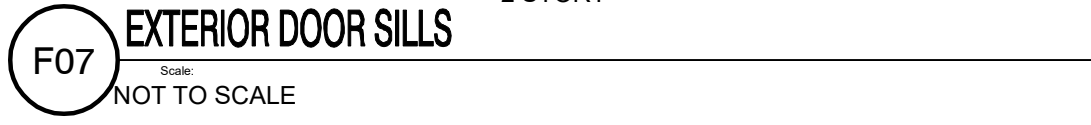
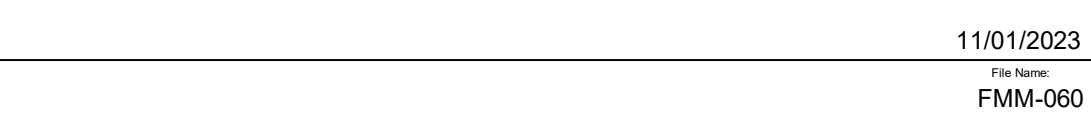
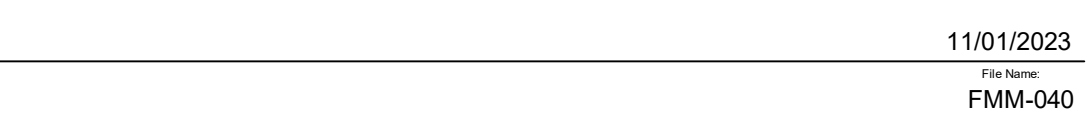
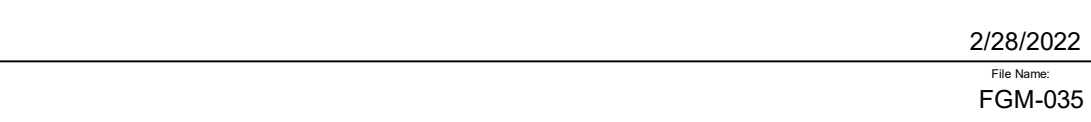
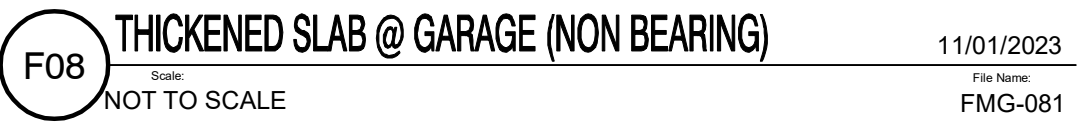
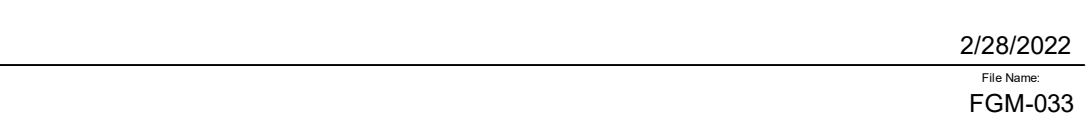
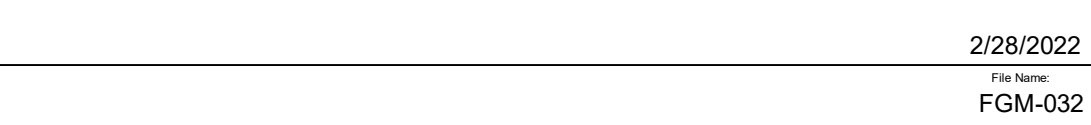
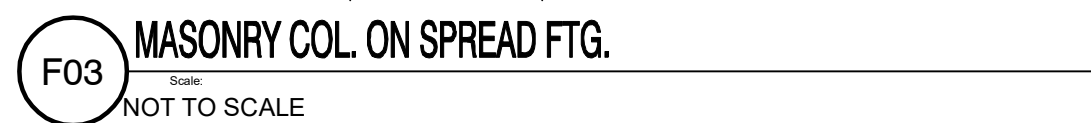
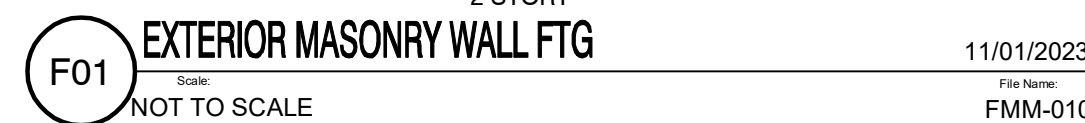
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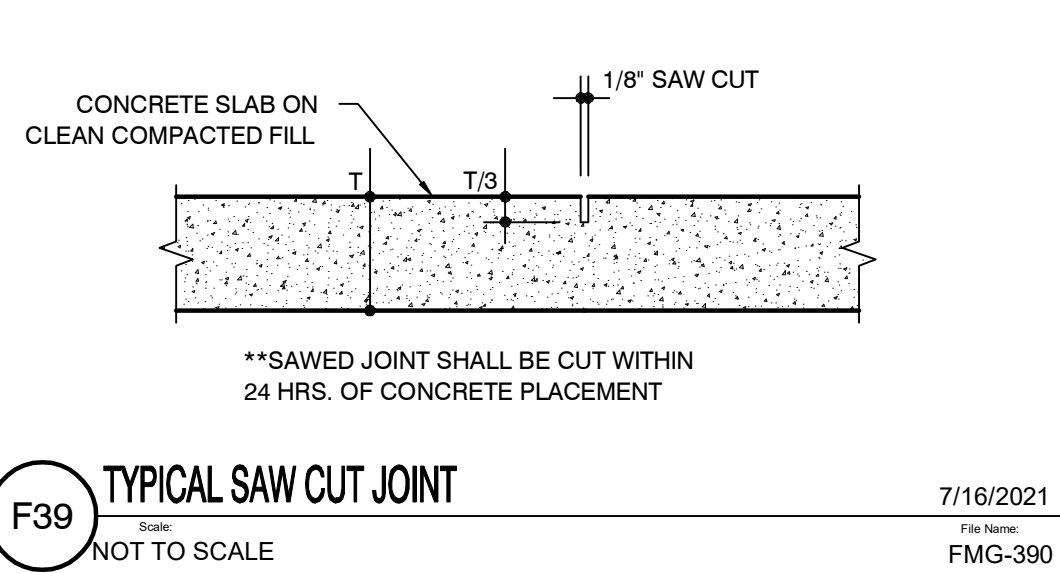
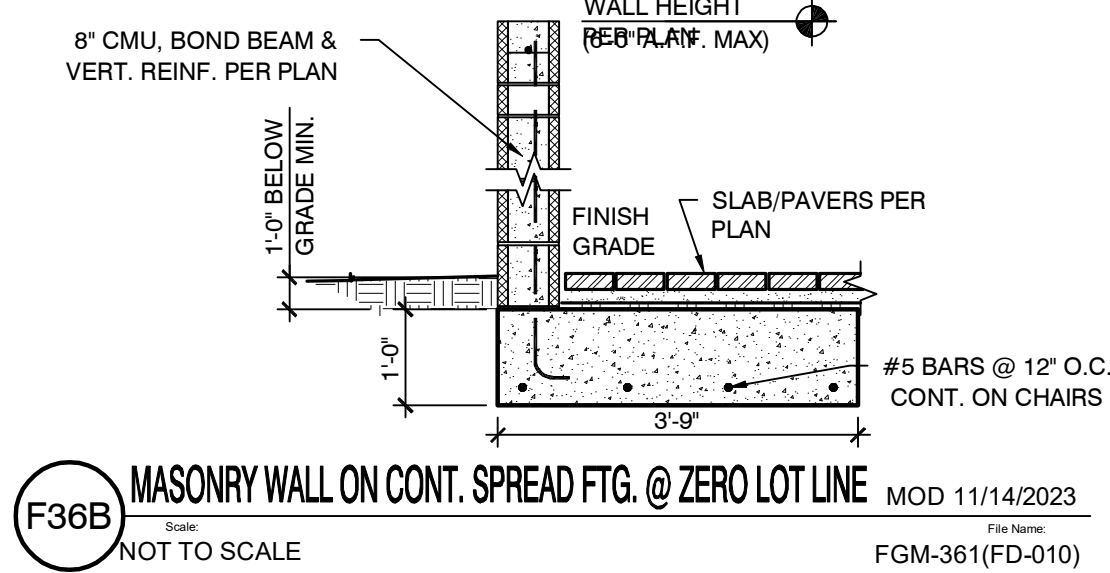
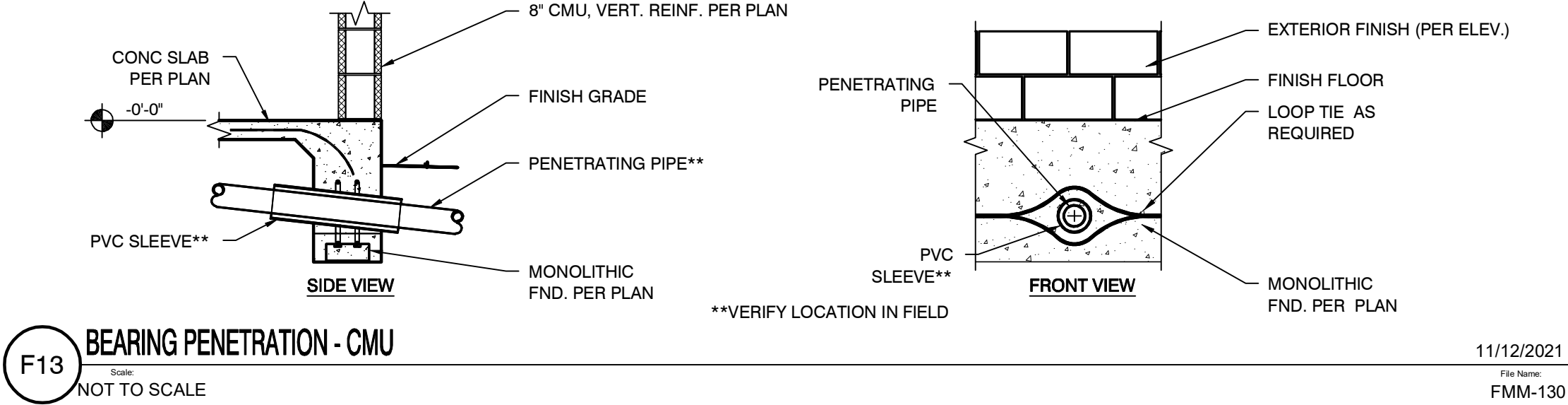
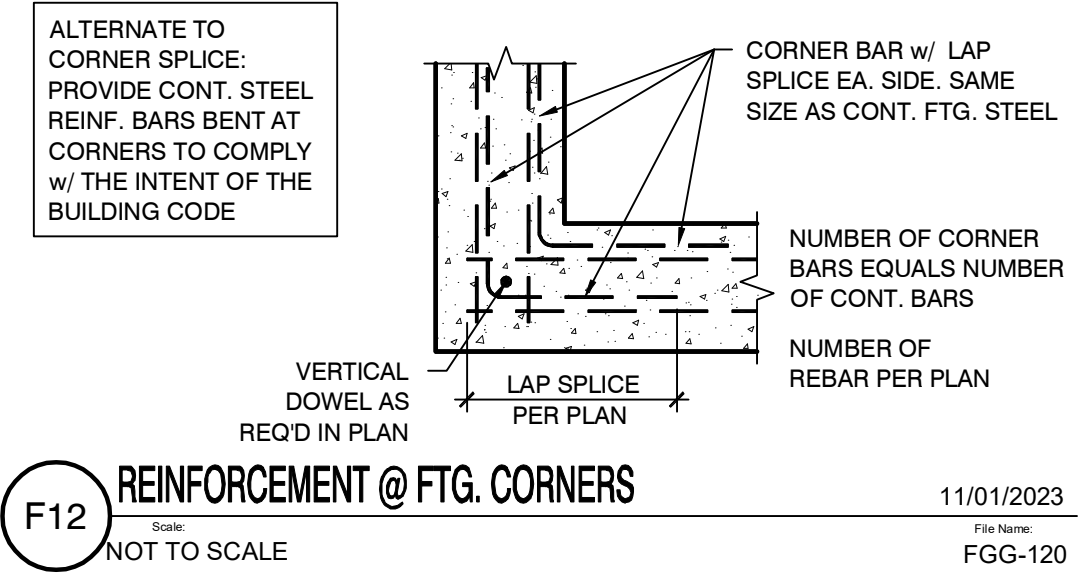


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Home for * Market Home * 8338-09300
6990 Minneola Court, Nokomis, FL 34275
COVER SHEET

PRODUCT MANAGER



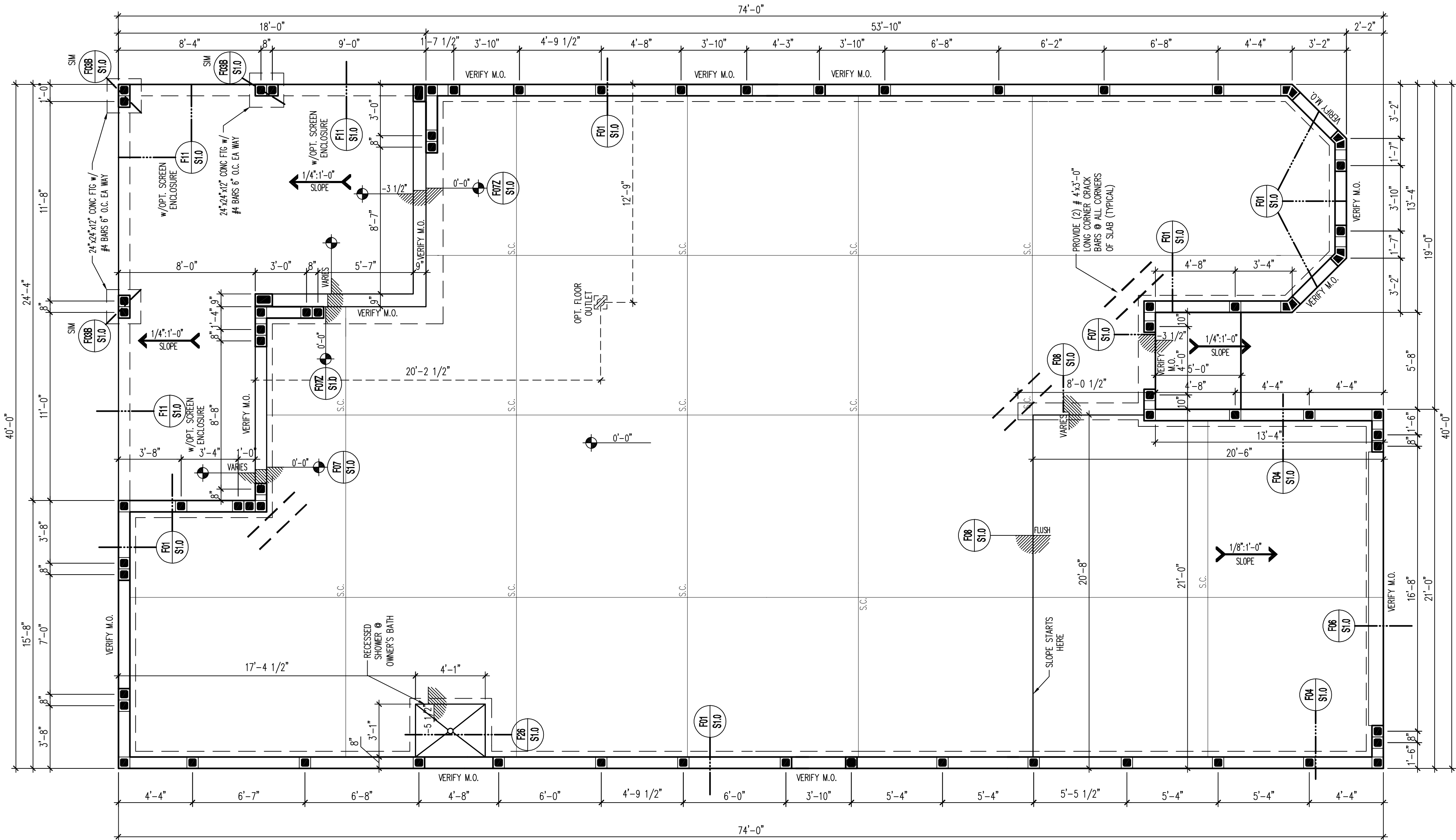


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Home for * Market Home * 8338-09300
 6990 Minneola Court, Nokomis, FL 34275
 FOUNDATION DETAILS

PRODUCT MANAGER
 RELEASE
 DATE: #####
 REV# DATE/DESCRIPTION
 PROJECT TYPE
 Single Family
 SPECIFICATION LEVEL
 Pulte
 PLAN NAME
 Mystique
 NPC CHILD NUMBER
 2684.500
 SHEET
 S1.0.1

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 C.O.E.# ENGINEERING: 29349
 ■ IBRAHEM ELAYEB, PE
 P.E.# 91476
 □ IAN MCCULLOUGH, PE
 P.E.# 80956
 10/23/2025
 DATE

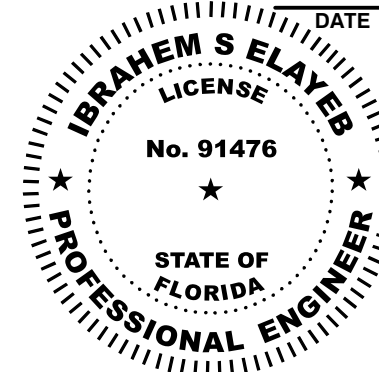


FOUNDATION PLAN

- SCALE: 1/4"=1'-0"
- NOTES:
- 3 1/2" CONCRETE SLAB ON GRADE OVER VAPOR BARRIER
 - REFERENCE FINISHED FLOOR ELEVATION = 0'-0"
 - REFER TO DWG S0.1 FOR GENERAL STRUCTURAL NOTES

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 P.E.# 91476
 □ IAN MCCULLOUGH, PE
 P.E.# 80956



PRODUCT MANAGER	
RELEASE	
DATE: #####	
REV#	DATE/DESCRIPTION
1	
2	
3	
4	
5	

PROJECT TYPE
 Single Family

SPECIFICATION LEVEL
 Pulte

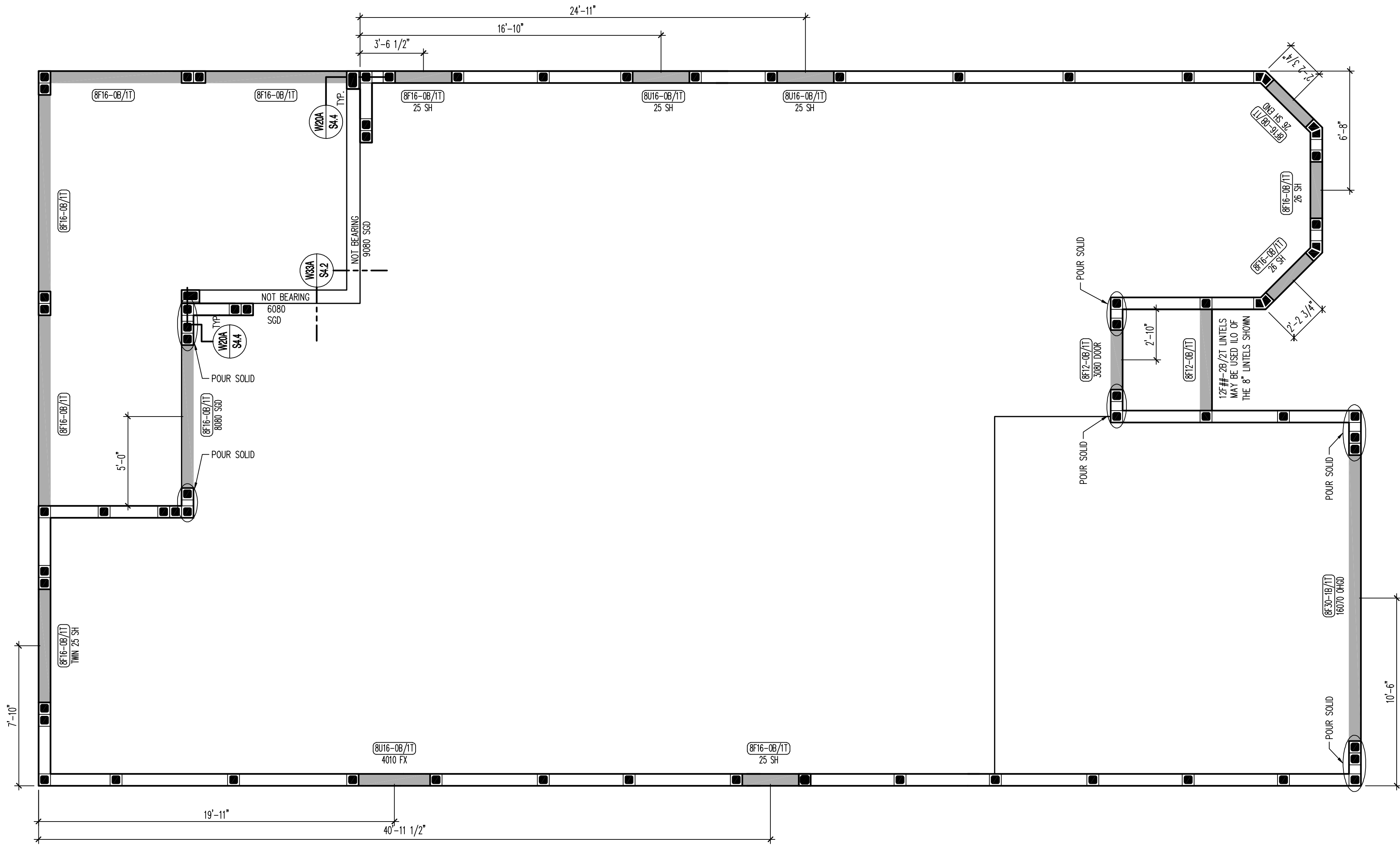
PLAN NAME
 Mystique
 NPC CHILD NUMBER
 2684.500

SHEET
 S1.1

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SCALE: 1/4"=1'-0"
NOTES:
1. REFER TO DWG S0.1 F

SCALE: 1/4"=1'-0"
NOTES:

1. REFER TO DWG S0.1 FOR GENERAL STRUCTURAL NOTES

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LINTEL AND BOND BEAM LEGEND

1. BOND BEAM IS TO BE 8" KNOCK OUT BLOCK WITH #5 BAR GROUTED SOLID, CONTINUOUS AROUND PERIMETER OF STRUCTURE
2. MINIMUM 4" NOMINAL BEARING PER END @ LINTELS OR AS REQUIRED BY MANUF.
3. LINTELS WITH A CLEAR SPAN LONGER THAN 12'-4" SHALL BE PRESTRESSED.
4. BOND BEAM ELEV = 9'-4" U.N.O.
5. GARAGE DOOR LINTEL HEIGHT MAY HAVE A TOLERANCE OF +/- 4" FOR ALL LINTELS 28" OR GREATER

TYPE DESIGNATION

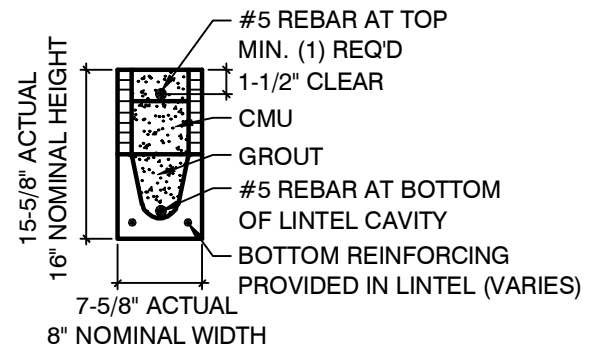
F = FILLED WITH GROUT / U = UNFILLED
QUANTITY OF #5 REBAR AT
BOTTOM OF LINTEL CAVITY

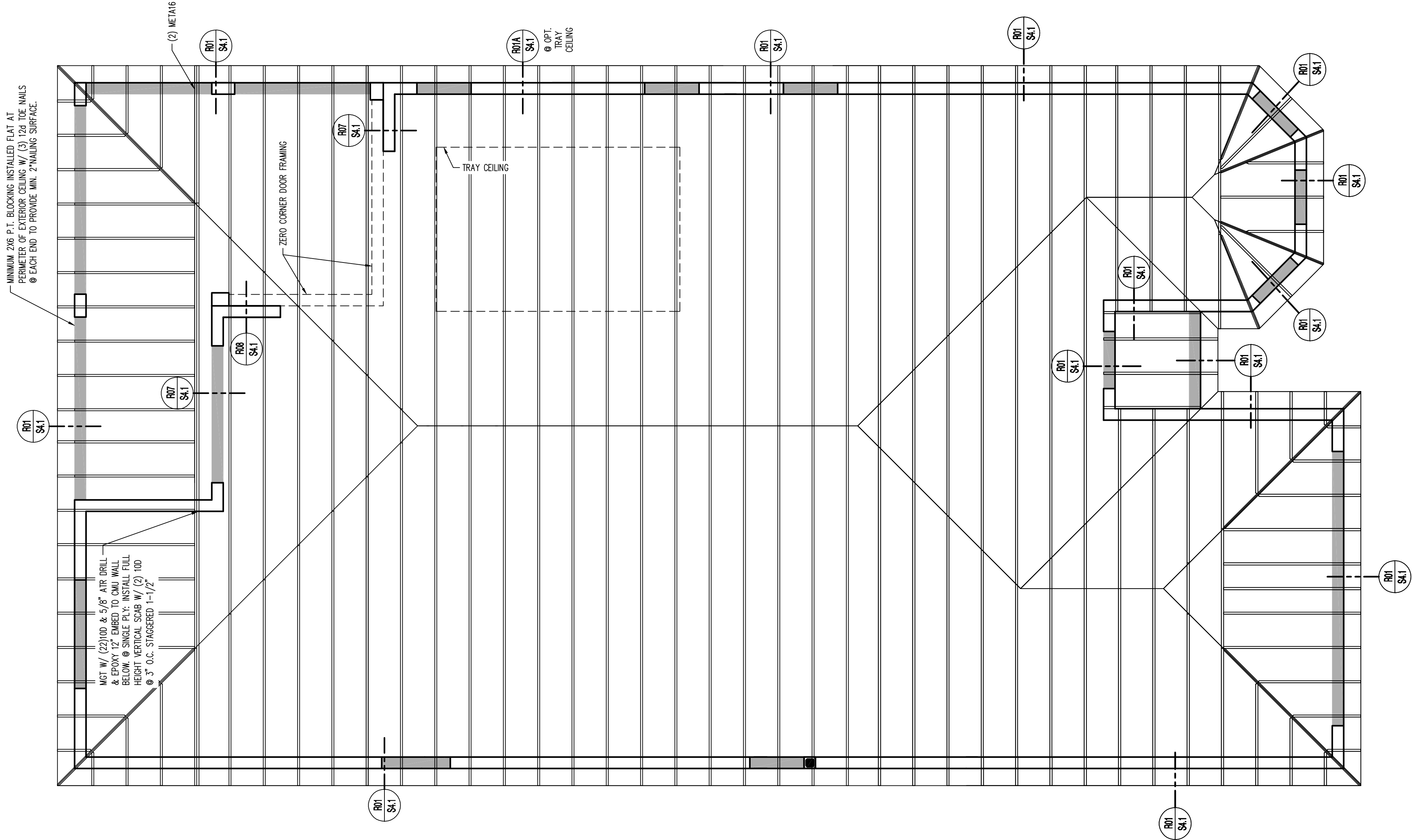
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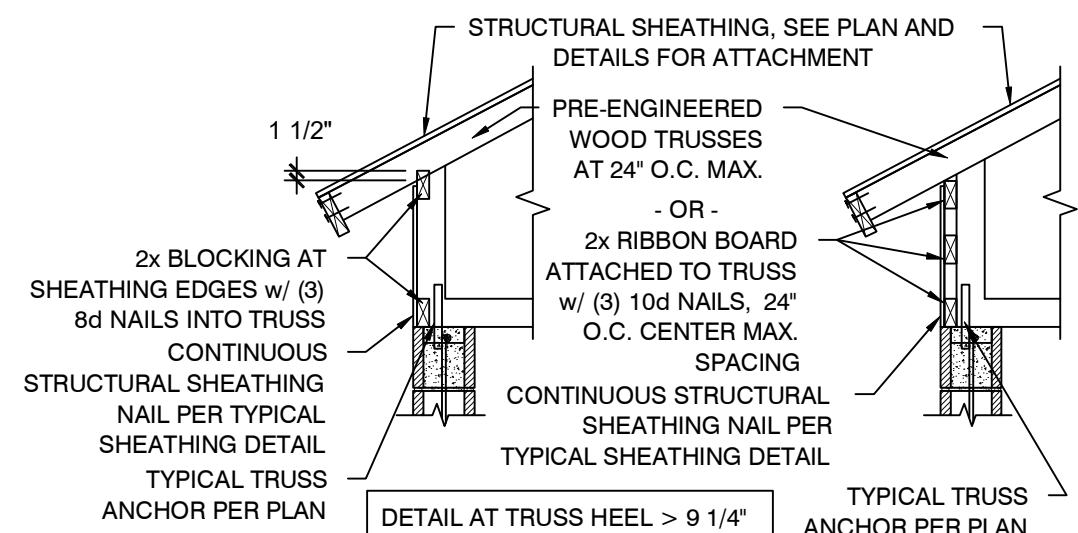
QUANTITY OF #5 REBAR AT TOP

NOMINAL HEIGHT

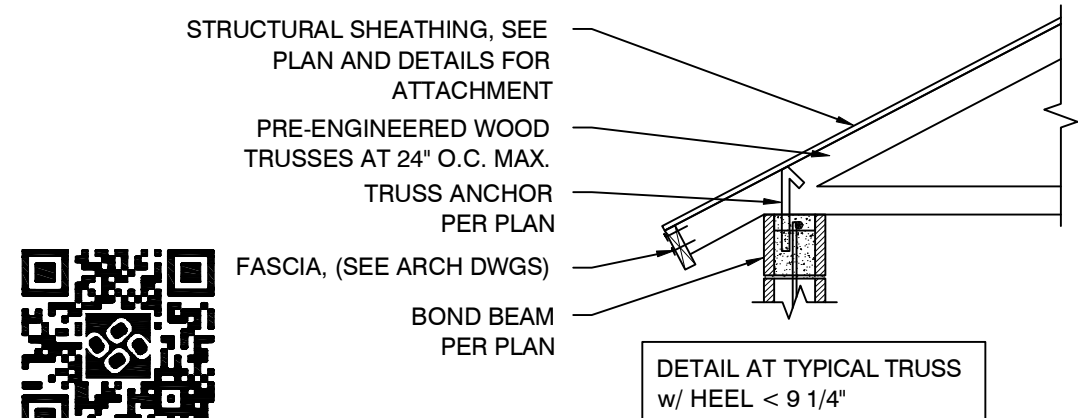
NOMINAL WIDTH



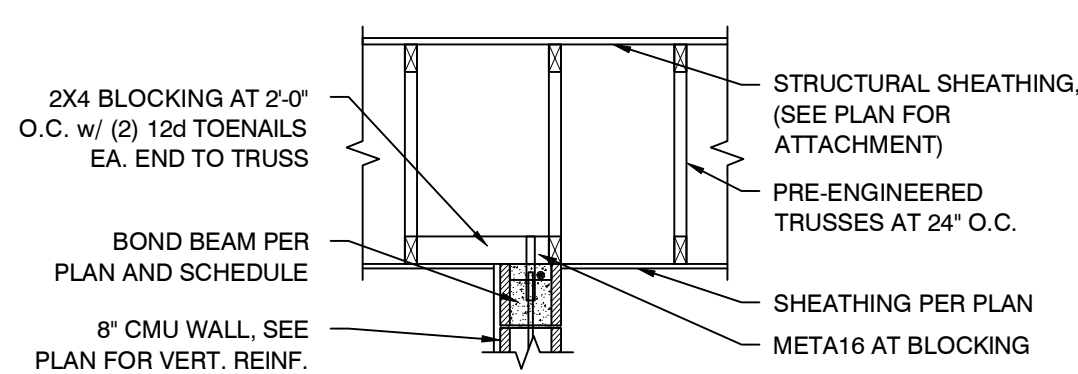




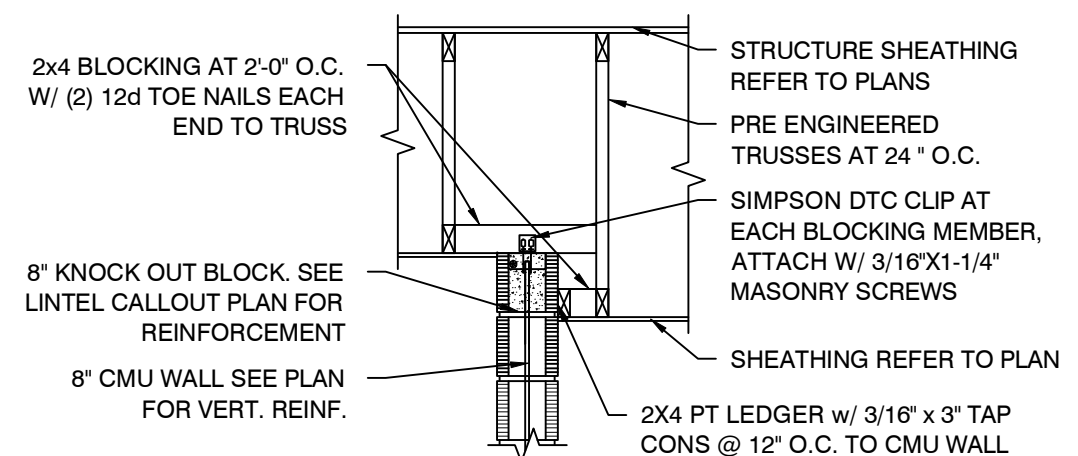
NOTE: REFER TO ARCHITECTURAL DRAWINGS AND/OR TRUSS MANUFACTURER DRAWINGS FOR SPECIFIC TRUSS PROFILE INFORMATION. (I.e. HEEL HEIGHTS, SLOPE, OVERHANGS)



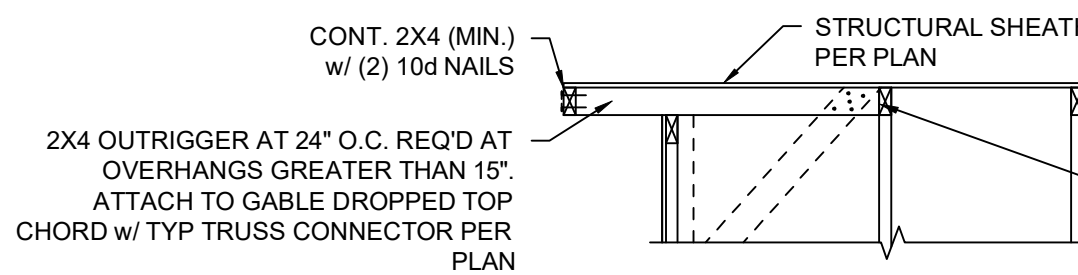
R01 ROOF TRUSS ON MASONRY
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11/01/2023



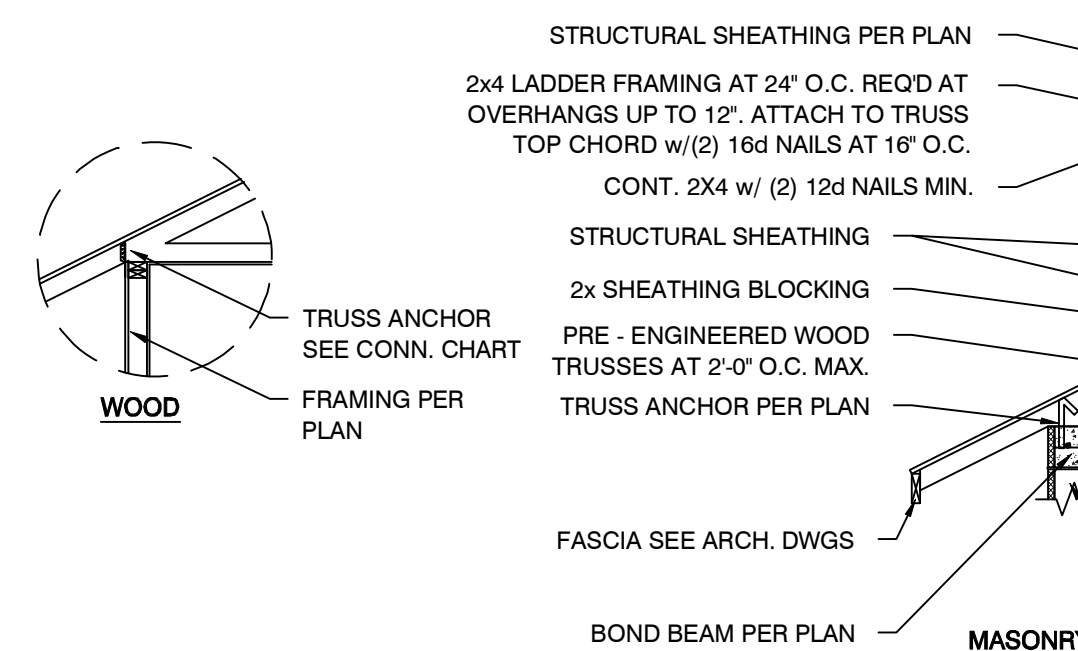
R07 WALL BRACING @ NON-BRG. EXT. WALL (NON-SHEAR)
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File Name: RWM-070
11/01/2023



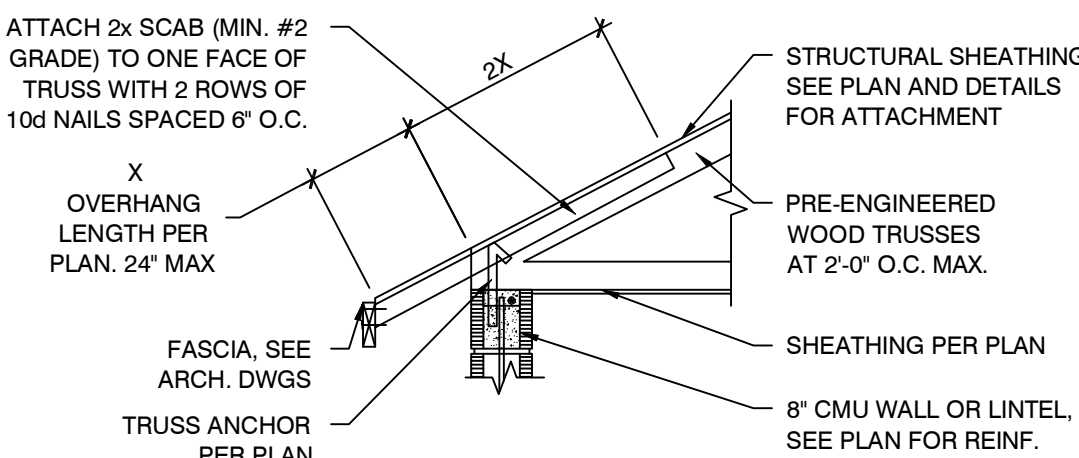
R07B WALL BRACING AT EXTERIOR WALL
Scale: NOT TO SCALE
File Name: RWM-072
8/18/2022



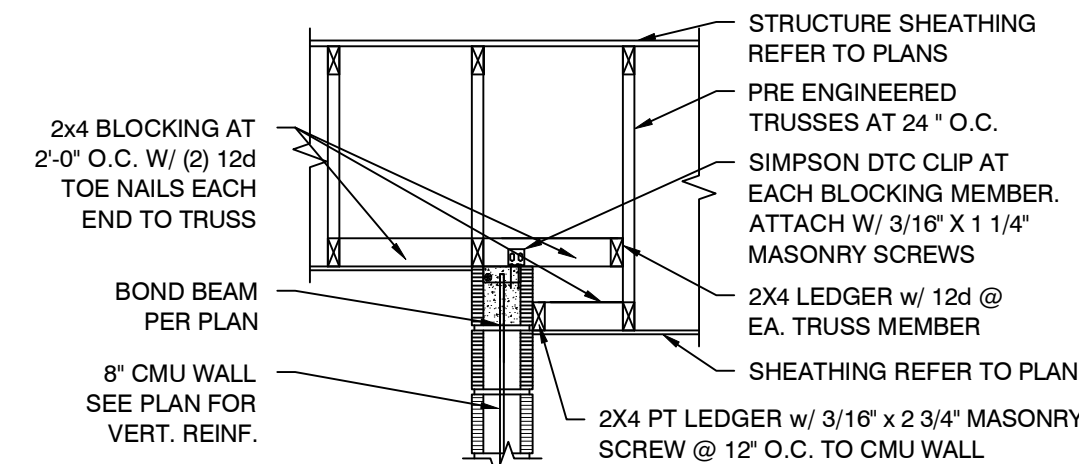
DROPPED TOP GABLE w/ OUTRIGGERS



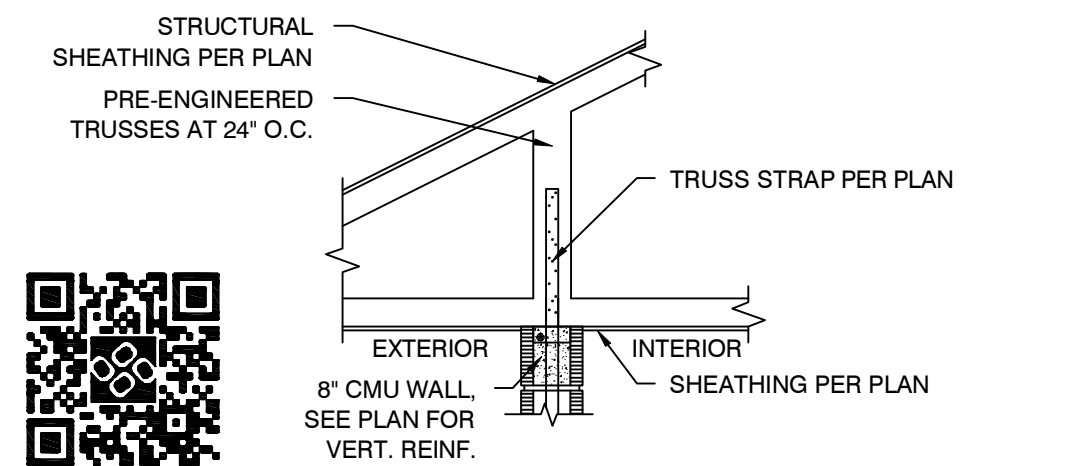
R10 DUTCH HIP GABLE TRUSS BRACING DETAIL W/ DROPPED GABLE
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11/01/2023



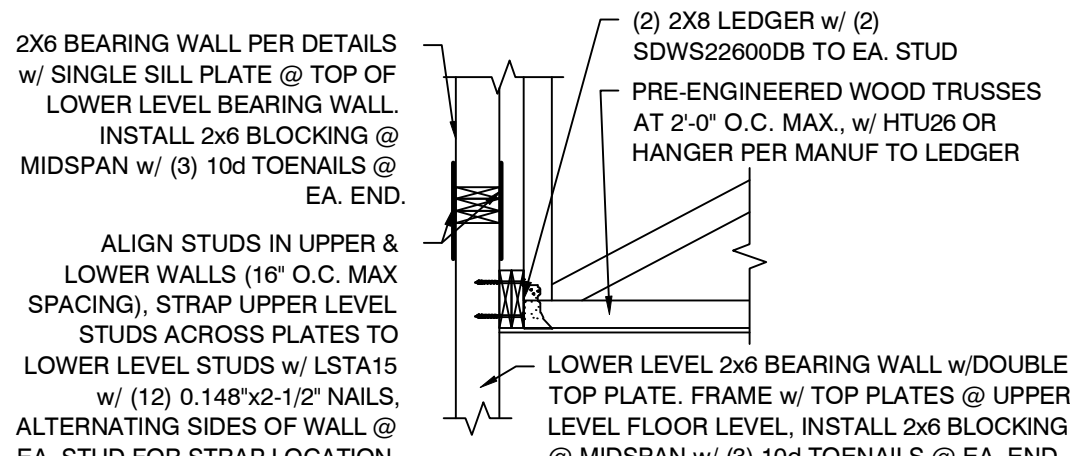
R01A SCAB OVERHANG
Scale: NOT TO SCALE
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7/16/2021



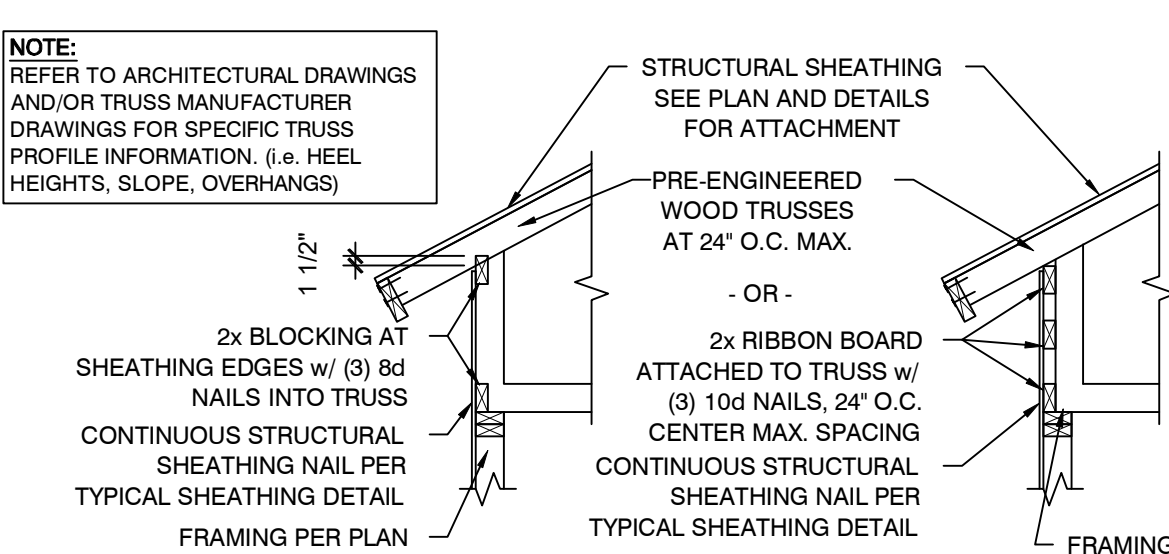
R07D WALL BRACING AT EXTERIOR WALL
Scale: NOT TO SCALE
File Name: RWM-074
11/12/2021



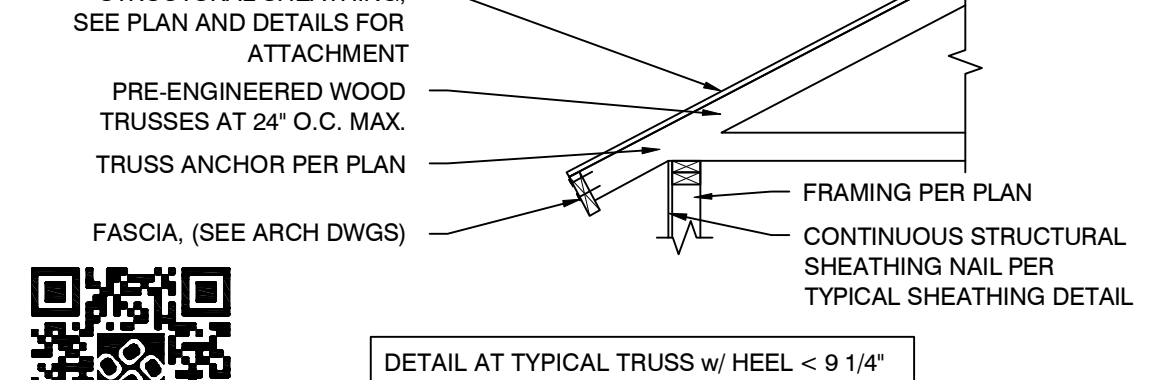
R08 BRACING @ PERPENDICULAR BRG. WALL (NON-SHEAR)
Scale: NOT TO SCALE
File Name: RWM-080
11/01/2023



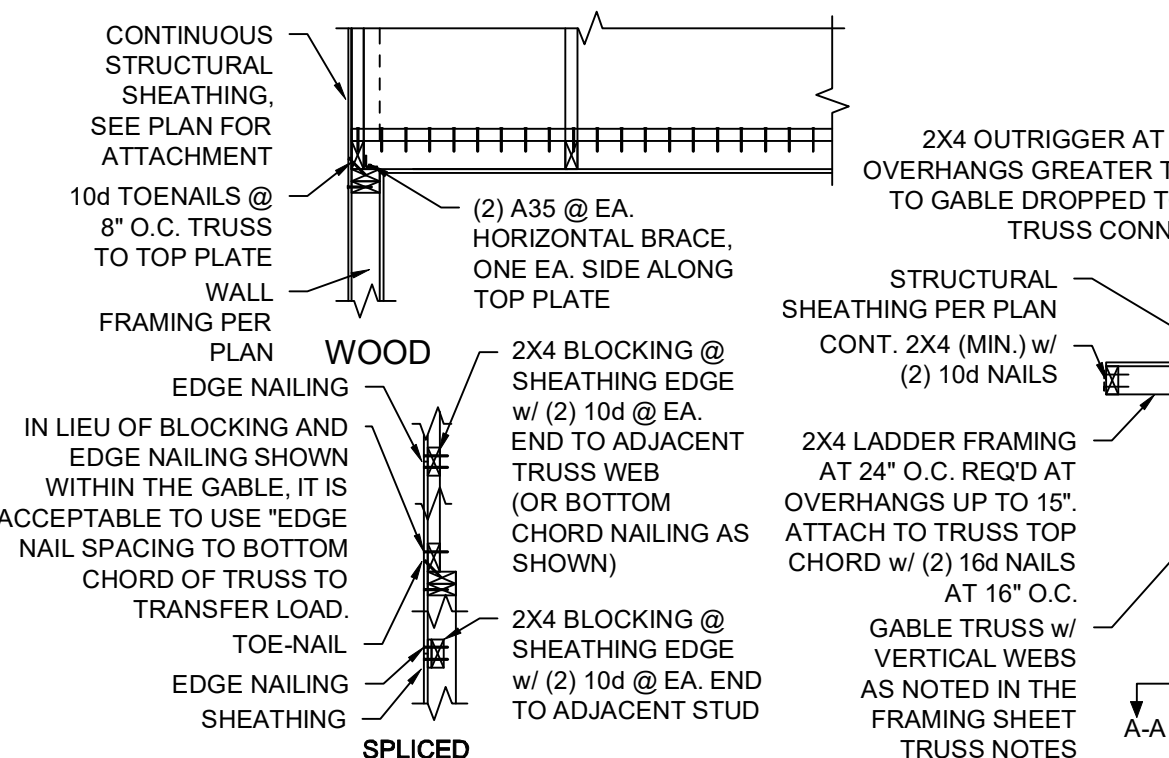
R50A 2 STORY FRAMED WALL (PERPENDICULAR TRUSSES)
Scale: NOT TO SCALE
File Name: RWW-504
2/28/2022



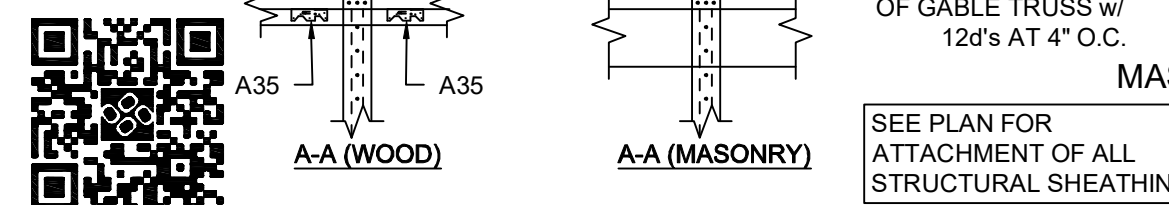
R02 ROOF TRUSS ON FRAMING
Scale: NOT TO SCALE
File Name: RWW-020
10/07/2025



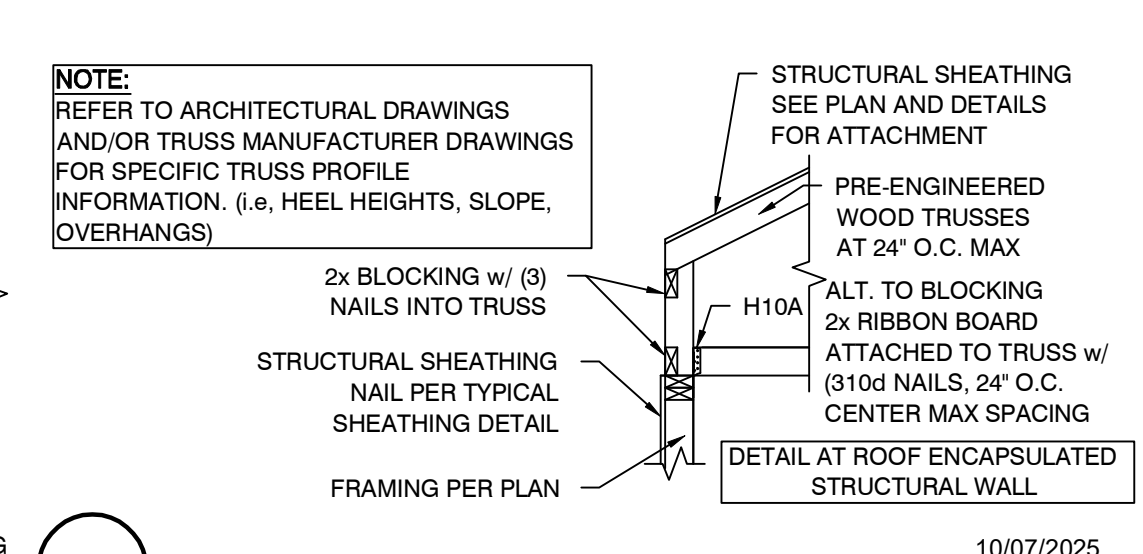
R02 ROOF TRUSS ON FRAMING
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10/07/2025



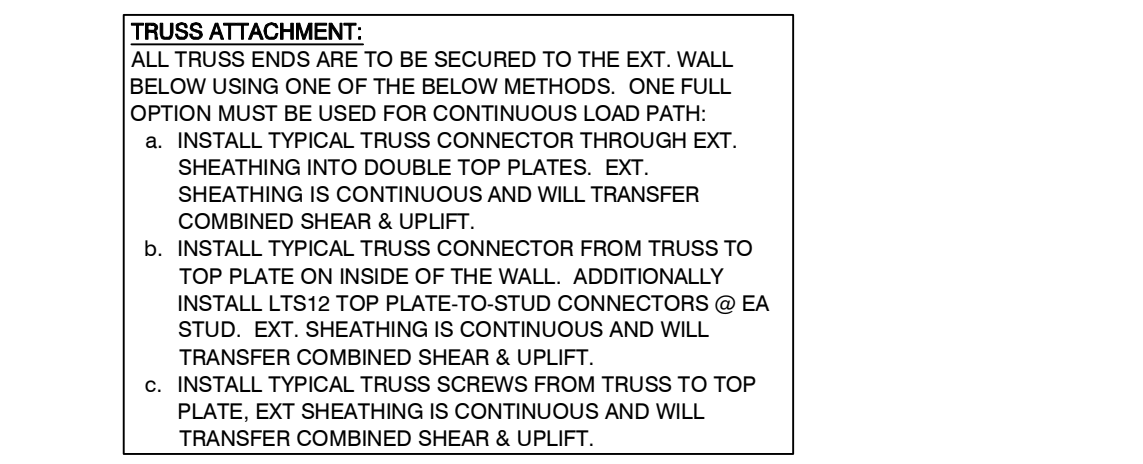
R09 GABLE END TRUSS BRACING DETAIL
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11/01/2023



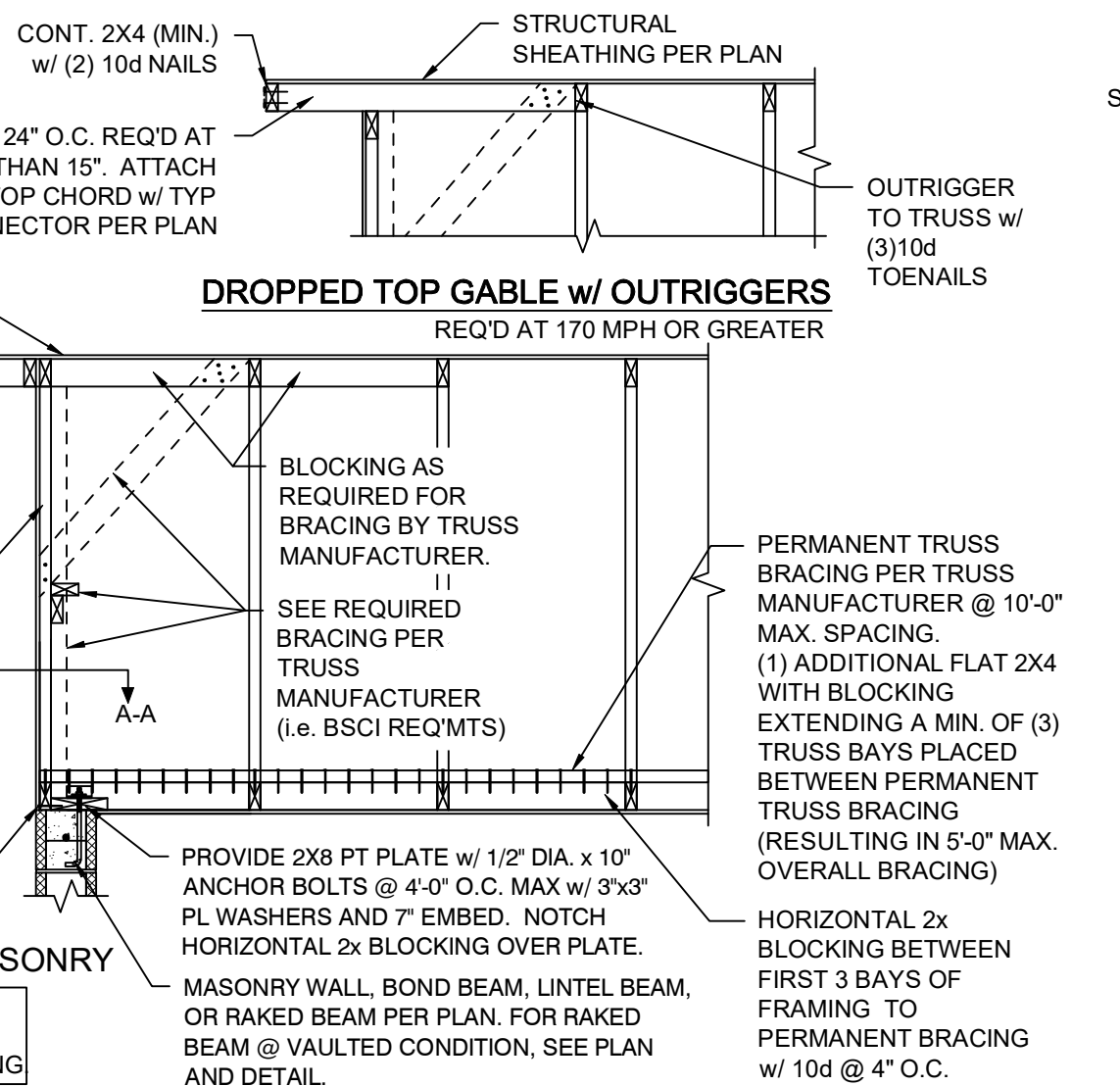
R09 GABLE END TRUSS BRACING DETAIL
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File Name: RWM-090
11/01/2023



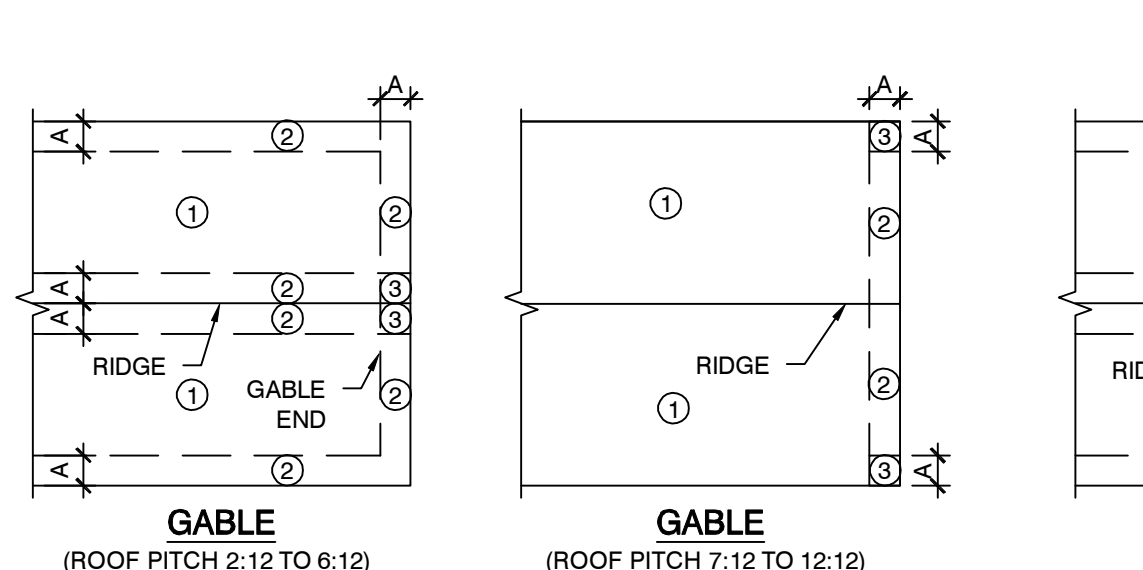
R02B ROOF TRUSS ON FRAMING
Scale: NOT TO SCALE
File Name: RWW-020
10/07/2025



R02B ROOF TRUSS ON FRAMING
Scale: NOT TO SCALE
File Name: RWW-020
10/07/2025



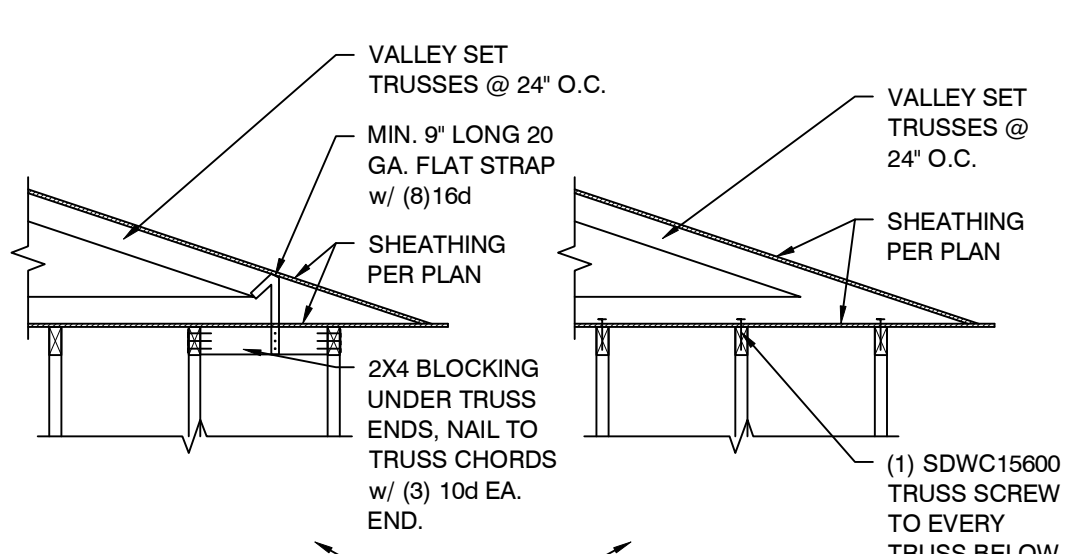
R09C STRUCTURAL GABLE END TRUSS DETAIL (SHEATHED STRUCTURAL GABLE)
Scale: NOT TO SCALE
File Name: RWG-093
11/01/2023



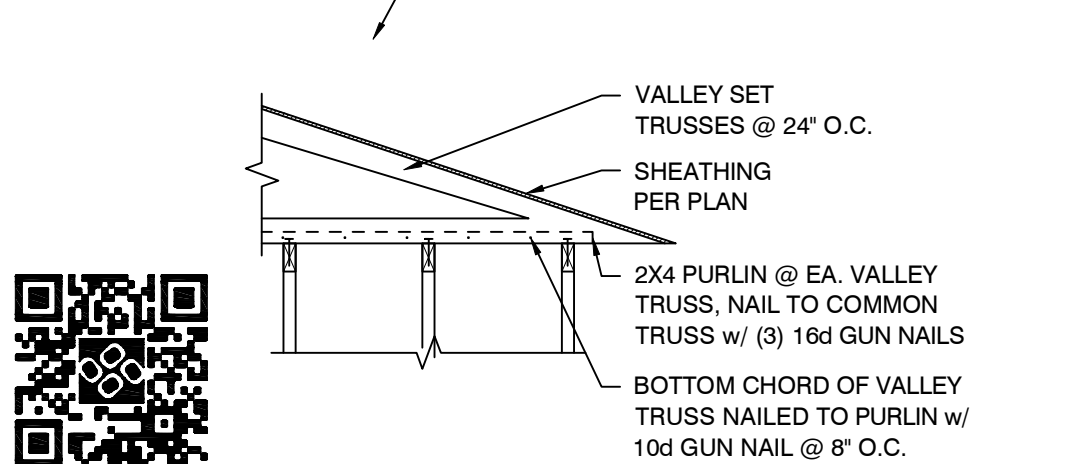
ROOF SHEATHING	NAILS	ROOF FASTENING ZONES		
19/32" SHEATHING (40/20 SPAN RATED PLYWOOD OR OSB)	RING SHANK NAILS: 0.131" x 2-1/2" x0.281 HEAD DIA.	NOTE: "A" = 4 FEET IN ALL CASES	1	2
		PANEL EDGE NAILING (EN)	6" O.C.	6" O.C.
		PANEL FIELD NAILING (FN)	6" O.C.	6" O.C.
		FOR BASIC WIND SPEED 170 MPH OR GREATER, EN & FN SHALL BE 4" O.C. ALL ZONES		

NOTES:
1. WOOD STRUCTURAL PANELS USED AS ROOF SHEATHING SHALL BE INSTALLED WITH JOINTS STAGGERED & LONG SIDE OF PANELS PERPENDICULAR TO ROOF FRAMING MEMBERS

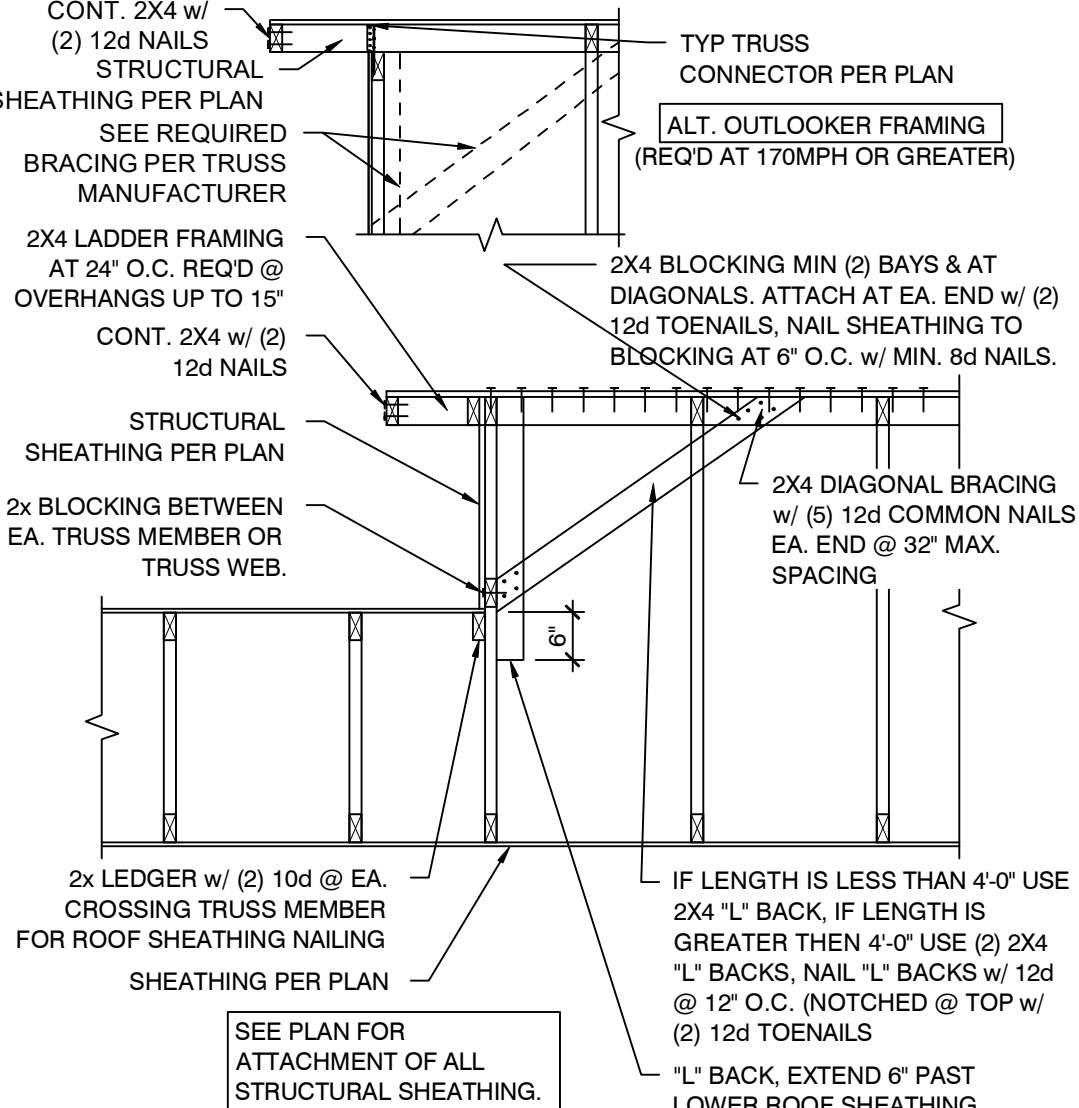
R11 ROOF SHEATHING ATTACHMENT DIAGRAM
Scale: NOT TO SCALE
File Name: RWG-112
11/01/2023



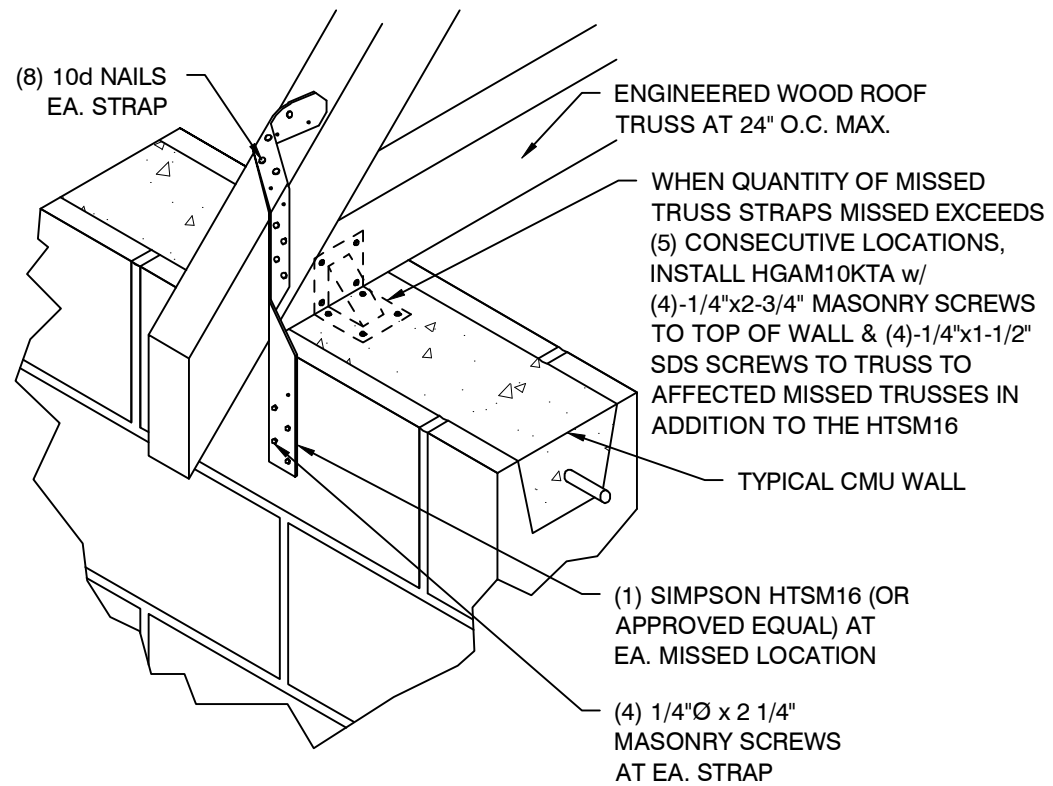
R06 TRUSS VALLEY SET OVER FRAME
Scale: NOT TO SCALE
File Name: RWG-060
11/01/2023



R06 TRUSS VALLEY SET OVER FRAME
Scale: NOT TO SCALE
File Name: RWG-060
11/01/2023



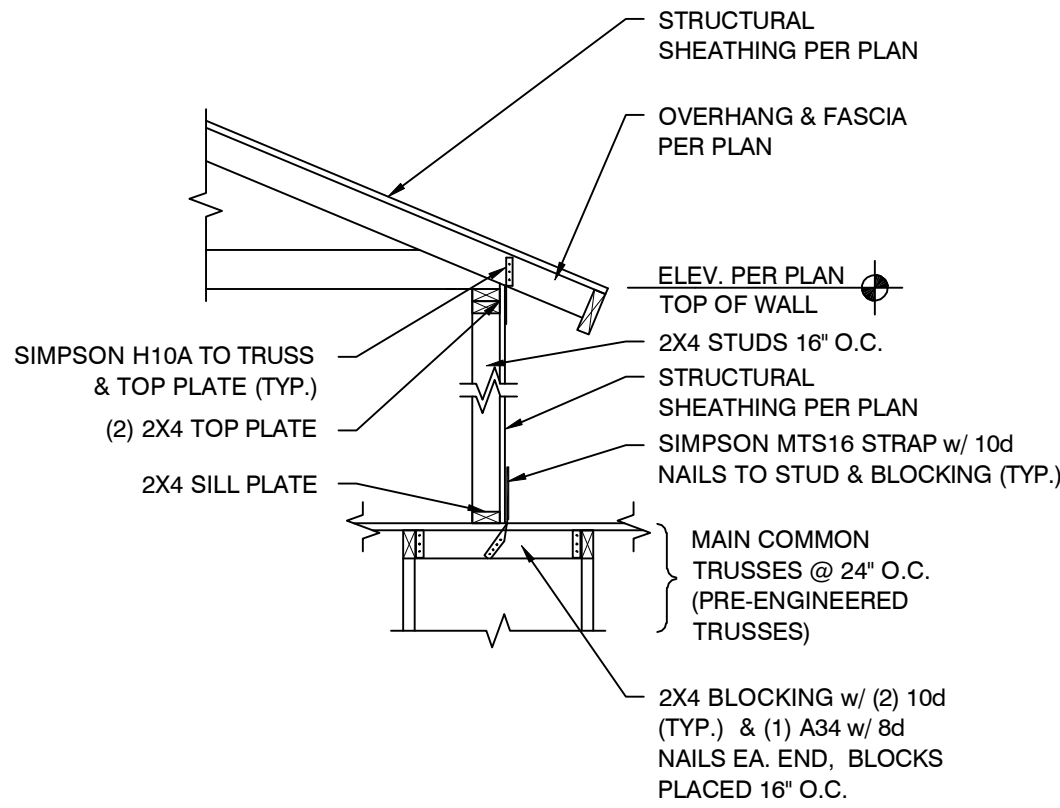
R09C STRUCTURAL GABLE END TRUSS DETAIL (SHEATHED STRUCTURAL GABLE)
Scale: NOT TO SCALE
File Name: RWG-093
11/01/2023



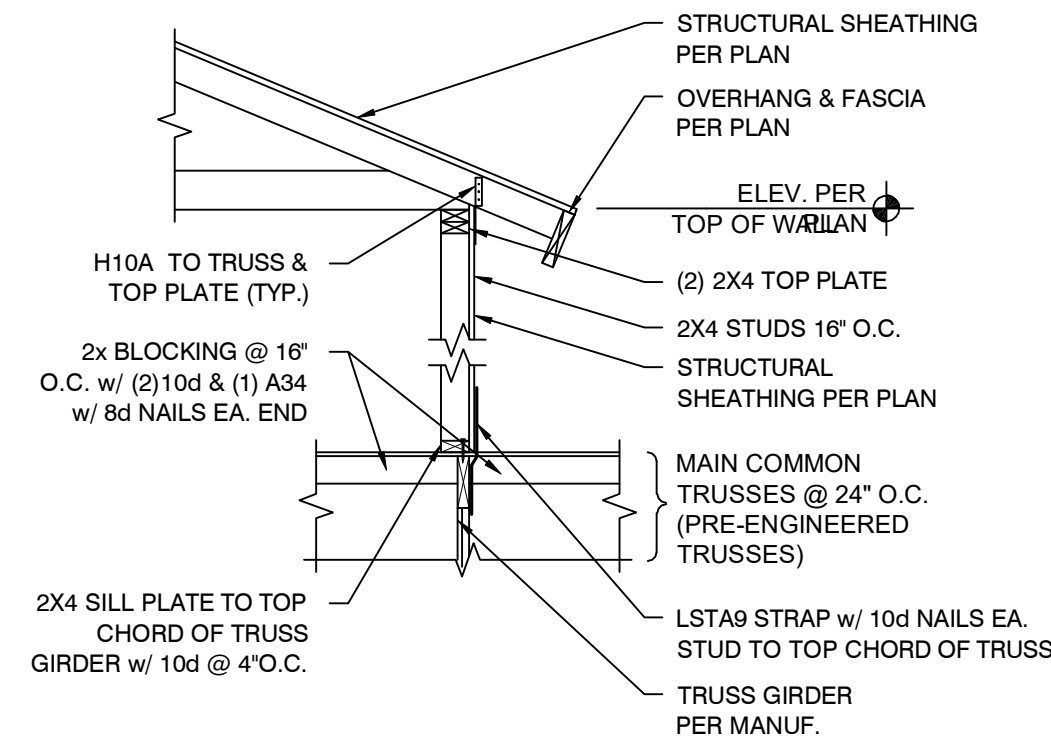
CONDITIONS OF USE:
1. UPLIFT VALUE FOR TRUSS LOCATION DOES NOT EXCEED 1,110 lbs. UPLIFT.
2. NOT ACCEPTABLE ON MULTI-PLY TRUSS APPLICATIONS.

IF FIELD CONDITIONS EXCEED CONDITIONS OF USE, CONTACT ENGINEER OR ARCHITECT OF RECORD FOR APPROPRIATE SOLUTION.

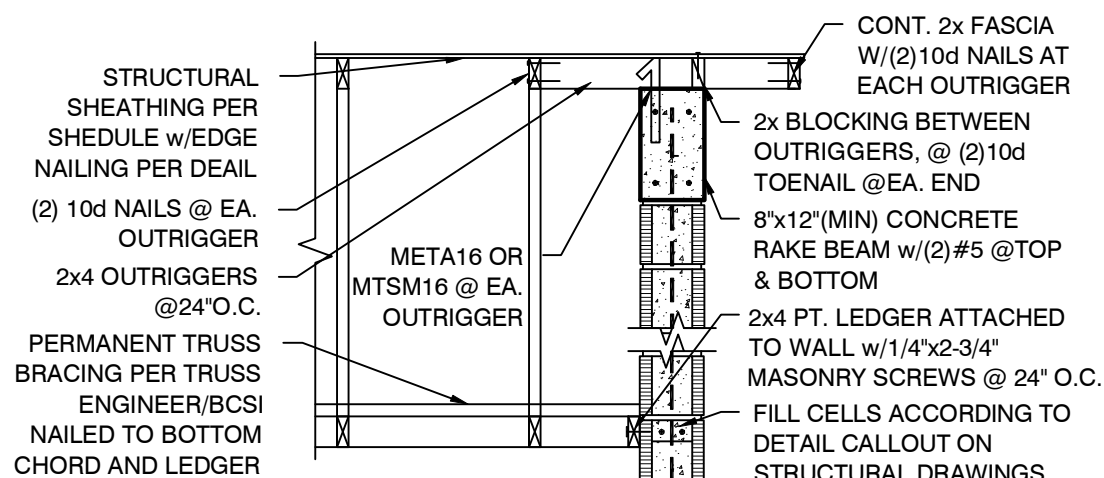
R13 MISSED TRUSS STRAP REPAIR
Scale: NOT TO SCALE
MOD (08/08/2025)
File Name: RWM-130 (TC-010)



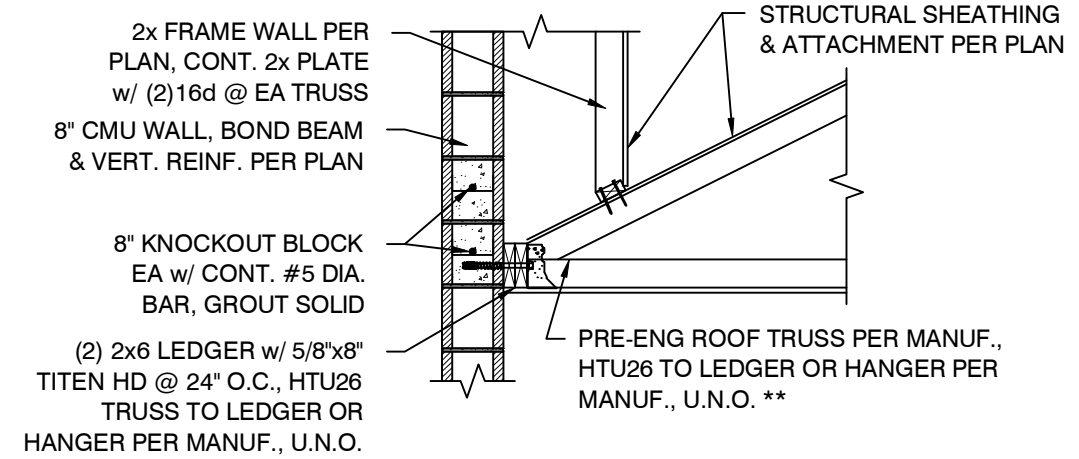
R15A ROOF FRAMING DETAIL
Scale: NOT TO SCALE
7/16/2021
File Name: RWG-151



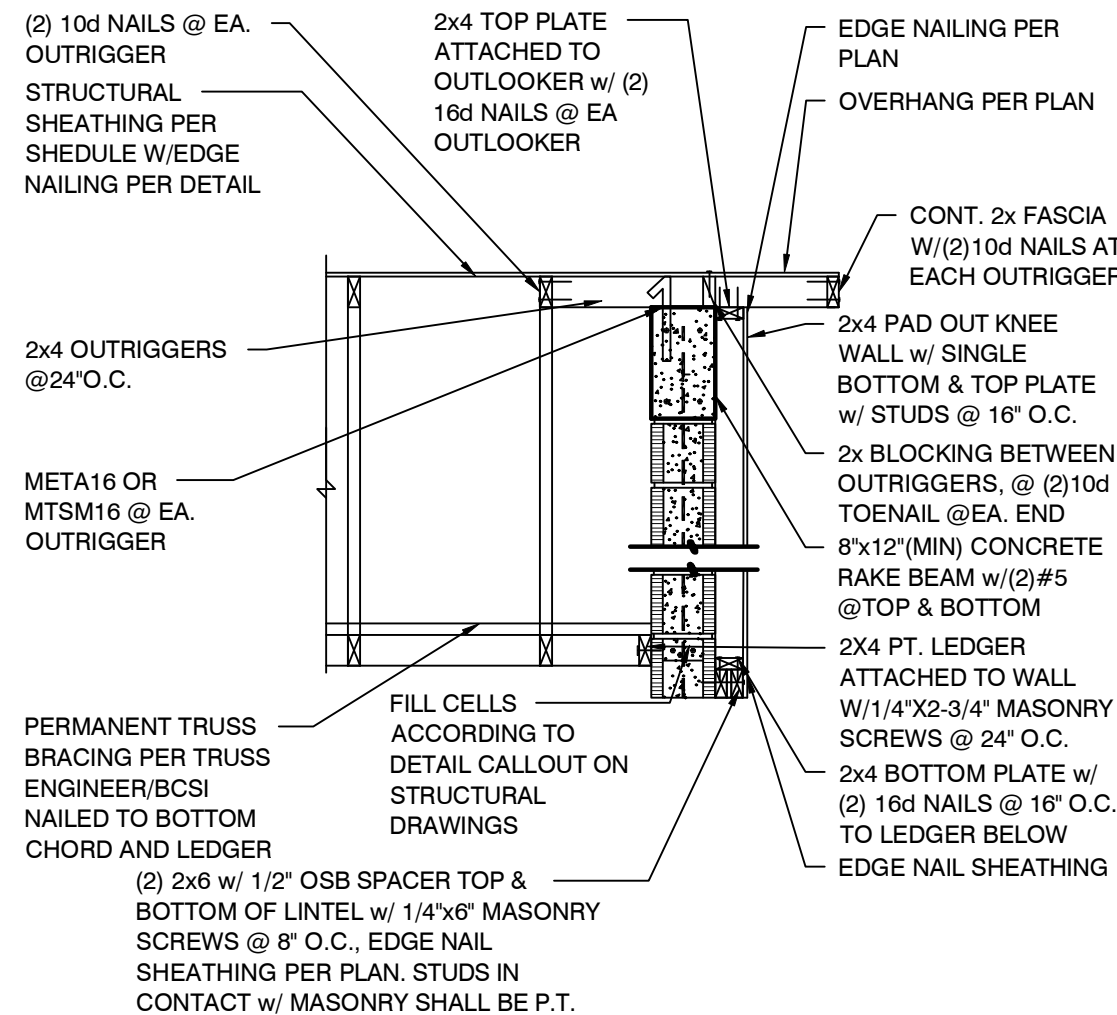
R15B CUPOLA ROOF FRAMING
Scale: NOT TO SCALE
7/16/2021
File Name: RWG-152



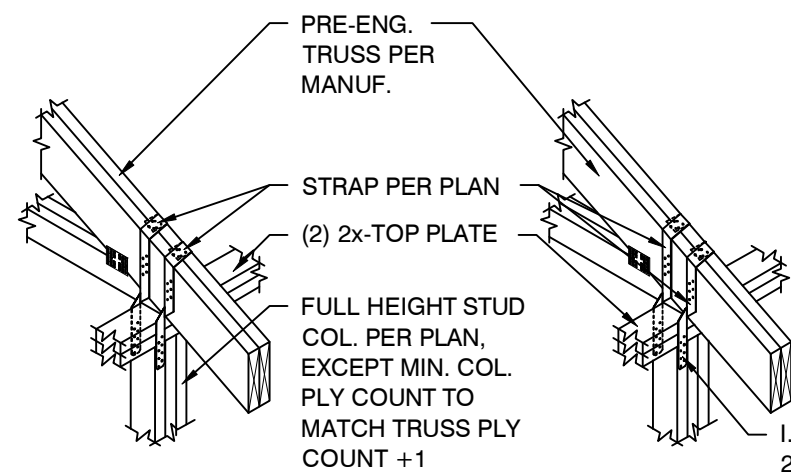
R33 GABLE END DETAIL @ CONCRETE RAKE BEAM
Scale: NOT TO SCALE
11/12/2021
File Name: RWM-330



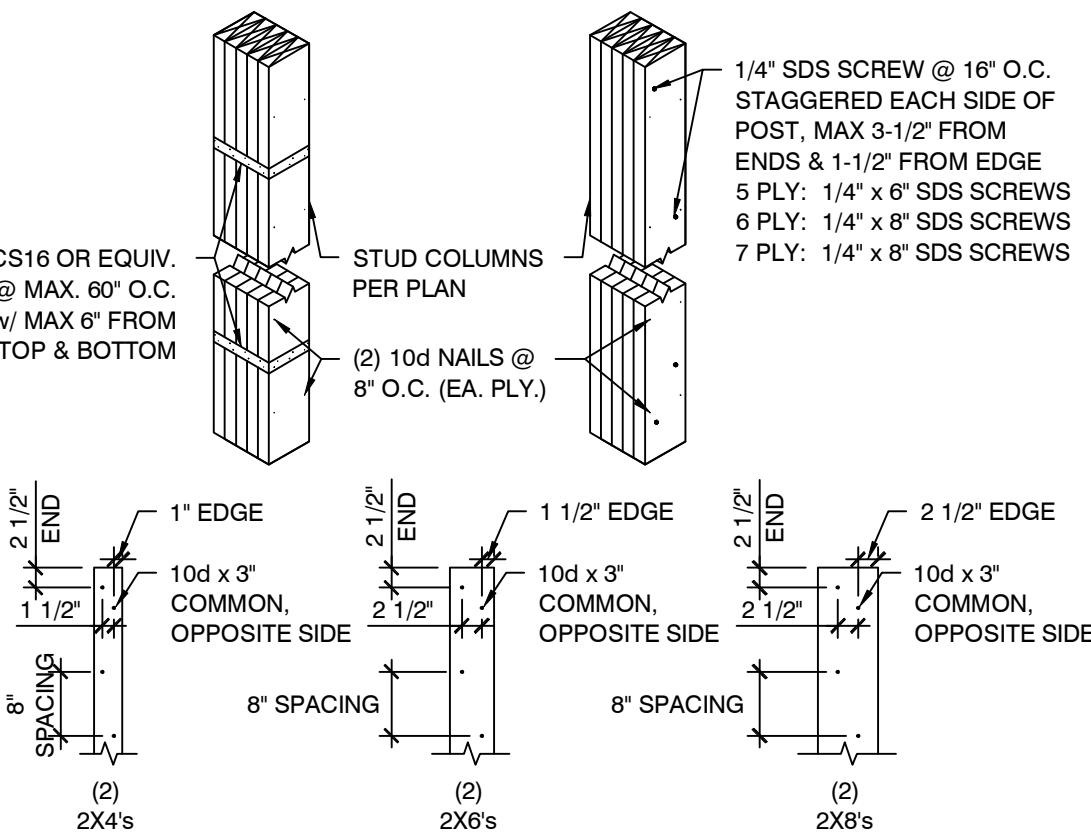
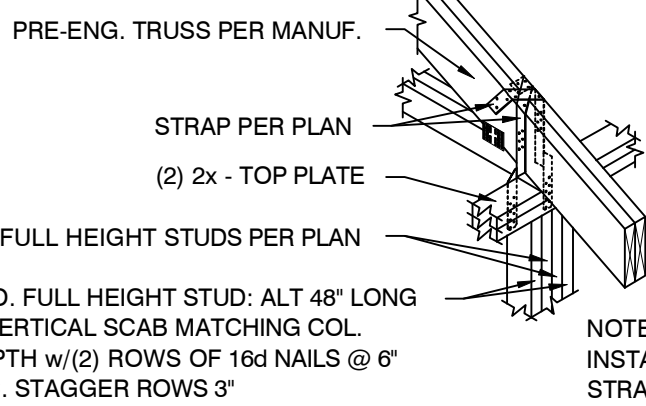
R38A NON-BEARING KNEE WALL
Scale: NOT TO SCALE
12/17/2021
File Name: RWM-381



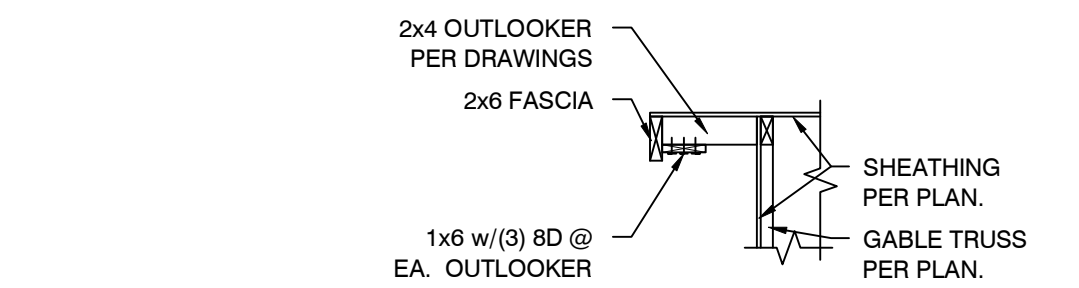
R33B GABLE END DETAIL @ CONCRETE RAKE BEAM
Scale: NOT TO SCALE
MOD (04/03/2023)
File Name: RWM-330 (FD-010)



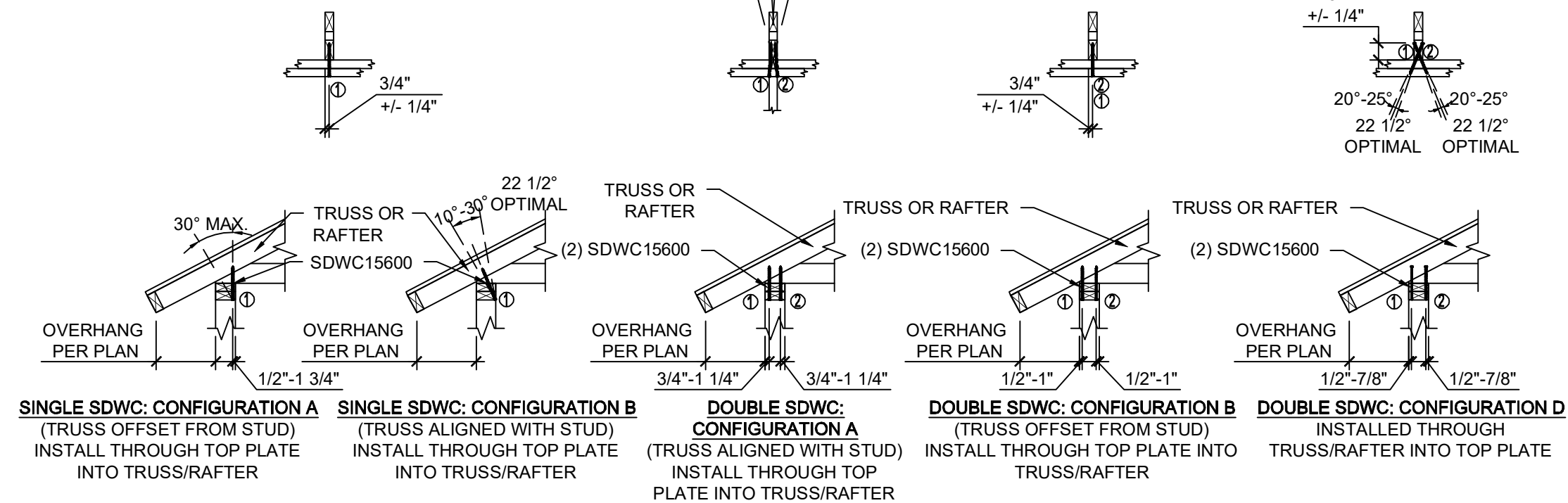
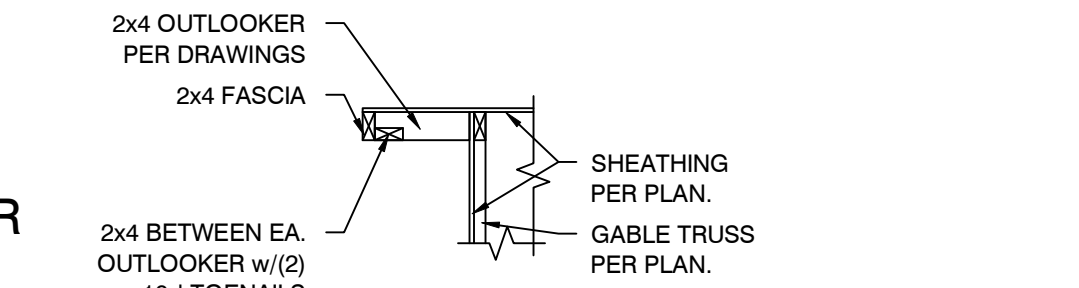
R60 TYPICAL GIRDER TRUSS TWIST STRAP INSTALLATION
Scale: NOT TO SCALE
03/27/2023
File Name: RWW-601



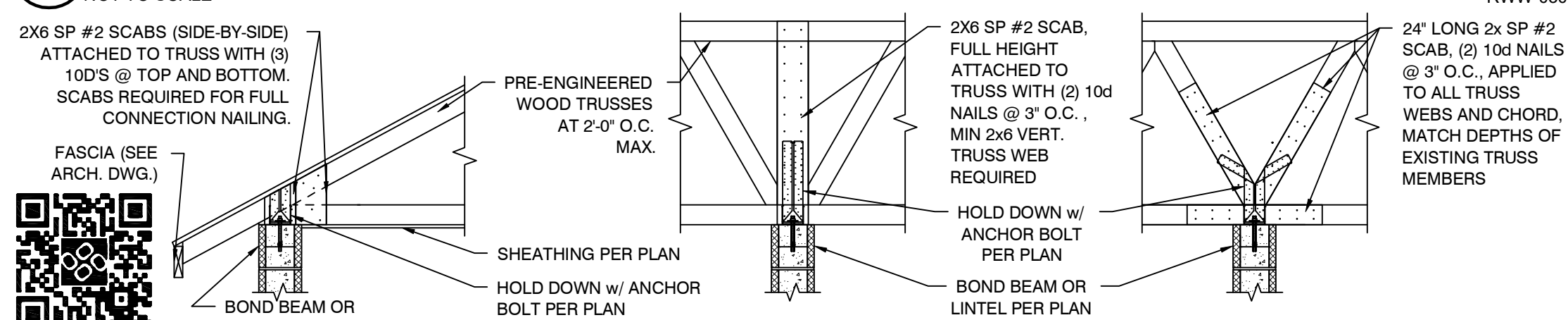
W03 STUD PACK DETAIL
Scale: NOT TO SCALE
03/32/2023
File Name: WGW-030



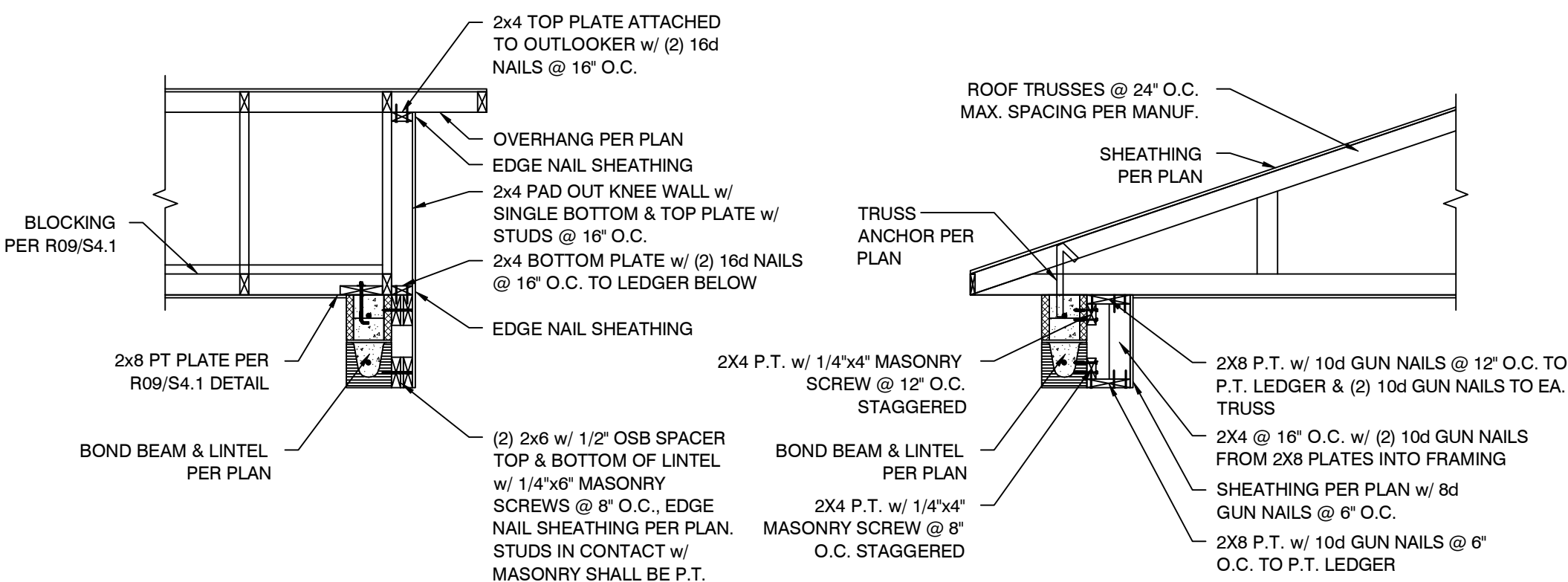
S15 PEDIMENT BLOCKING @ GABLE
Scale: NOT TO SCALE
03/23/2023
File Name: WSG-150



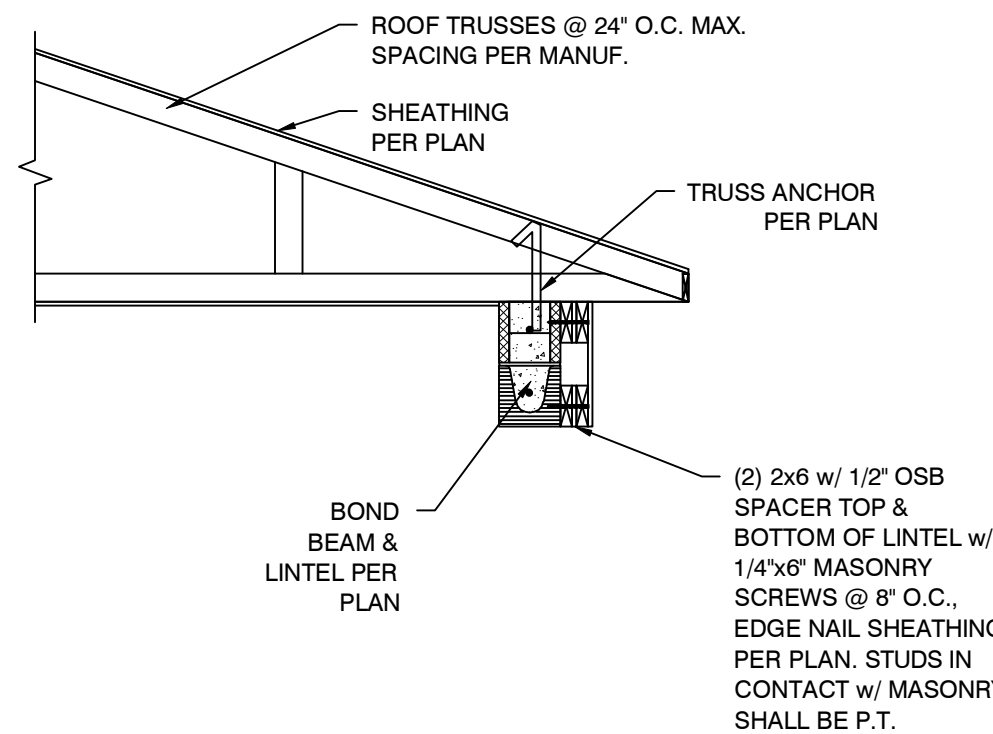
R03 TRUSS/RAFTER TO DOUBLE TOP PLATE SCREW CONNECTIONS
Scale: NOT TO SCALE
11/01/2023
File Name: RWW-030



R14 TRUSS GIRDER CONNECTION w/ SCAB
Scale: NOT TO SCALE
11/01/2023
File Name: RWM-140

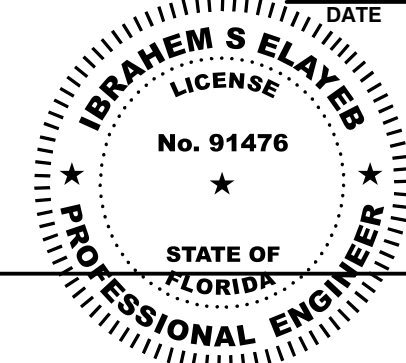


W37 LINTEL PAD OUT DETAIL
Scale: NOT TO SCALE
MOD (3/30/2023)
File Name: WGM-320-1(FD-010)



MOD (3/30/2023)
File Name: WGM-320-1(FD-010)

Fieldstone
3400 AUBURN RD., STE. 200
AUBURN HILLS, MI 48236
C.O.A.# ARCHITECTURE: AA26002802
C.O.E.# ENGINEERING: 29349
■ IBRAHEM ELAYEB, PE
P.E. #91476
□ IAN MCCULLOUGH, PE
P.E.# 80956 10/23/2025



Florida Zone
24311 Walden Center Dr., Suite 300
Bonita Spring, Florida 34134 (239)495-4800 (770)381-3450



FIELDSTONE
ARCHITECTURE & ENGINEERING
Michigan: 5400 Auburn Rd., Ste. 200 Auburn Hills, MI 48236
Florida: 15001 Tampa City Blvd., Ste. 100 Tampa, FL 33607
248.622.4035 fieldstoneAE.com 813.446.3410

Home for * Market Home * 8338-09300
6990 Minneola Court, Nokomis, FL 34275
DETAILS - ROOF

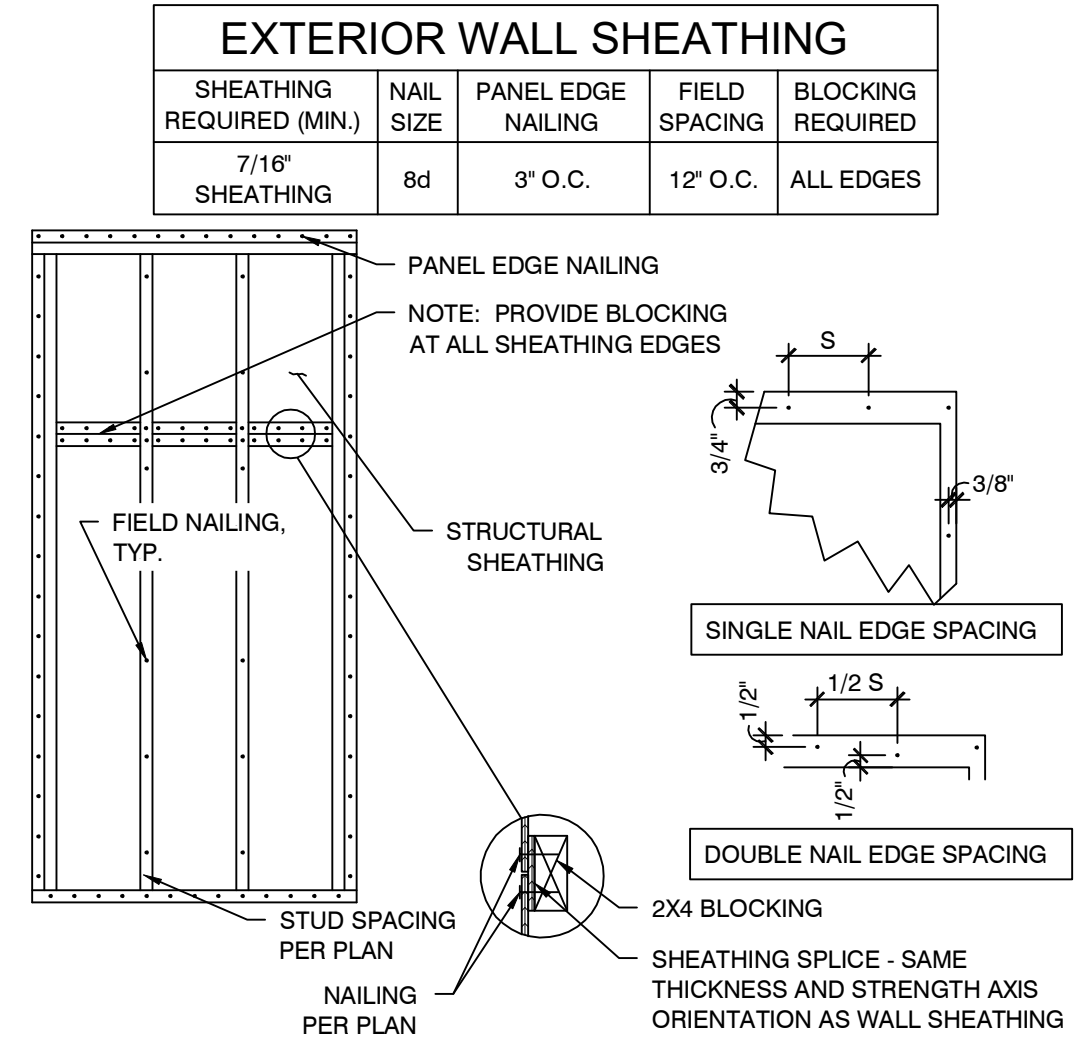
PRODUCT MANAGER
RELEASE
DATE: 10/23/2025
NEW DATE/DESCRIPTION
P.E. #91476
P.E. #80956

PROJECT TYPE
Single Family
SPECIFICATION LEVEL
Pulte

PLAN NAME
Mystique
NPC CHILD NUMBER
2684.500

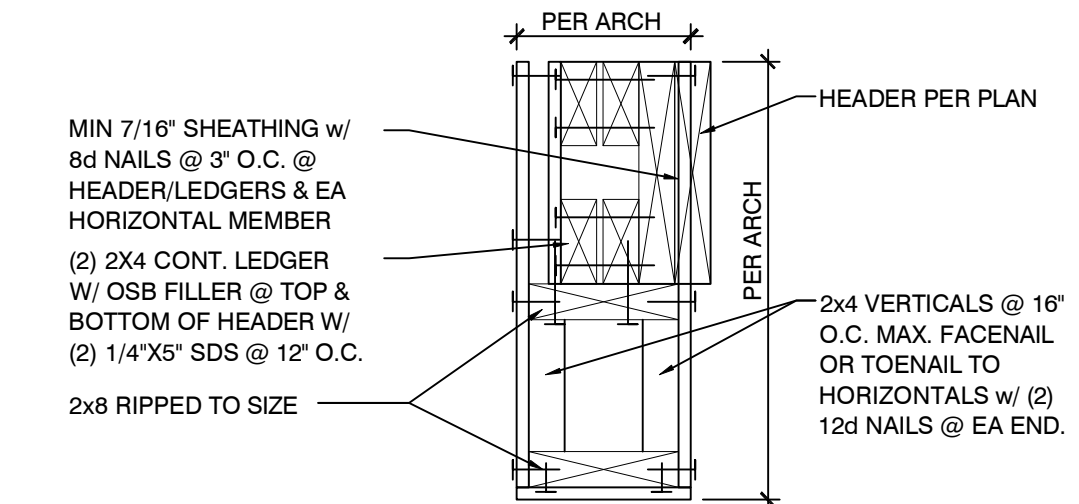
SHEET
S4.1.1

PLOTTED: October 20, 2025 / Proconmk / \$3.00.dwg



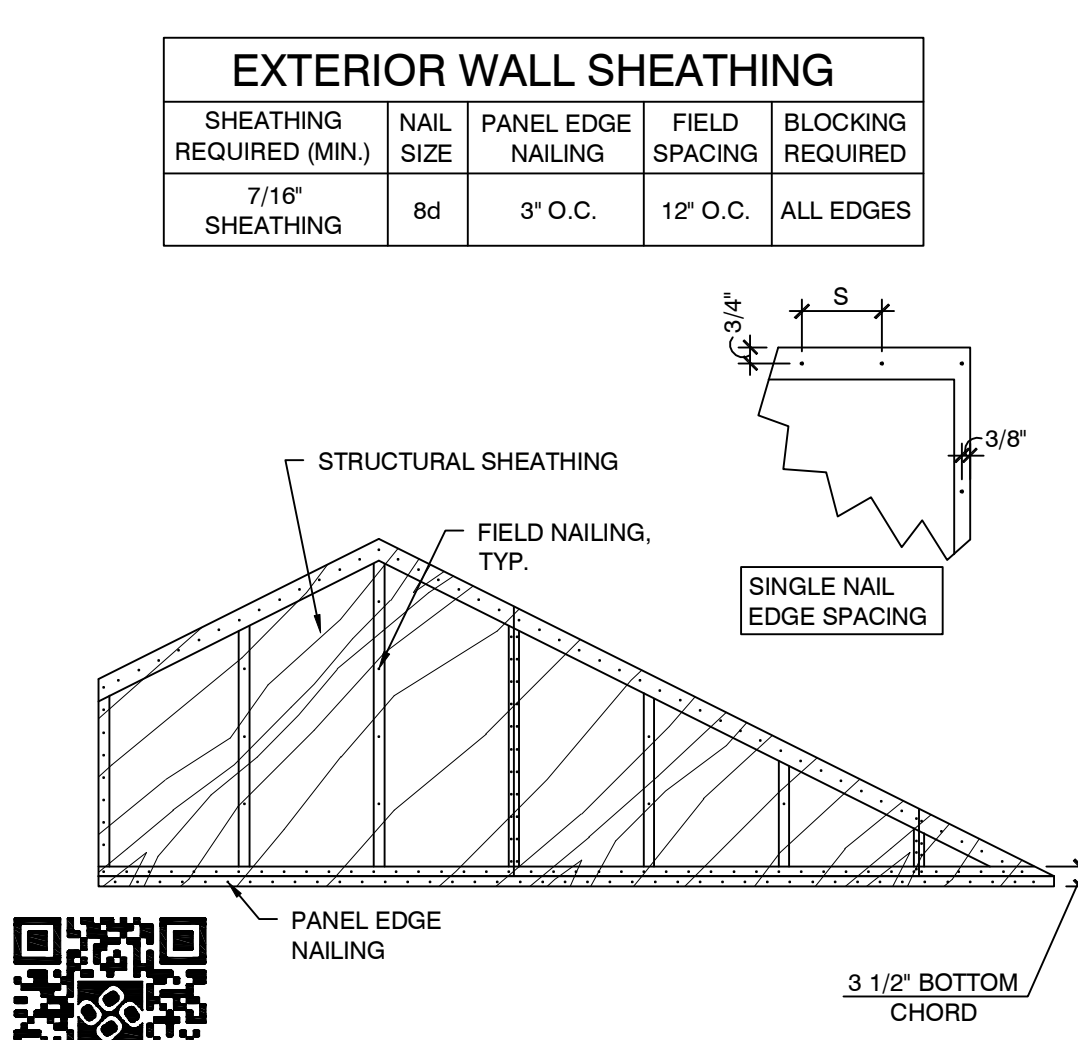
W01 TYPICAL SHEATHING DETAIL

11/01/2023
File Name: WGW-010



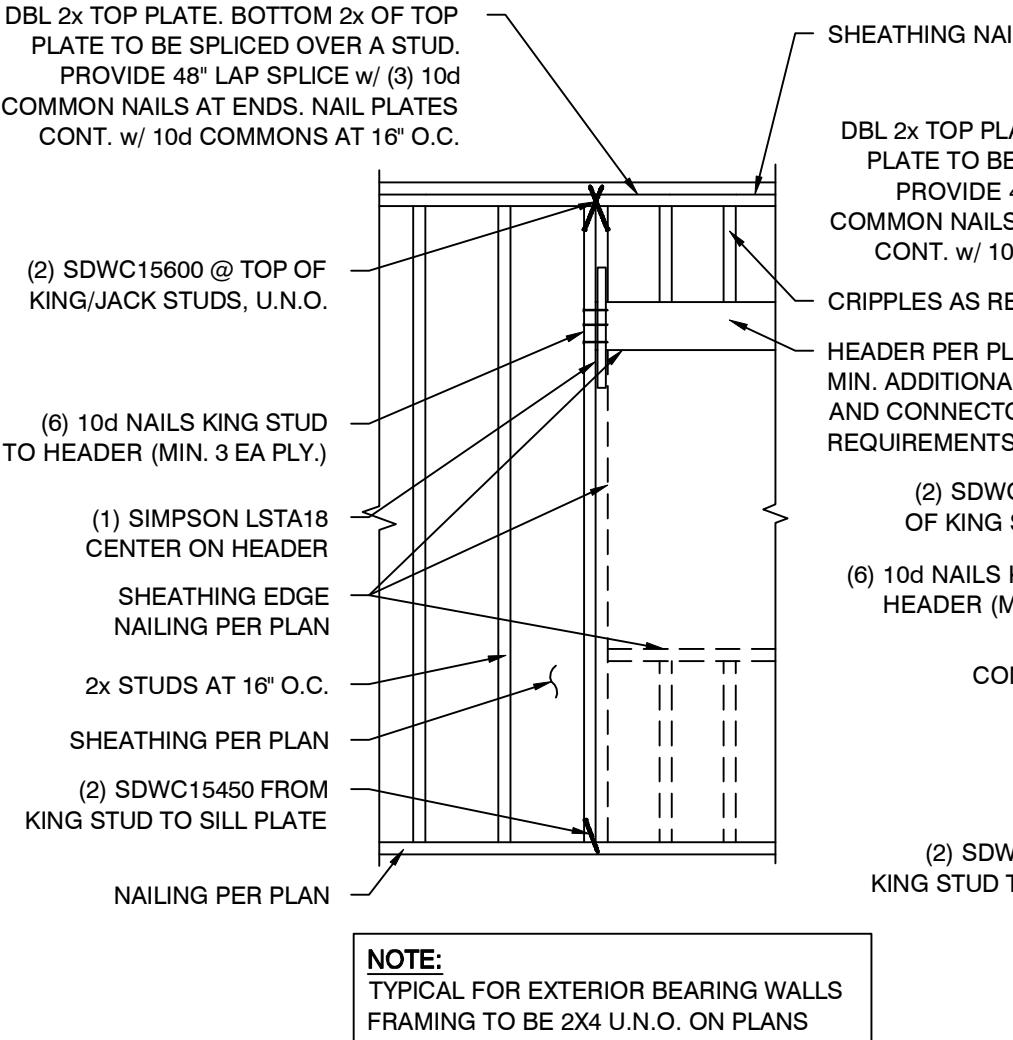
W03C BOX OUT DETAIL

NOT TO SCALE



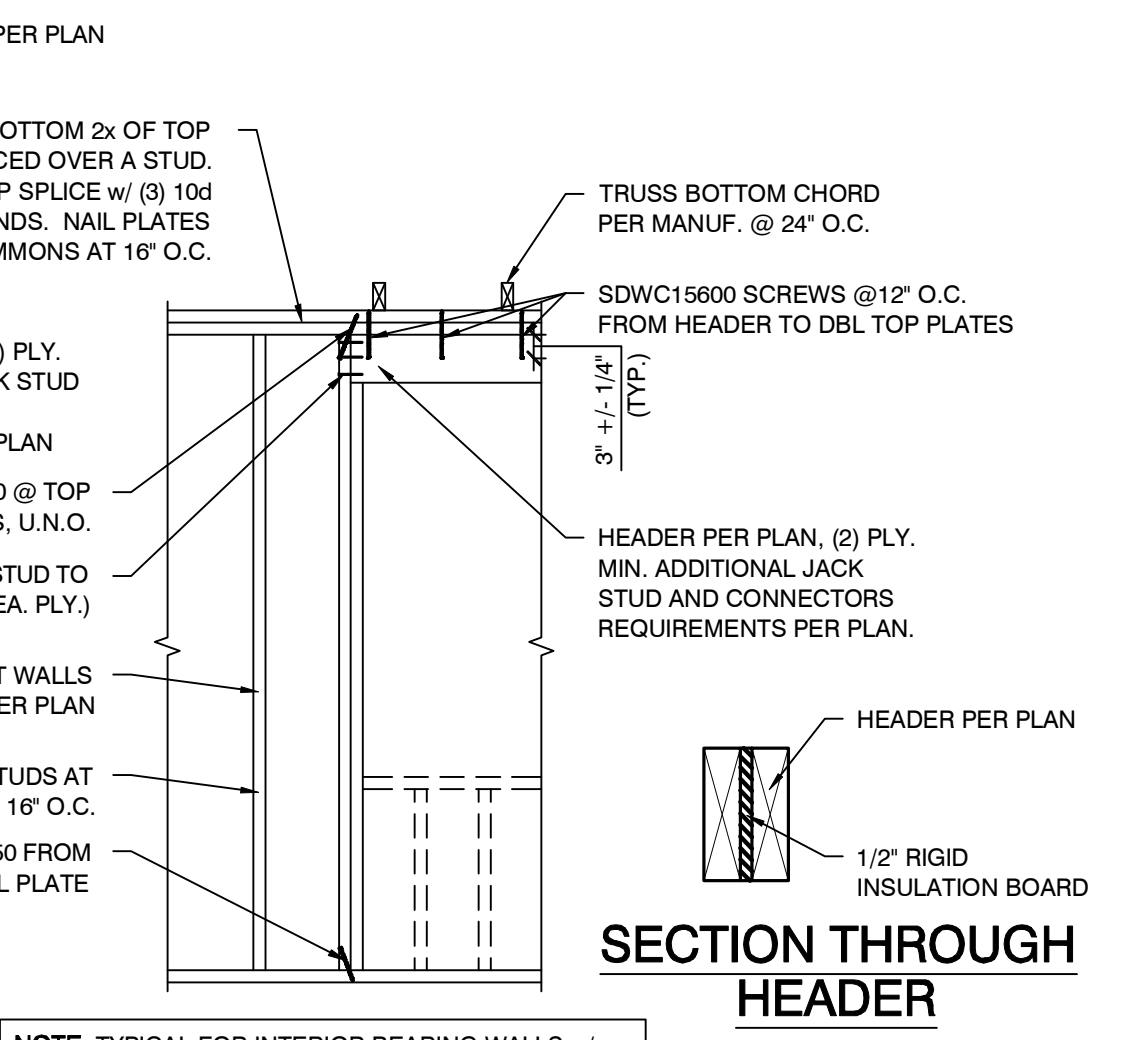
W02 GABLE END SHEATHING DETAIL

11/01/2023
File Name: WGG-020



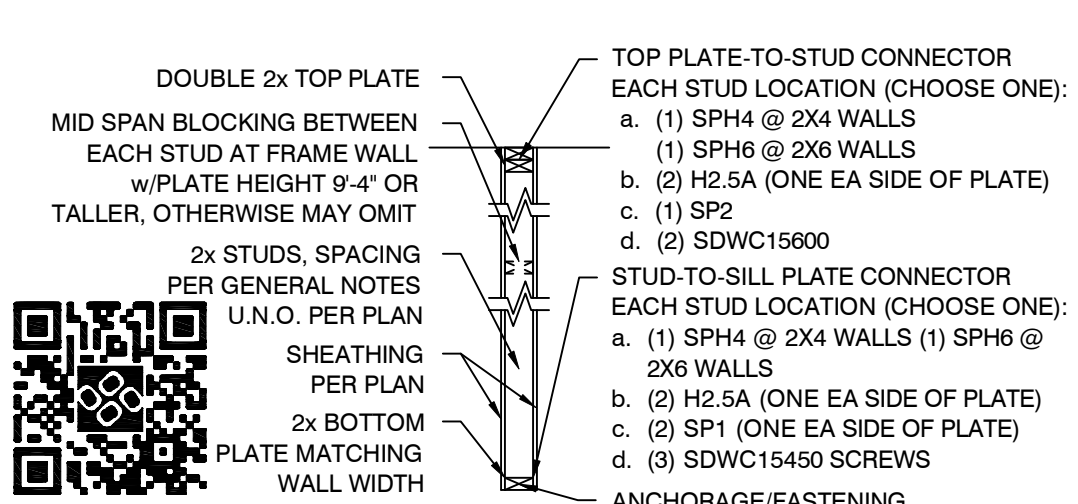
W05 TYPICAL FRAMED BEARING WALL

NOT TO SCALE



SECTION THROUGH HEADER

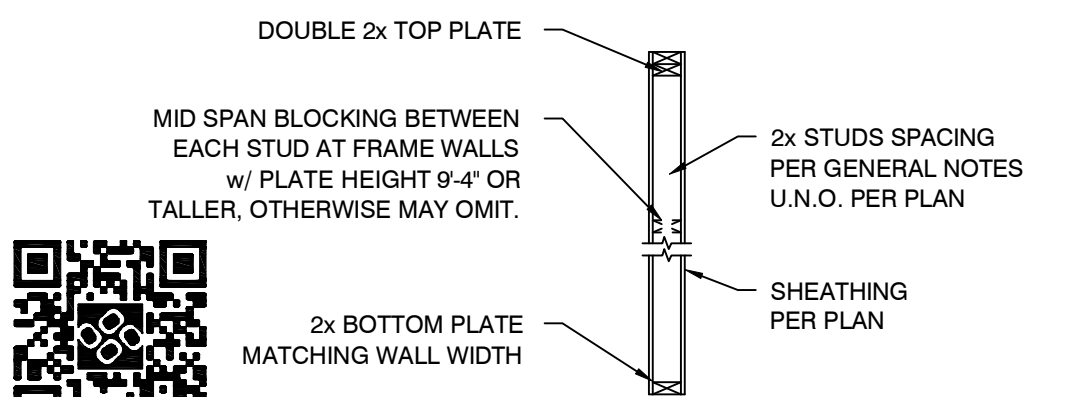
01/27/2025
File Name: WGW-056



W10 LOAD BEARING WALL w/ UPLIFT

11/01/2023
File Name: WGW-105

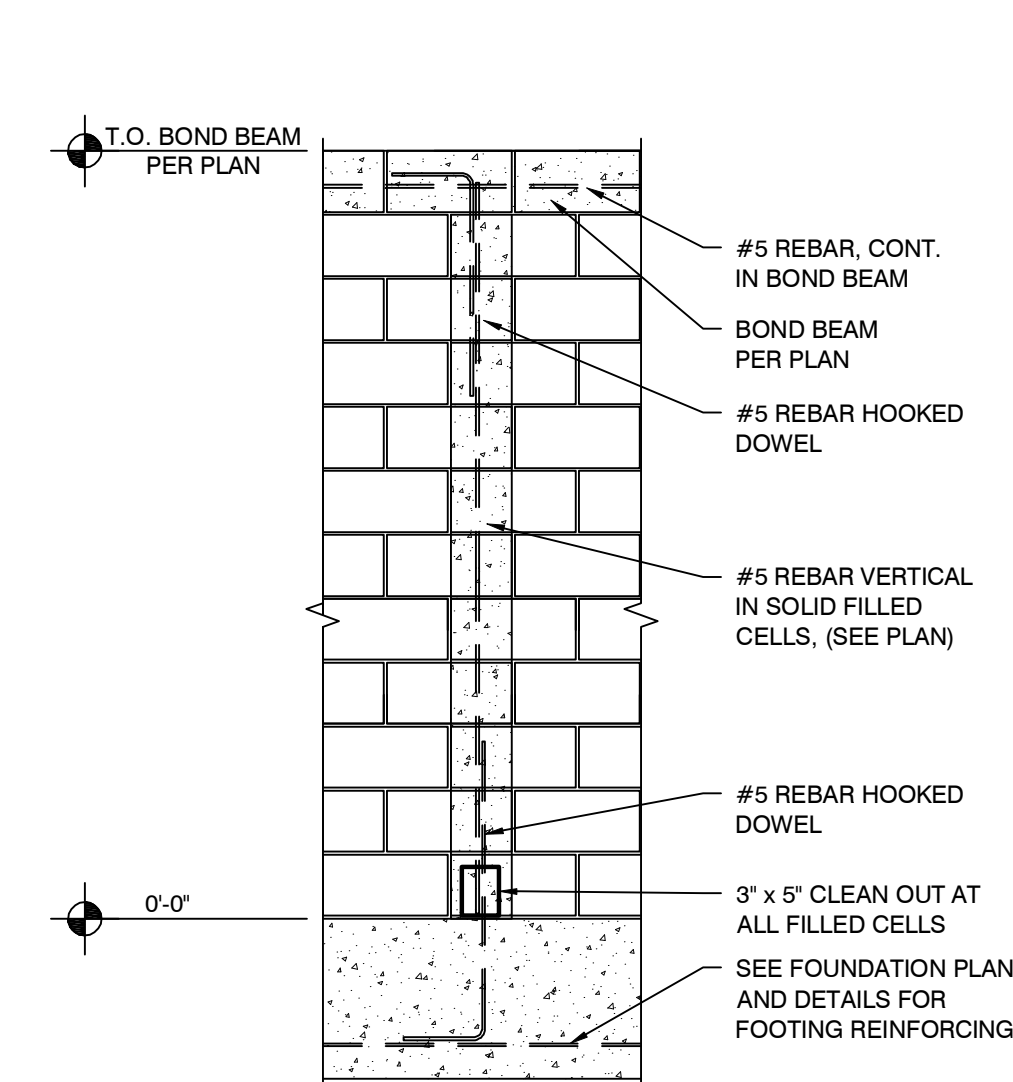
NOT TO SCALE



W11 LOAD BEARING WALL w/o UPLIFT

11/01/2023
File Name: WGW-111

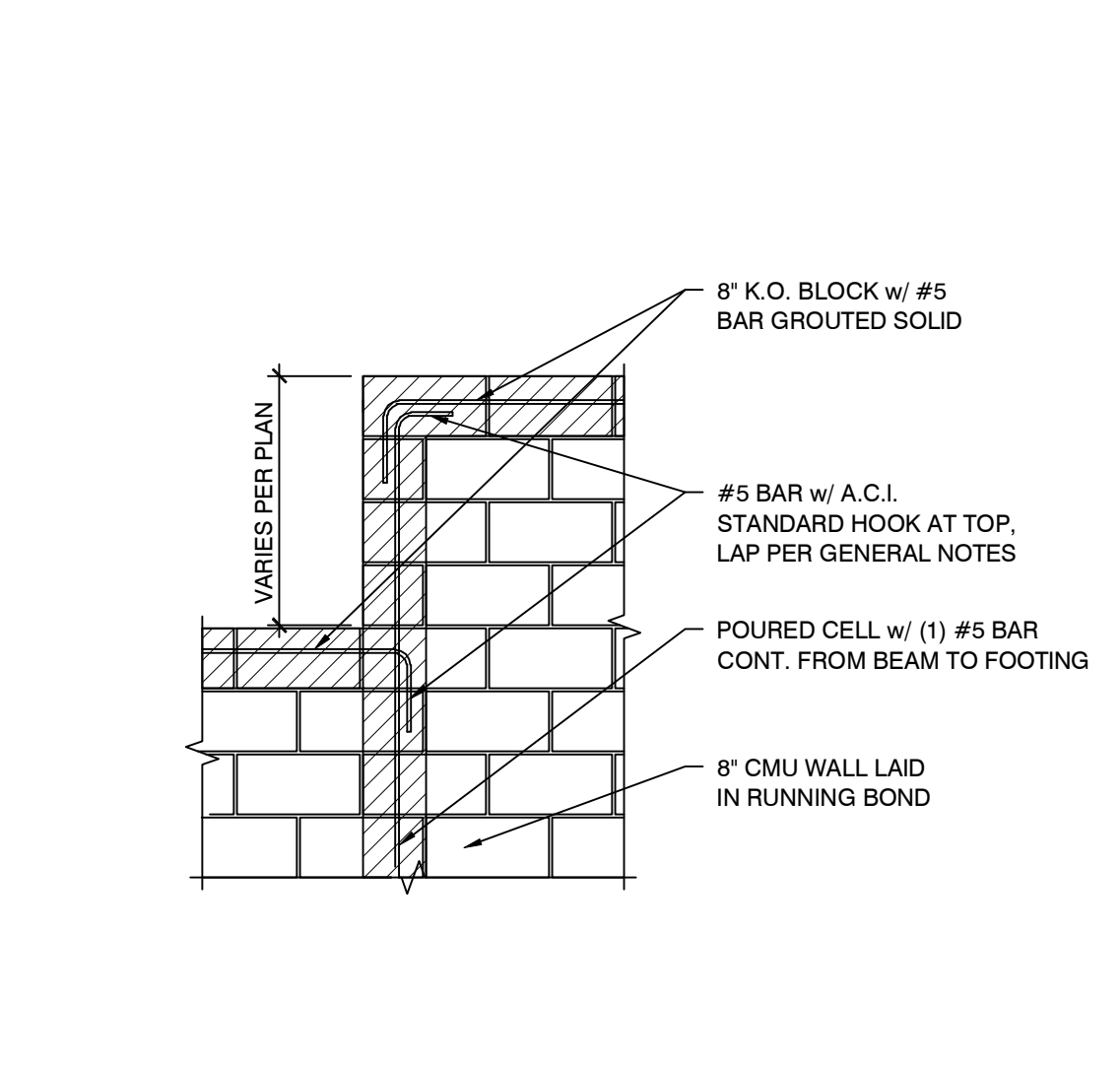
NOT TO SCALE



W19 TYPICAL REINFORCED FILLED CELL DETAIL

11/01/2023
File Name: WGM-190

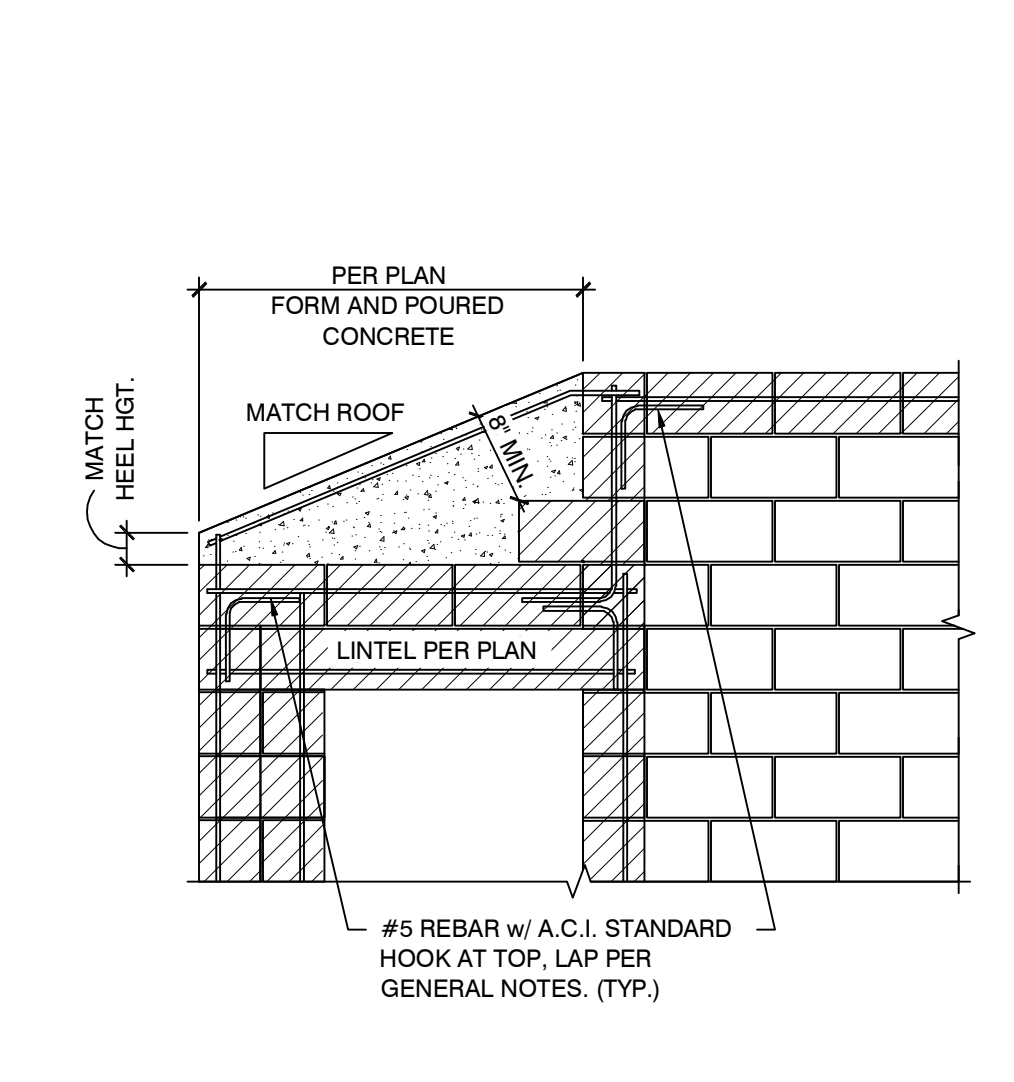
NOT TO SCALE



W21 STEPPED BOND BEAM

7/16/2021
File Name: WGM-210

NOT TO SCALE



W42C RAKED BEAM @ OPENING

7/16/2021
File Name: WGM-423

NOT TO SCALE

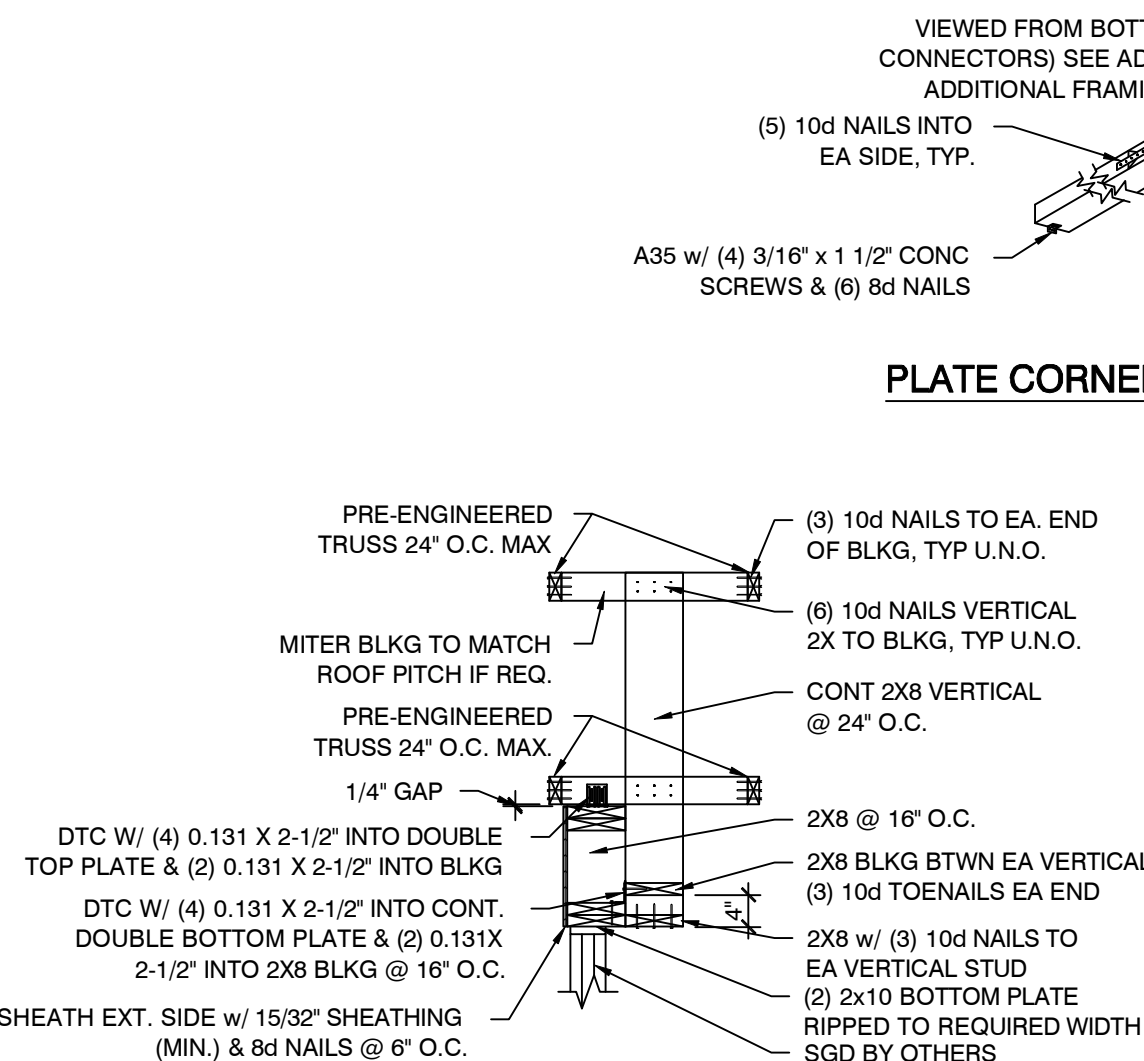
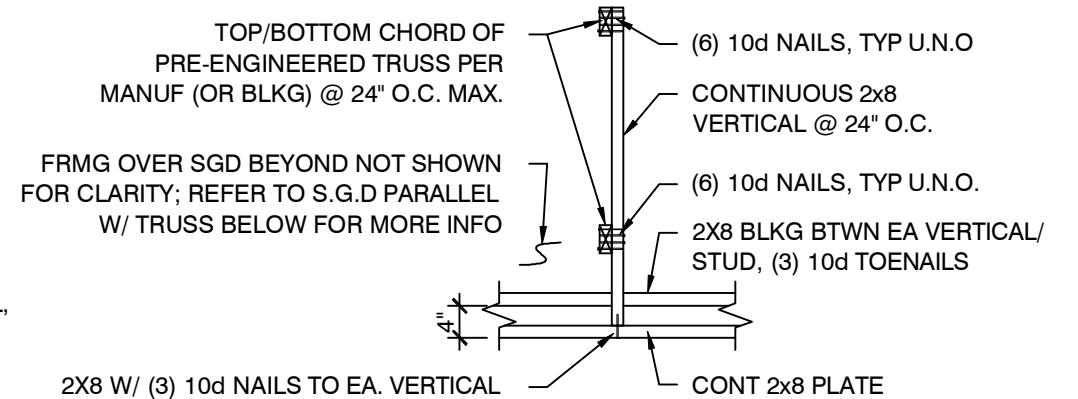


PLATE CORNER ATTACHMENT

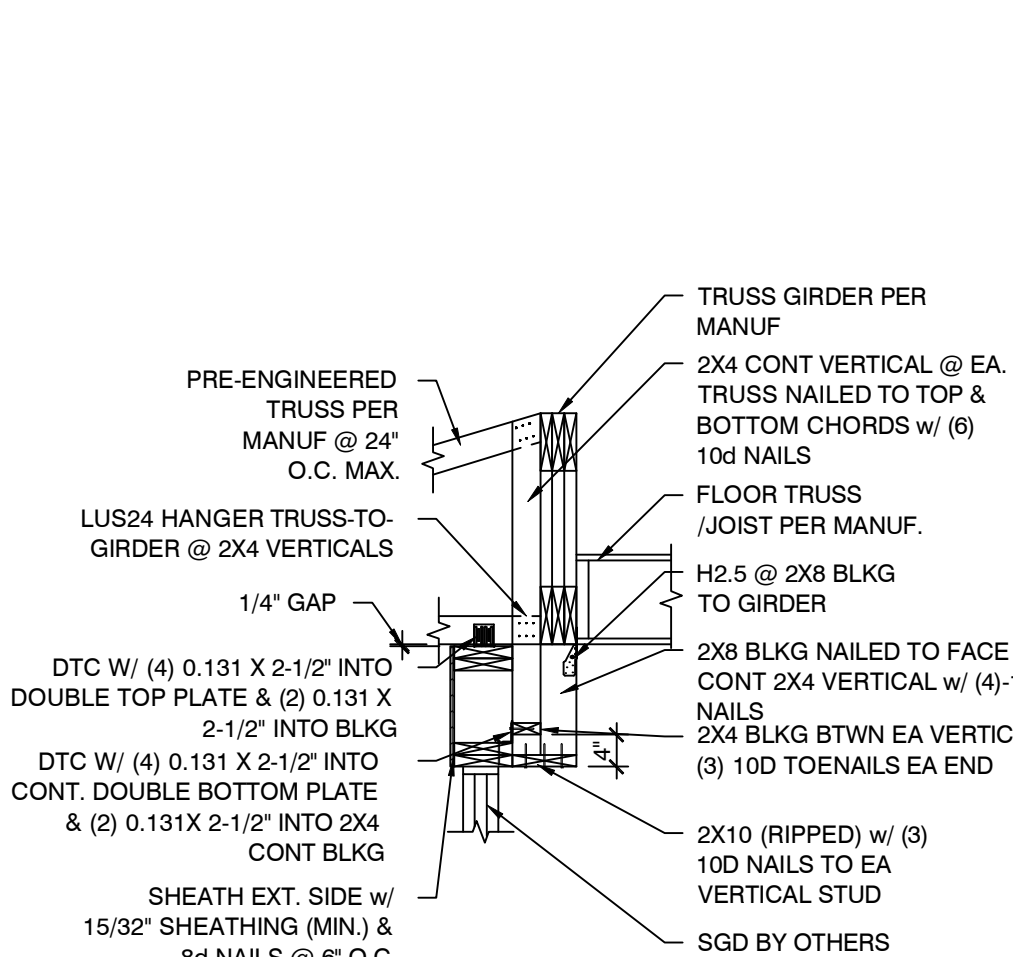


S.G.D. PERPENDICULAR TO TRUSS

W33A FRAMED ZERO CORNER SLIDING GLASS DOOR

5/2/2023
File Name: WGG-330

NOT TO SCALE

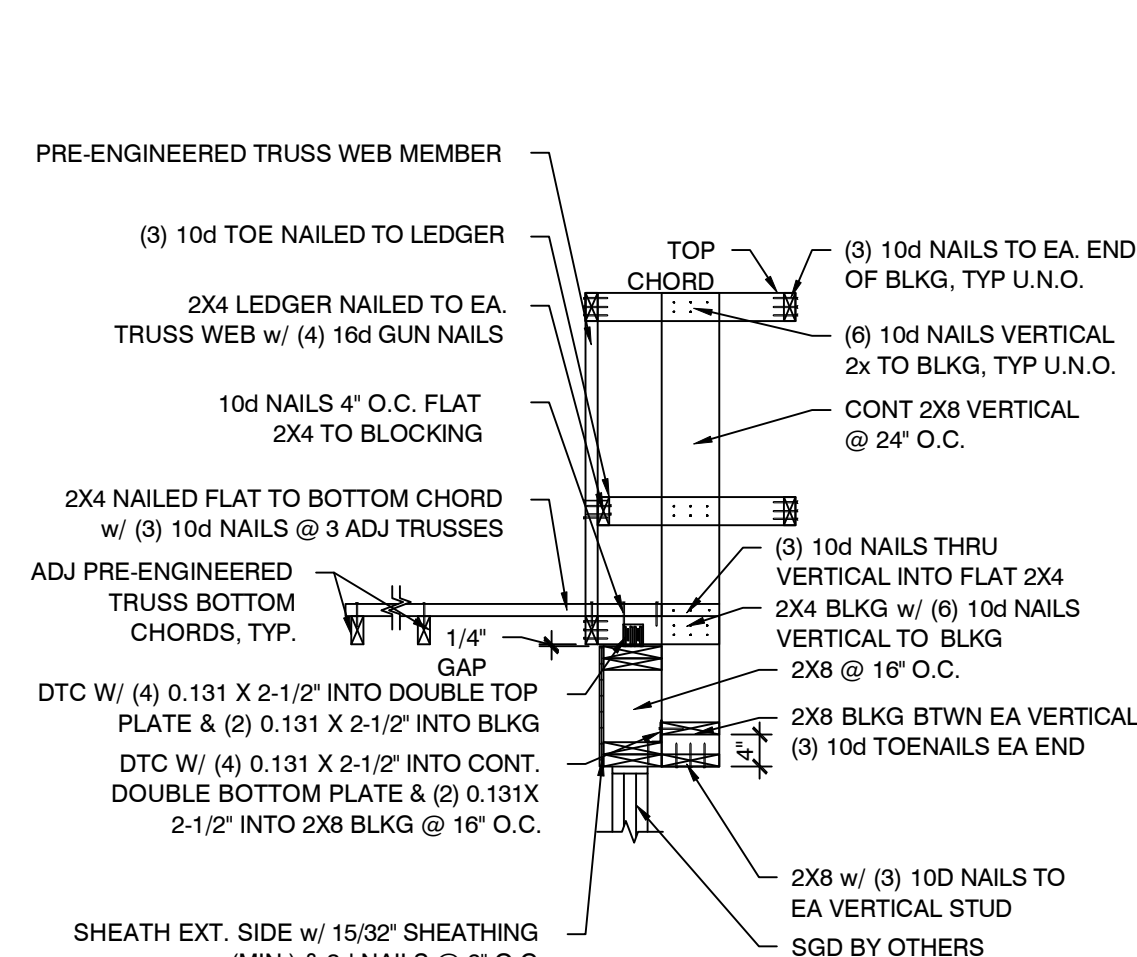


S.G.D. w/ GIRDER @ EXTERIOR WALL

W33B FRAMED ZERO CORNER SLIDNG GLASS DOOR

5/2/2023
File Name: WGG-331

NOT TO SCALE



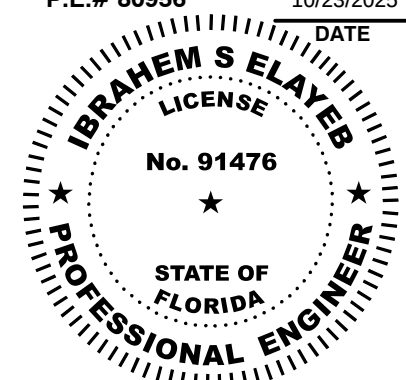
S.G.D. PARALLEL WITH TRUSS & CEILING HEIGHT VARYING

W33C FRAMED ZERO CORNER SLIDNG GLASS DOOR

5/2/2023
File Name: WGG-332

NOT TO SCALE

Fieldstone
3400 AUBURN RD., STE. 200
AUBURN HILLS, MI 48236
C.O.A.# ARCHITECTURE: AA26002802
C.O.E.# ENGINEERING: 29349
■ IBRAHEM ELAYEB, PE
P.E. #91476
□ IAN MCCULLOUGH, PE
P.E.# 80956



Florida Zone
24311 Walden Center Dr., Suite 300
Bonita Spring, Florida 34134 (239)495-4800 (770)381-3450



FIELDSTONE
ARCHITECTURE & ENGINEERING
Michigan: 5400 Auburn Rd., Ste. 200 Auburn Hills, MI 48236
Florida: 1900 Tampa Oaks Blvd., Ste. 180 Tampa, FL 33607
248.622.4035 FieldstoneAE.com 813.446.3310

Home for * Market Home * 8338-09300
6990 Minneola Court, Nokomis, FL 34275
WALL DETAILS

PRODUCT MANAGER	
RELEASE	DATE: #####
REV	DATE/DESCRIPTION

PROJECT TYPE
Single Family

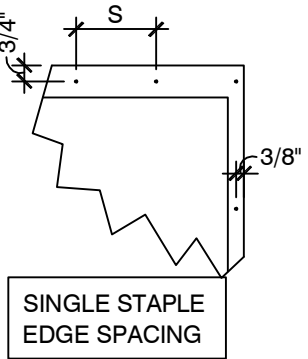
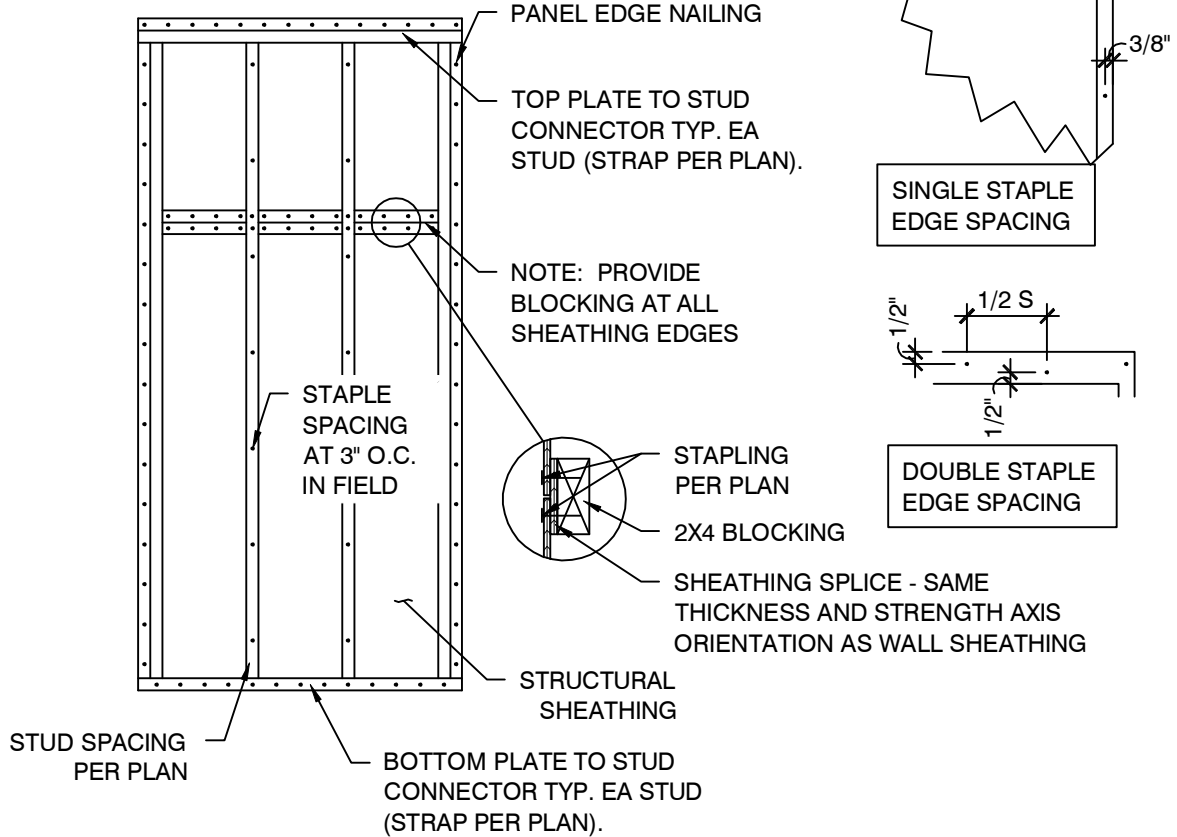
SPECIFICATION LEVEL
Pulte

PLAN NAME
Mystique
NPC CHILD NUMBER
2684.500

SHEET
S4.2

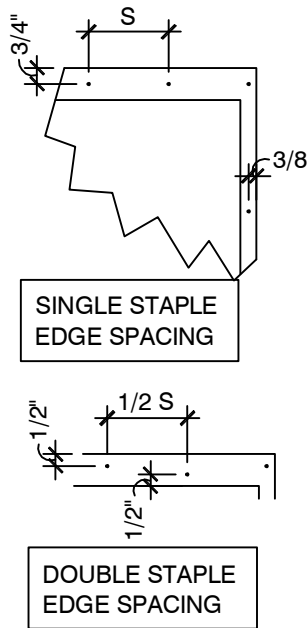
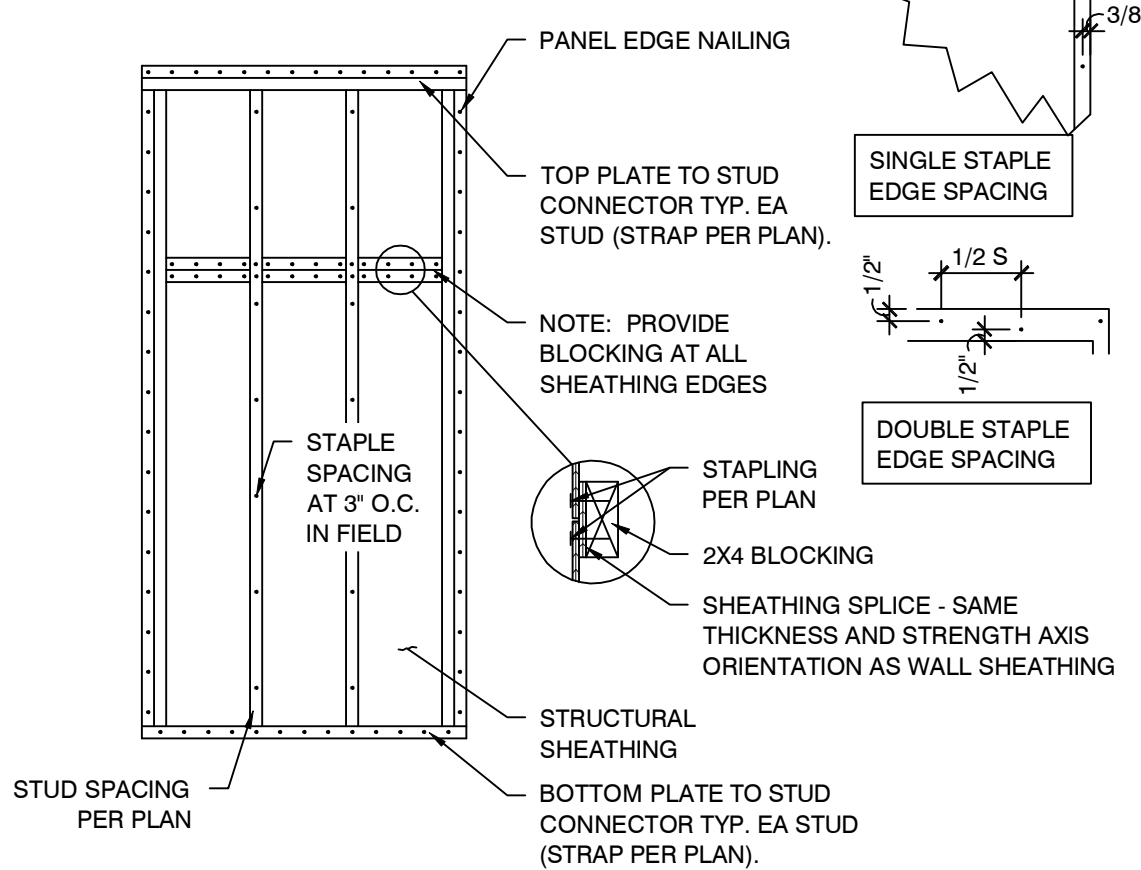
LOFT WALL SHEATHING				
SHEATHING REQUIRED (MIN.)	STAPLE SIZE	PANEL EDGE NAILING	FIELD SPACING	BLOCKING REQUIRED
THERMO-PLY GREEN	15"x16" CROWN x 1-3/4" LEG	3" O.C.	3" O.C.	ALL EDGES

- NOTE:
- ALL LOFT FRAMING SHALL CONSIST OF 2x STUDS AT 16" O.C. MAX SPACING
 - ATTACH 1/2" GYPSUM WALL BOARD TO INTERIOR SIDE OF STUDS w/ #6x1-1/4" TYPE W OR S SCREWS OR 5d COOLER NAILS w/16" O.C. EDGE & FIELD SPACING
 - IN ADDITION TO THERMO-PLY SHEATHING, INSTALL STUD TO TOP/SILL PLATE STRAPPING PER W10/S4.2. ADDITIONAL STRAPPING FROM UPPER LEVEL WALLS TO SUPPORTING TRUSSES/WALL FRAMING BELOW TO BE NOTED ON FRAMING PLANS OR DETAILS.
 - MINIMUM 7/16" WOOD STRUCTURAL PANELS (OR ZIP SYSTEM ® SHEATHING) MAY BE USED IN LIEU OF THERMO-PLY SHEATHING FOR ALL INSTANCES OF THIS DETAIL NOTED ON THE PLANS. REFER TO DETAIL W01/S4.2 FOR FASTENING REQUIREMENTS WHEN WOOD STRUCTURAL PANELS ARE USED.



LOFT WALL SHEATHING				
SHEATHING REQUIRED (MIN.)	STAPLE SIZE	PANEL EDGE NAILING	FIELD SPACING	BLOCKING REQUIRED
THERMO-PLY RED	15"x16" CROWN x 1-3/4" LEG	3" O.C.	3" O.C.	ALL EDGES

- NOTE:
- ALL LOFT FRAMING SHALL CONSIST OF 2x STUDS AT 16" O.C. MAX SPACING
 - ATTACH 1/2" GYPSUM WALL BOARD TO INTERIOR SIDE OF STUDS w/ #6x1-1/4" TYPE W OR S SCREWS OR 5d COOLER NAILS w/16" O.C. EDGE & FIELD SPACING
 - IN ADDITION TO THERMO-PLY SHEATHING, INSTALL STUD TO TOP/SILL PLATE STRAPPING PER W10/S4.2. ADDITIONAL STRAPPING FROM UPPER LEVEL WALLS TO SUPPORTING TRUSSES/WALL FRAMING BELOW TO BE NOTED ON FRAMING PLANS OR DETAILS.
 - MINIMUM 7/16" WOOD STRUCTURAL PANELS (OR ZIP SYSTEM ® SHEATHING) MAY BE USED IN LIEU OF THERMO-PLY SHEATHING FOR ALL INSTANCES OF THIS DETAIL NOTED ON THE PLANS. REFER TO DETAIL W01/S4.2 FOR FASTENING REQUIREMENTS WHEN WOOD STRUCTURAL PANELS ARE USED.



AT CMU WALLS USE 2x P.T. ATTACHED TO BLOCK OR LINTEL WITH POWER DRIVEN FASTENERS AT 12" O.C. FASTENERS TO BE 1/8" DIA. MIN. AND EMBEDDED 3/4" MIN. MINIMUM SHEAR CAPACITY TO BE 150#.

2x ANGLE AS REQUIRED w/ (2) 10d TOE NAILS AT EACH END.

STRUCTURAL SHEATHING REQUIRED AT 2 SIDES IN WINDOWS AND DOORS AND AT ENTRIES WITHOUT BARREL FRAMING. SHEATH OUTSIDE ONLY WHEN BARREL FRAMING IS USED.

2X4 FILL w/ 2" MAX. GAP BETWEEN BLOCKS. BLOCK AS REQUIRED AT WINDOW FASTENER LOCATIONS

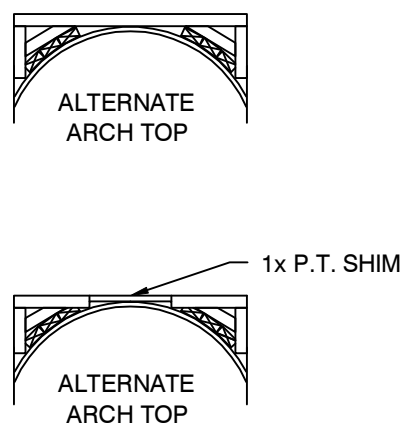
0"-16" MAX. GAP

NOTES:

MINOR CHANGES MAY OCCUR ACCORDING TO INDIVIDUAL OPENING.

IF WINDOW OR DOOR IS INSTALLED IN OPENING SEE "TYPICAL WINDOW AND DOOR ATTACHMENT DETAIL" FOR INSTALLATION DETAIL.

WHERE WINDOW FASTENER DOES NOT HAVE 2x BACKING, THE FASTENER MUST EXTEND THROUGH 1x SHIM INTO BASE MATERIAL PER THE MANUFACTURER'S PUBLISHED RECOMMENDATIONS.

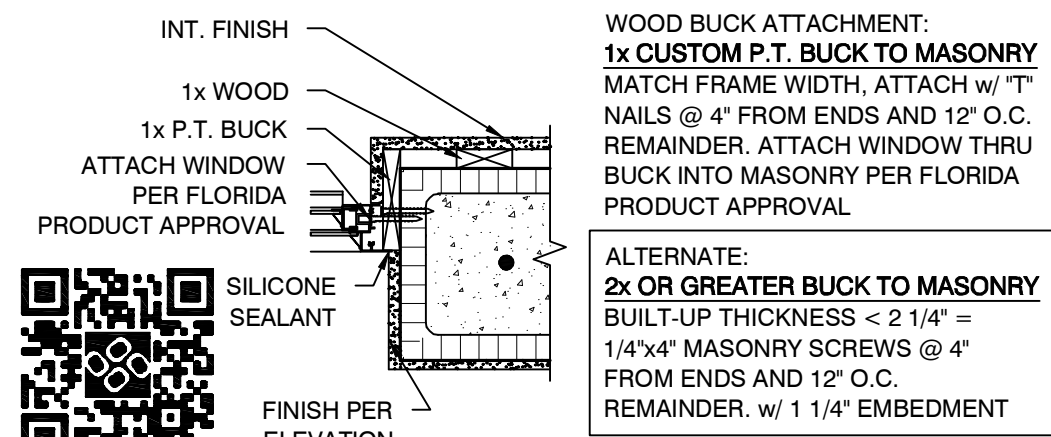


W01G GREEN THERMO-PLY SHEATHING DETAIL

Scale: NOT TO SCALE

11/01/2023

File Name: WGW-011

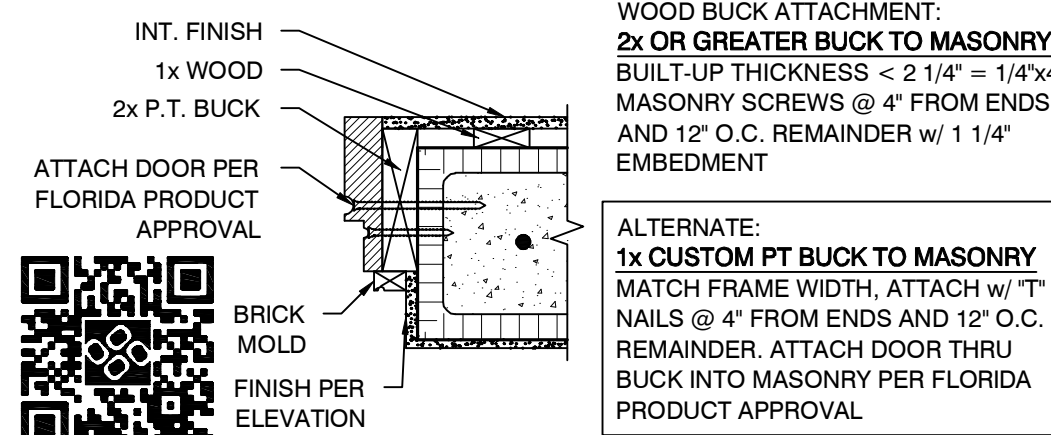


W14

Scale: NOT TO SCALE

11/01/2023

File Name: WGM-140

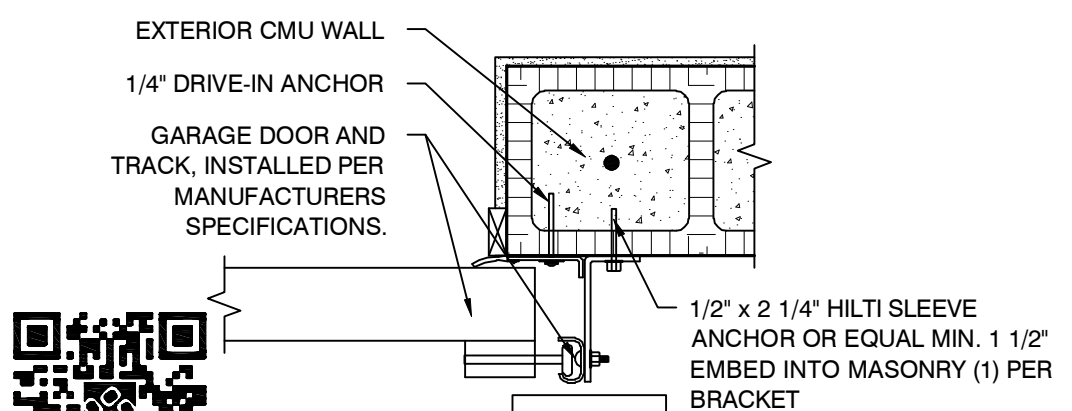
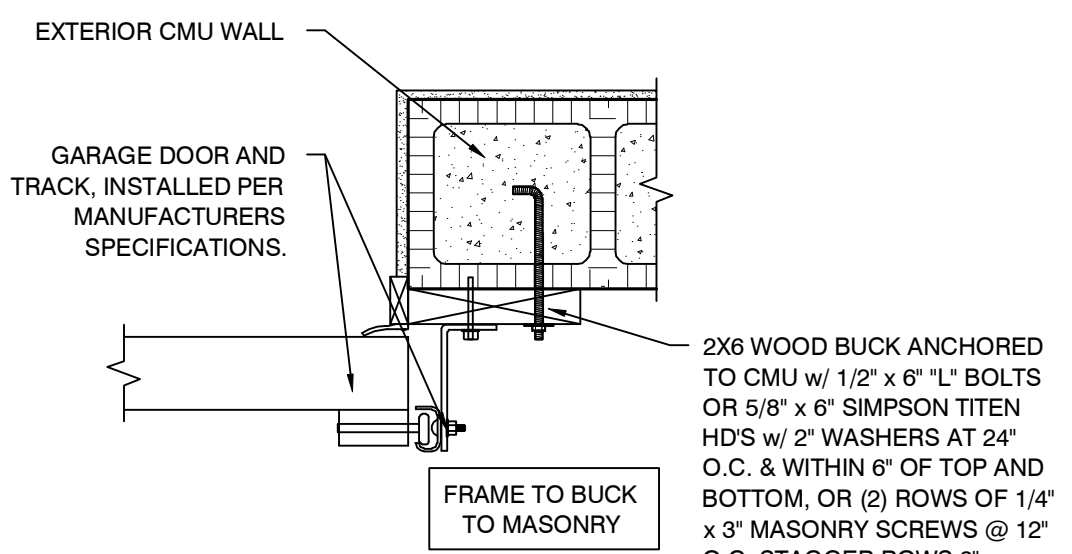


W15

Scale: NOT TO SCALE

11/01/2023

File Name: WGM-150



W16

Scale: NOT TO SCALE

11/01/2023

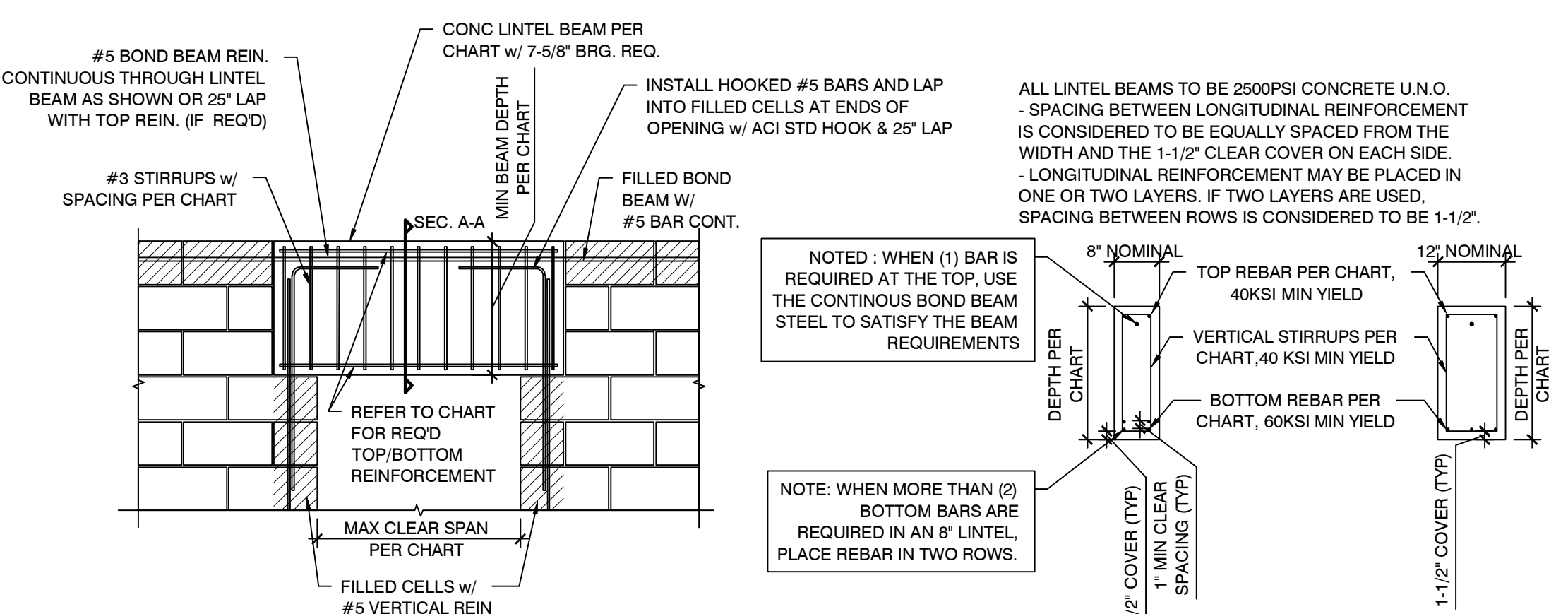
File Name: WGM-160

W01R RED THERMO-PLY SHEATHING DETAIL

Scale: NOT TO SCALE

11/01/2023

File Name: WGW-012



ELEVATION VIEW

SECTION VIEW A - A

W22 TYPICAL CAST-IN-PLACE CONCRETE LINTEL DETAIL

Scale: NOT TO SCALE

7/16/2021

File Name: WGM-220

8" Lintel per plan									
Max Clear Span (S)	8F8-08/1T 8F8-18/1T	8F8-08/1T w/ RECESS 8F8-18/1T w/ RECESS	8F12-08/1T 8F12-18/1T	8F16-08/1T 8F16-18/1T	8F20-08/1T 8F20-18/1T	8F24-08/1T 8F24-18/1T	8F28-08/1T 8F28-18/1T	8F32-08/1T 8F32-18/1T	
0' < S ≤ 4'	8"x8" CONC BEAM w/ (1) #5 BAR TOP/ (2) #4 BAR BOTTOM. #3 STIRRUPS @ 4" O.C.	8"x6" CONC BEAM w/ (1) #5 BAR TOP/ (2) #5 BARS BOTTOM. #3 STIRRUPS @ 4" O.C.	8"x12" CONC BEAM w/ (1) #5 BAR TOP/ (1) #4 BAR BOTTOM. #3 STIRRUPS @ 5" O.C.	8"x16" CONC BEAM w/ (1) #5 BAR TOP/ (1) #4 BAR BOTTOM. #3 STIRRUPS @ 7" O.C.	8"x20" CONC BEAM w/ (1) #5 BAR TOP/ (2) #4 BARS BOTTOM. #3 STIRRUPS @ 9" O.C.	8"x24" CONC BEAM w/ (1) #5 BAR TOP/ (2) #4 BARS BOTTOM. #3 STIRRUPS @ 10" O.C.	8"x28" CONC BEAM w/ (1) #5 BAR TOP/ (2) #4 BARS BOTTOM. #3 STIRRUPS @ 12" O.C.	8"x32" CONC BEAM w/ (1) #5 BAR TOP/ (2) #4 BARS BOTTOM. #3 STIRRUPS @ 14" O.C.	
4' < S ≤ 7'	8"x8" CONC BEAM w/ (1) #5 BAR TOP/ (2) #4 BARS BOTTOM. #3 STIRRUPS @ 4" O.C.	8"x6" CONC BEAM w/ (1) #5 BARS TOP/ (2) #5 BARS BOTTOM. #3 STIRRUPS @ 2" O.C.	8"x12" CONC BEAM w/ (1) #5 BARS TOP/BOTTOM. #3 STIRRUPS @ 5" O.C.	8"x16" CONC BEAM w/ (2) #5 BARS TOP/ (2) #4 BARS BOTTOM. #3 STIRRUPS @ 6" O.C.	8"x20" CONC BEAM w/ (2) #5 BARS TOP/ (2) #5 BARS BOTTOM. #3 STIRRUPS @ 9" O.C.	8"x24" CONC BEAM w/ (2) #5 BARS TOP/ (2) #5 BARS BOTTOM. #3 STIRRUPS @ 10" O.C.	8"x28" CONC BEAM w/ (2) #5 BARS TOP/ (2) #5 BARS BOTTOM. #3 STIRRUPS @ 12" O.C.	8"x32" CONC BEAM w/ (2) #5 BARS TOP/ (2) #5 BARS BOTTOM. #3 STIRRUPS @ 14" O.C.	
7' < S ≤ 12'-8"	CONTACT E.O.R. FOR CONCRETE BEAM DESIGN	CONTACT E.O.R. FOR CONCRETE BEAM DESIGN	8"x12" CONC BEAM w/ (2) #5 BARS TOP/BOTTOM. #3 STIRRUPS @ 5" O.C.	8"x16" CONC BEAM w/ (2) #5 BARS TOP/ (2) #5 BARS BOTTOM. #3 STIRRUPS @ 6" O.C.	8"x20" CONC BEAM w/ (2) #5 BARS TOP/ (2) #5 BARS BOTTOM. #3 STIRRUPS @ 9" O.C.	8"x24" CONC BEAM w/ (2) #5 BARS TOP/ (2) #5 BARS BOTTOM. #3 STIRRUPS @ 10" O.C.	8"x28" CONC BEAM w/ (2) #5 BARS TOP/ (2) #5 BARS BOTTOM. #3 STIRRUPS @ 12" O.C.	8"x32" CONC BEAM w/ (2) #5 BARS TOP/ (2) #5 BARS BOTTOM. #3 STIRRUPS @ 14" O.C.	
12'8"<S ≤ 21'-4"	CONTACT E.O.R. FOR CONCRETE BEAM DESIGN	CONTACT E.O.R. FOR CONCRETE BEAM DESIGN	CONTACT E.O.R. FOR CONCRETE BEAM DESIGN	8"x16" CONC BEAM w/ (2) #5 BARS TOP/ (4) #5 BARS BOTTOM. #3 STIRRUPS @ 6" O.C.	8"x20" CONC BEAM w/ (2) #5 BARS TOP/ (4) #5 BARS BOTTOM. #3 STIRRUPS @ 9" O.C.	8"x24" CONC BEAM w/ (2) #5 BARS TOP/ (4) #5 BARS BOTTOM. #3 STIRRUPS @ 10" O.C.	8"x28" CONC BEAM w/ (2) #5 BARS TOP/ (4) #5 BARS BOTTOM. #3 STIRRUPS @ 12" O.C.	8"x32" CONC BEAM w/ (2) #5 BARS TOP/ (4) #5 BARS BOTTOM. #3 STIRRUPS @ 14" O.C.	

W22A CIP BEAM/8" LINTEL CONVERSION CHART

Scale: NOT TO SCALE

MOD (01/08/2025)

File Name: WGM-221 (EP-010)

W22B CIP BEAM/12" LINTEL CONVERSION CHART

Scale: NOT TO SCALE

MOD (01/08/2025)

File Name: WGM-222 (EP-010)

12" Lintel per plan					
Max Clear Span (S)	12F8-2B/2T	12F12-2B/2T	12F16-2B/2T	12F20-2B/2T	12F24-2B/2T
S ≤ 7'	12"x8" CONC BEAM w/ (2) #4 BAR TOP/ (2) #5 BAR BOTTOM. #3 STIRRUPS @ 3" O.C.	12"x12" CONC BEAM w/ (2) #4 BAR TOP/ (2) #4 BAR BOTTOM. #3 STIRRUPS @ 3" O.C.	12"x16" CONC BEAM w/ (2) #4 BAR TOP/ (2) #5 BAR BOTTOM. #3 STIRRUPS @ 7" O.C.	12"x20" CONC BEAM w/ (2) #4 BARS TOP/ (3) #5 BARS BOTTOM. #3 STIRRUPS @ 8" O.C.	12"x24" CONC BEAM w/ (2) #4 BARS TOP/ (3) #5 BARS BOTTOM. #3 STIRRUPS @ 8" O.C.
7' < S ≤ 12'-8"	CONTACT E.O.R. FOR CONCRETE BEAM DESIGN	12"x12" CONC BEAM w/ (2) #5 BARS TOP/ (3) #5 BARS BOTTOM. #3 STIRRUPS @ 5" O.C.	12"x16" CONC BEAM w/ (2) #5 BARS TOP/ (3) #5 BARS BOTTOM. #3 STIRRUPS @ 7" O.C.	12"x20" CONC BEAM w/ (2) #5 BARS TOP/ (3) #5 BARS BOTTOM. #3 STIRRUPS @ 8" O.C.	12"x24" CONC BEAM w/ (2) #5 BARS TOP/ (3) #5 BARS BOTTOM. #3 STIRRUPS @ 10" O.C.
12'8"<S ≤ 18'-8"	CONTACT E.O.R. FOR CONCRETE BEAM DESIGN	CONTACT E.O.R. FOR CONCRETE BEAM DESIGN	12"x16" CONC BEAM w/ (2) #5 BARS TOP/ (4) #5 BARS BOTTOM. #3 STIRRUPS @ 7" O.C.	12"x20" CONC BEAM w/ (2) #5 BARS TOP/ (3) #5 BARS BOTTOM. #3 STIRRUPS @ 8" O.C.	12"x24" CONC BEAM w/ (2) #5 BARS TOP/ (3) #5 BARS BOTTOM. #3 STIRRUPS @ 10" O.C.

THIS ITEM HAS BEEN ELECTRONICALLY SIGNED AND SEALED BY IBRAHEM ELAYEB, P.E. ON THE DATE NOTED ON THIS SHEET USING A SHA256 AUTHENTICATION CODE. PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED AND THE SHA256 AUTHENTICATION CODE MUST BE VERIFIED ON ANY ELECTRONIC COPIES.

Florida Zone

24311 Walden Center Dr., Suite 300
Bonita Spring, Florida 34134 (239)495-4800 (770)381-3450

PulteGroup

FIELDSTONE

ARCHITECTURE & ENGINEERING

Michigan 5400 Auburn Rd., Ste. 200 Auburn Hills, MI 48003
Florida 1500 Vance Oaks Blvd., Ste. 150 Tampa, FL 33607
248.622.4035 FieldstoneAE.com 813.446.5310

Home for * Market Home * 8338-09300

6990 Minneola Court, Nokomis, FL 34275

WALL DETAILS

PRODUCT-MANAGER

RELEASE

DATE: #####

NEW DATE/DESCRIPTION

△

△

△

△

PROJECT TYPE

Single Family

SPECIFICATION LEVEL

Pulte

PLAN NAME

Mystique

NPC CHILD NUMBER

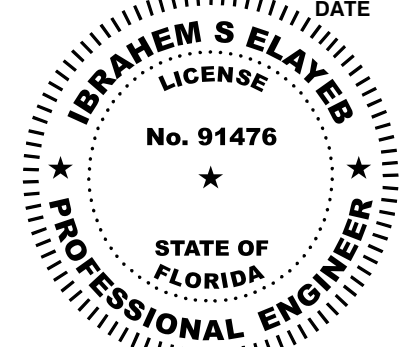
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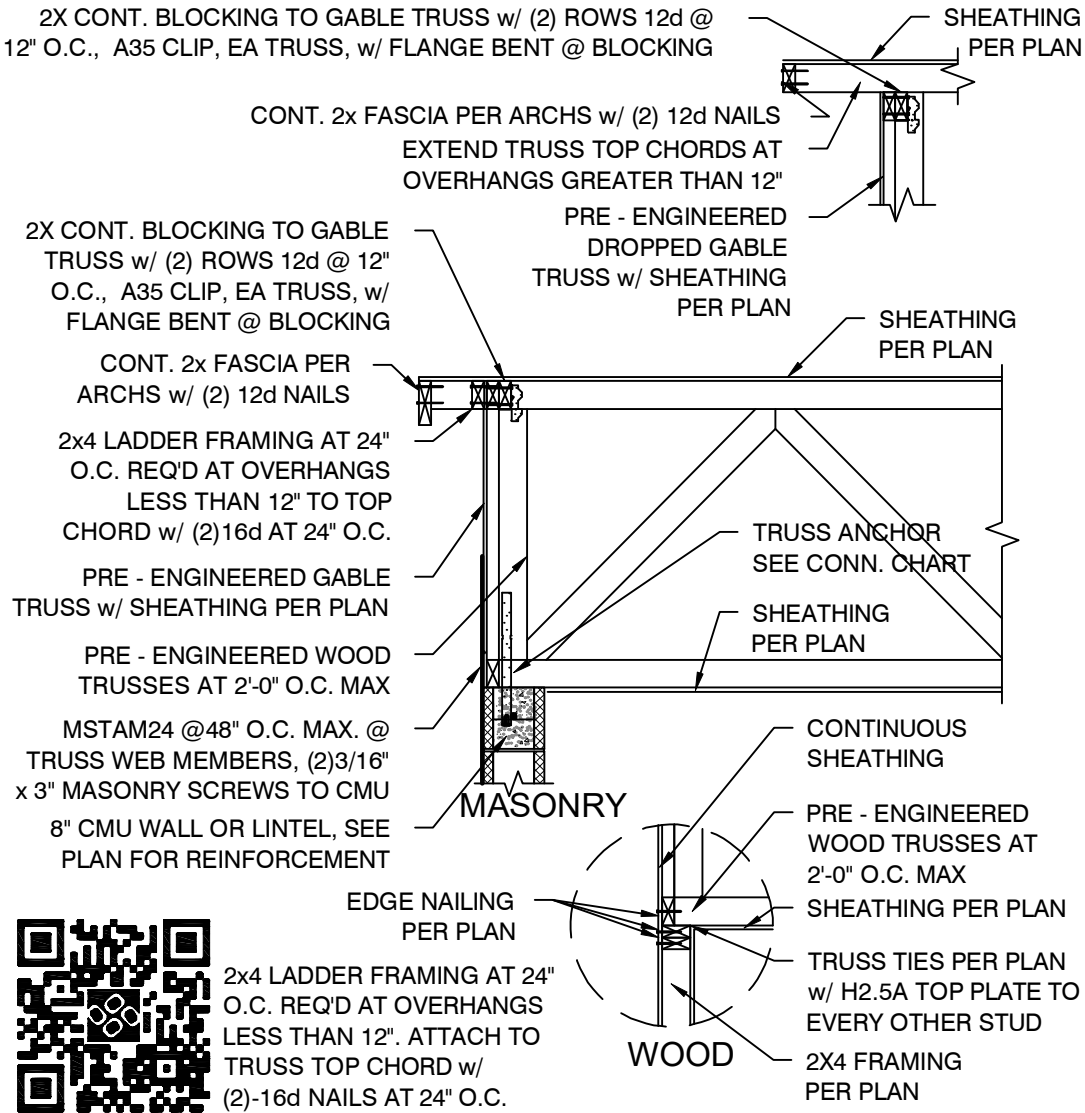
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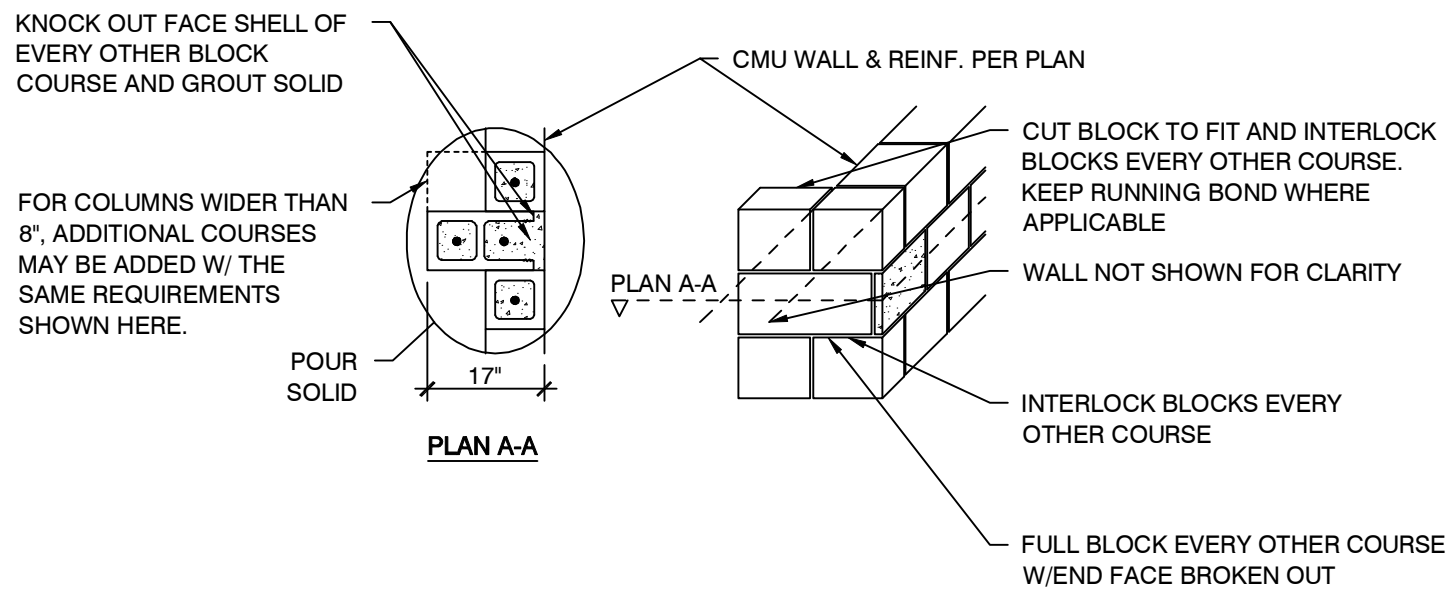
Fieldstone

3400 AUBURN RD., STE. 200
AUBURN HILLS, MI 48236
C.O.A.# ARCHITECTURE: AA26002802
C.O.E.# ENGINEERING: 29349
P.E. #91476
IBRAHEM ELAYEB, PE
P.E.# 80956

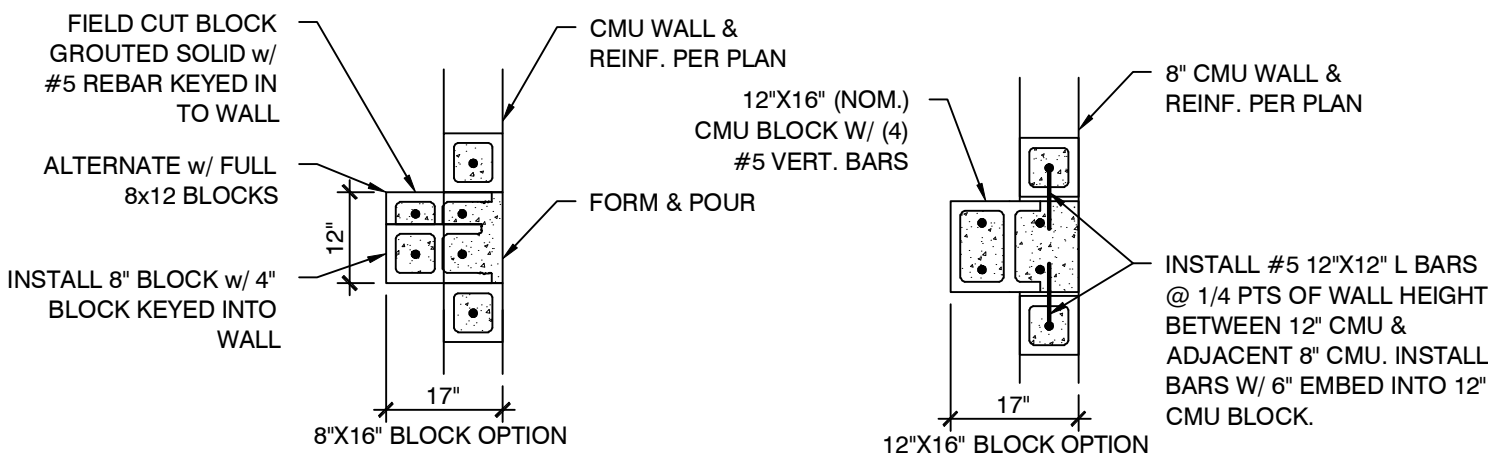




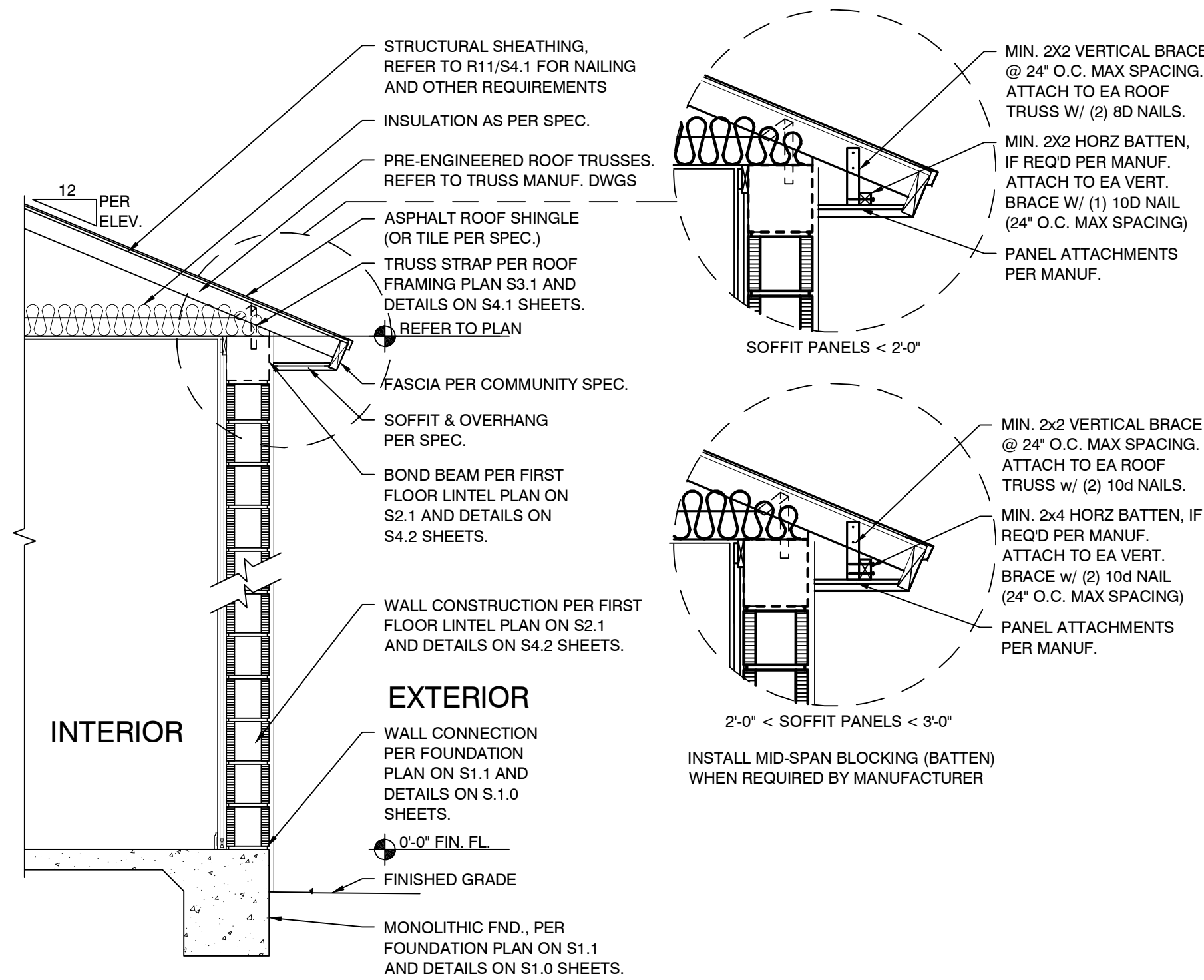
R21 GABLE END w/ NO VALLEY
NOT TO SCALE
11/01/2023
RWG-210



W20A WALL CONNECTION @ SINGLE 8" COLUMN
NOT TO SCALE
MOD (01/07/2025)
WGM-225(RB-010)



W20B WALL CONNECTION @ 12" WIDE COLUMN
NOT TO SCALE
MOD (01/07/2025)
WGM-225(RB-010)



S01 TYPICAL 1 STORY WALL SECTION
NOT TO SCALE
MOD (12/22/2023)
WSM-010

Fieldstone
3400 AUBURN RD., STE. 200
AUBURN HILLS, MI 48236
C.O.A.# ARCHITECTURE: AA26002802
C.O.E.# ENGINEERING: 29349
■ IBRAHEM ELAYEB, PE
P.E.# 91476
□ IAN MCCULLOUGH, PE
P.E.# 80956

10/23/2025

DATE

IBRAHEM S. ELAYEB
LICENSE
No. 91476
STATE OF FLORIDA
PROFESSIONAL ENGINEER

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248.622.4035 FieldstoneAE.com 813.446.3310

Home for * Market Home * 8338-09300
6990 Minneola Court, Nokomis, FL 34275
MISC. - DETAILS

PRODUCT MANAGER	
RELEASE	
DATE: #####	
REV#	DATE/DESCRIPTION
1	
2	
3	
4	

PROJECT TYPE
Single Family

SPECIFICATION LEVEL
Pulte

PLAN NAME
Mystique
NPC CHILD NUMBER
2684.500

SHEET
S4.4

160 MPH - 2023 FL - C & C Pressures (ASD)																
Effective Wind Area	Gable Roof Components						Hip Roof Components						Wall Components			
	Zone 1		Zone 2		Zone 3		Zone 1		Zone 2		Zone 3		Zone 4		Zone 5	
A > 100 SQFT (Girders)	17.1	-36.2	17.1	-48.8	17.1	-61.5	14.8	-37.6	14.8	-53.8	14.8	-57.6	31.1	-34.2	31.1	-38.0
A < 50 SQFT (Windows)	19.2	-45.6	19.2	-61.1	19.2	-78.3	18.7	-44.8	18.7	-61.8	18.7	-66.3	32.7	-35.9	32.7	-41.3
A < 10 SQFT (Fasteners)	24.3	-67.6	24.3	-89.3	24.3	-117.3	27.3	-61.5	27.3	-80.0	27.3	-86.3	36.6	-39.8	36.6	-49.0

Components and Cladding Pressures (ASD) - 2023 FL						
Description	Zone	Width (ft)	Height (ft)	Effective Area (ft2)	Positive Pressure (psf)	Negative Pressure (psf)
16070 OHGD	---	16	7	112	30.9	-34.5
16080 OHGD	---	16	8	128	30.9	-34.5
9080 SGD	4	9	8	72.0	32.0	-35.2
26SH	4	3	6	18.0	35.3	-38.4
25SH (2)	4	6	5.25	31.5	34.1	-37.2
25SH (2)	END 5	6	5.25	31.5	34.1	-44.0
25SH (3)	4	9	5.25	47.3	32.9	-36.1
25SH	4	3	5.25	15.8	35.7	-38.8
4010 FIXED	4	4	1	4.0	36.6	-39.8
3050SH	4	3	5	15.0	35.8	-38.9
3050SH (2)	4	6	5	30.0	34.2	-37.3
1/2 35 SH	4	2.33	5.25	12.2	36.2	-39.4
1/2 35 SH	END 5	2.33	5.25	12.2	36.2	-48.3
26SH	END 5	3	6	18.0	35.3	-46.4
26SH (2)	END 5	6	6	36.0	33.8	-43.4
26SH & 4060 FX	END 5	7	6	42.0	33.3	-42.5
3080 DOOR	4	3	8	24.0	34.7	-37.8
8080 SGD	END 5	8	8	64.0	32.3	-40.4
26SH	END 5	3	6	18.0	35.3	-46.4
6080 SGD	END 5	6	8	48.0	32.9	-41.6
1/2 36SH	END 5	2.33	6	14.0	35.9	-47.7

DESIGN PRESSURES SHOWN ARE MINIMUM ALLOWABLE VALUES. VALUES ARE TO BE INCREASED AS REQUIRED BY MORE STRINGENT LOCAL REQUIREMENTS

PRECAST LINTEL GRAVITY LOAD TABLE											v4.21
LOADS LISTED HAVE BEEN CALCULATED OR COMPILED FROM APPROVED MANUFACTURERS: CAST-CRETE, LOTTS CONCRETE, QUALITY PRECAST CO.											
MAXIMUM LOAD (PLF)											
APPROX. LINTEL LENGTH	MAX. CLEAR OPENING	TYPE A 8" UNTEL	8" TYPE B 8" TYPE C	12" TYPE B 12" TYPE C	14" TYPE B ** 14" TYPE C **	16" TYPE B 16" TYPE C	20" TYPE B 20" TYPE C	24" TYPE B 24" TYPE C	28" TYPE B 28" TYPE C	32" TYPE B 32" TYPE C	
2' 10"	1' 6"	2231	3069 3069	3719 4605	3280 5206	5163 6113	6607 7547	8054 8974	9502 10000	10000 10000	
3' 6"	2' 2"	2231	3069 3069	3719 4605	3280 5206	5163 6113	6607 7547	8054 8974	9502 10000	10000 10000	
4' 0"	2' 8"	1783	2561 2561	2751 4605	3280 5206	3820 6113	4890 7547	5961 8974	7034 10000	8107 10000	
4' 6"	3' 2"	1343	1969 1969	2110 4375	2992 5206	2931 6113	3756 7019	4576 8672	5400 10000	6224 10000	
4' 8"	3' 4"	1217	1349 1349	1438 3090	1716 4567	1999 5365	2560 4766	3132 5817	3686 7074	4249 8203	
5' 4"	4' 0"	1006	1349 1349	1438 3090	1716 4567	1999 5365	2560 4766	3132 5817	3686 7074	4649 8203	
5' 10"	4' 6"	860	1105 1105	1173 2622	1600 4242	1631 4360	2090 3942	2549 4889	3009 5817	3470 6662	
6' 6"	5' 2"	710	1232 1232	2177 2177	3738 2738	3480 3480	3031 5381	3707 8360	4383 8625	5061 7590	
7' 6"	6' 2"	570	991 991	1729 1729	2108 2108	2632 2632	2205 3898	2698 5661	3191 4742	3685 5501	
8' 4"	7' 0"	488	699 699	1160 1245	1516 1516	1626 1843	2564 2564	3486 3486	2818 2964	3302 3387	
9' 4"	8' 0"	416	699 699	1160 1245	N/A N/A	1625 1843	2564 2564	3486 3486	2818 2964	3302 3387	
10' 6"	9' 2"	354	535 535	890 1054	N/A N/A	1247 1533	2093 2093	2777 2781	2163 2259	2536 2615	
11' 4"	10' 0"	315	567 567	890 945	N/A N/A	1366 1366	1846 1846	2423 2423	3127 3127	4006 4006	
12' 0"	10' 8"	292	525 525	873 873	N/A N/A	1254 1254	1684 1684	2193 2193	2805 2805	3552 3552	
12' 6"	11' 2"	277	471 471	755 755	N/A N/A	1075 1075	1428 1428	1838 1838	2316 2316	2883 2883	
13' 4"	12' 0"	255	471 471	755 755	N/A N/A	1075 1075	1428 1428	1838 1838	2316 2316	2535 2883	
14' 0"	12' 8"	239	424 424	706 706	N/A N/A	1002 1002	1326 1328	1697 1697	2127 2127	2278 2630	
14' 8"	13' 4"	N/A	N/A N/A	N/A 783*	N/A N/A	N/A 1234*	N/A 1421*	N/A 1779*	N/A 2188*	N/A 2662*	
15' 4"	14' 0"	N/A	N/A N/A	N/A 710*	N/A N/A	N/A 1160*	N/A 1331*	N/A 1660*	N/A 2032*	N/A 2459*	
17' 4"	16' 0"	N/A	N/A N/A	N/A 548*	N/A N/A	N/A 952*	N/A 1117*	N/A 1379*	N/A 1671*	N/A 1997*	
19' 4"	18' 0"	N/A	N/A N/A	N/A 420*	N/A N/A	N/A 750*	N/A 960*	N/A 1177*	N/A 1415*	N/A 1676*	
20' 0"	18' 8"	N/A	N/A N/A	N/A 340*	N/A N/A	N/A 598*	N/A 845*	N/A 1114*	N/A 1345*	N/A 1468*	
24' 0"	22' 8"	N/A	N/A N/A	N/A 250*	N/A N/A	N/A 450*	N/A 650*	N/A 880*	N/A 1090*	N/A 1220*	
* PRESTRESSED ** 8" W/2" RECESS U-LINELS											
Type A = Unfilled no Rebar; Type B = Continuous Grout, Steel per Plan in Bond Beam Only; Type C = Continous Grout,Steel per plan in Bond Beam & Lintel											

*PRESTRESSED ** 8" W/2" RECESS U-LINTELS

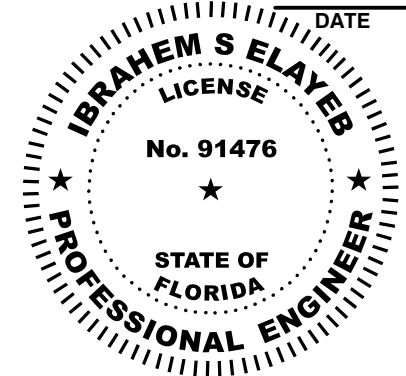
Type A = Unfilled no Rebar; Type B = Continuous Grout, Steel per Plan in Bond Beam Only; Type C = Continous Grout,Steel per plan in Bond Beam & Lintel

LEGEND			
	8x8 FILLED CELL W/ (1) - #5 REBAR		BEAMS
	8x8 FILLED CELL W/ (1) - #7 REBAR		HEADER - WIDTH TO MATCH WALL
	8x8 FILLED CELL W/ NO REBAR REQD		GIRDERS
	45° CMU - FILL CELLS W/ #5 REBAR		PLUMBING WALLS (FOR REFERENCE)
	12x8 FILLED CELL W/ (1) - #5 REBAR		BEARING/SHEAR WALLS (FOR REFERENCE)
	12x12 FILLED CELL W/ (4) - #4 REBAR (IN COLUMN)		ROOF BLOCKING
	ELEVATION INDICATOR		BALLOON FRAMED WALLS/HIGHER BEARING WALLS
	DETAIL INDICATOR		CONCRETE TIE BEAMS
	LINTEL OR WOOD HDR DESIGNATION (ALL WOOD HDR'S TO BE (2) PLY U.N.O.)		PLUMBING PENETRATION FOR FRAMING LAYOUT
			POINT LOAD

ABBREVIATIONS:			
AB	ANCHOR BOLT	EA	EACH
AFF	ABOVE FINISHED FLOOR	ELEV	ELEVATION
ALT	ALTERNATE	EMBED	EMBEDMENT
APPROX	APPROXIMATE	EN	EDGE NAILING
ARCH	ARCHITECT/ARCHITECTURAL	EQ	EQUAL
ATR	ALL THREADED ROD	ES	EACH SIDE
BLK	BLOCK	EW	EACH WAY
BLKG	BLOCKING	EXP	EXPANSION
BOF	BOTTOM OF FOOTING	EXT	EXTERIOR
BOT	BOTTOM	FD	FLOOR DRAIN
BRG	BEARING	FDN	FOUNDATION
BTWN	BETWEEN	FIN	FINISH
CANT	CANTILEVER	FLR	FLOOR
CL	CENTERLINE	FN	FIELD NAILING
CIP	CAST IN PLACE	FT	FOOT / FEET
CJ	CONTROL JOINT	GALV	GALVANIZED
CLR	CLEAR	GDR	GIRDER
CMU	CONCRETE MASONRY UNIT	GRD	GRADE
COL	COLUMN	GWB	GYP SUM WALL BOARD
CONC	CONCRETE	GYP	GYP SUM
CONT	CONTINUOUS	HD	HOLD DOWN
DBL	DOUBLE	HDR	HEADER
DIA	DIAMETER	ILO	IN LIEU OF
DWG	DRAWING	INCH	INCH
INT	INTERIOR	IST	JOIST
MAX	MAXIMUM	MFR	MANUFACTURER
MIN	MINIMUM	MISC	MISCELLANEOUS
MO	MASONRY OPENING	NOM	NOMINAL
NOM	NOMINAL	NOT	NOT TO SCALE
OC	ON CENTER	OPNG	OPENING
OPP	OPPOSITE	PELV	PANEL EDGE NAILING
PLYWD	PLYWOOD	PLWYD	PLYWOOD
REINF	REINFORCING/REINFORCEMENT	REQD	REQUIRED
RO	ROUGH OPENING	SC	STUD COLUMN
SC	STUD COLUMN	SHTG	SHEATHING
SP	SOUTHERN PINE #2 OR BETTER	SIM	SIMILAR
SPF	SPRUCE PINE FIR #1/#2 OR BETTER	SP	SOUTHERN PINE #2 OR BETTER
TYP	TYPICAL	SPF	SPRUCE PINE FIR #1/#2 OR BETTER
UNO	UNLESS NOTED OTHERWISE	TYP	TYPICAL

Fieldstone
 3400 AUBURN RD., STE. 200
 AUBURN HILLS, MI 48236
 C.O.A.# ARCHITECTURE: AA26002802
 C.O.E.# ENGINEERING: 29349

■ IBRAHEM ELAYEB, PE
 P.E. #91476
 □ IAN MCCULLOUGH, PE
 P.E.# 80956



PRODUCT MANAGER	
RELEASE	DATE: #####
NEW DATE/DESCRIPTION	

PROJECT TYPE
Single Family

SPECIFICATION LEVEL
Pulte

PLAN NAME
Mystique
 NPC CHILD NUMBER
2684.500

SHEET
S5.1

Florida Zone
 24311 Walden Center Dr., Suite 300
 Bonita Spring, Florida 34134 (239)495-4800 (770)381-3450

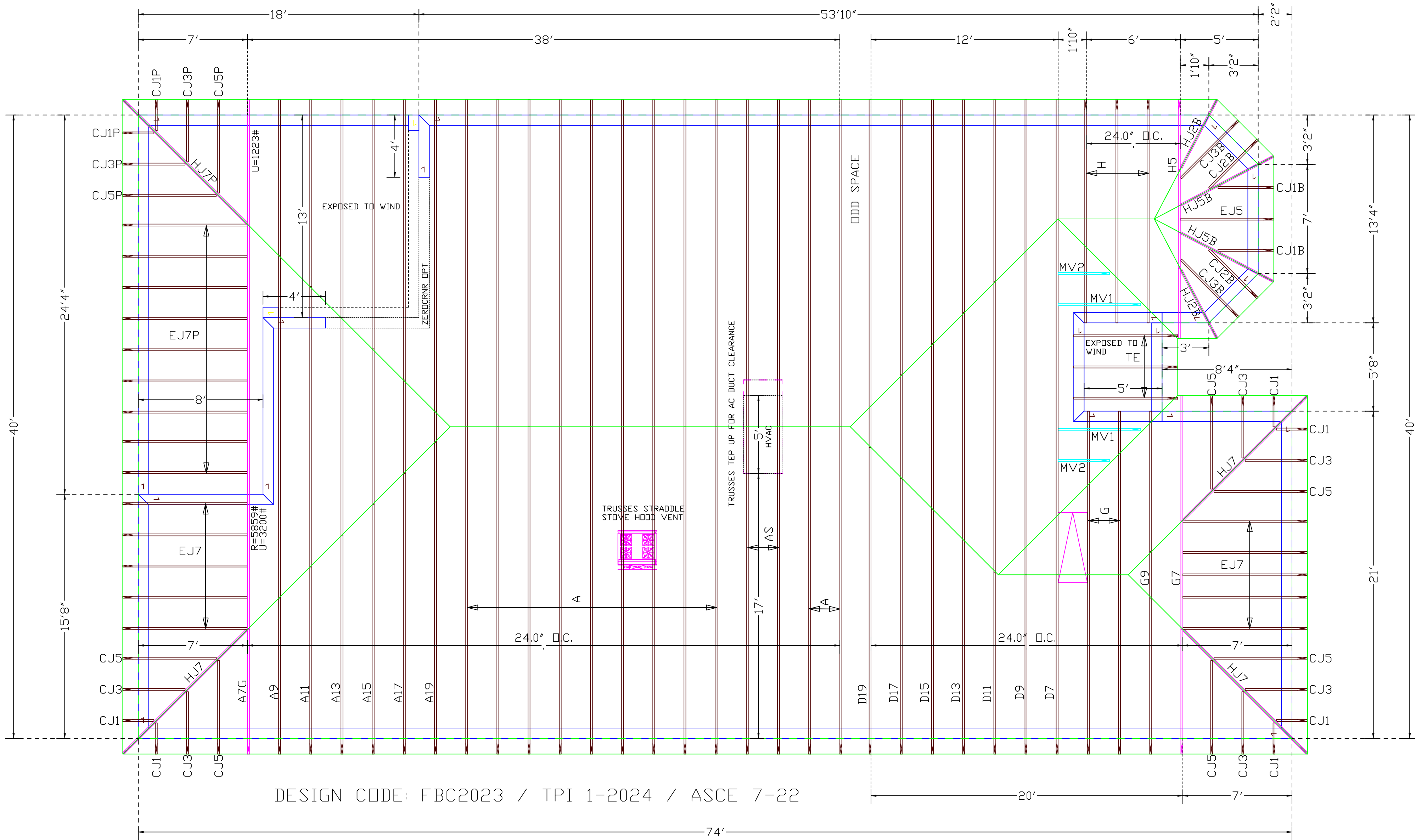


FIELDSTONE
 ARCHITECTURE & ENGINEERING
 3400 Auburn Rd., Ste. 200 Auburn Hills, MI 48236
 Mobile: 1800-Tempo-Data-Best - 340-580-Tempo E 3367
 248.622.4035 FieldstoneAE.com 813.448.3110

Home for * Market Home * 8338-09300
 6990 Minneola Court, Nokomis, FL 34275
 REFERENCE CHARTS

Wind Importance Factor	1
Occupancy/Risk Category	II
Lanai/Entry & Porch Areas	EXPOSED TO WIND

TRUSSES DESIGN FOR TILE AND SHINGLE LOADING



DESIGN CODE: FBC2023 / TPI 1-2024 / ASCE 7-22



SHOP DRAWING REVIEW

Review is for general compliance with the Contract Documents. Sole responsibility for the correctness of dimensions, detail quantities, and safety during fabrication and erection shall remain with the Contractor.

- ☒ No Exceptions Taken
- ☐ Furnish with Changes Noted
- ☐ Amend and Resubmit
- ☐ Rejected:

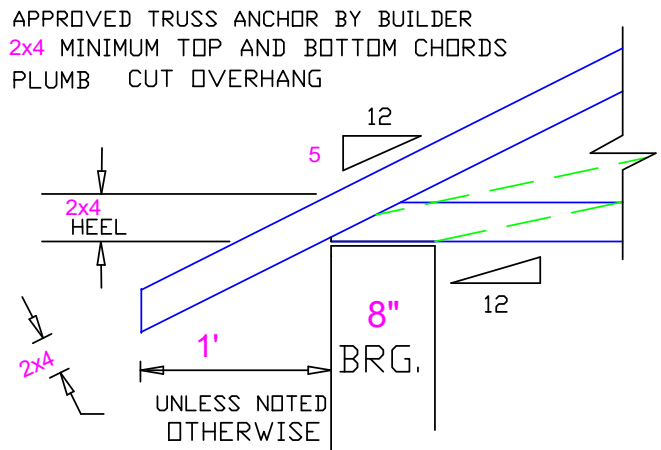
Reviewed by: Ibrahim Elayeb, PE Date: 10/23/2025

Truss List of >5,000# Reaction & >-1,000# Uplift			
Truss ID		Reactions	
A7G	lbs	5,859 lbs	lbs
	lbs	-3,200 lbs	-1,223 lbs

- GENERAL NOTES
- * Required interior bearing walls shown @ heights noted
 - * Trusses may not be cut or altered in any way without prior authorization from ABS, Inc.
 - * Any trusses that are cut or altered without authorization will be repaired or replaced at the customers expense
 - * No back charges of any kind will be accepted without prior review and written consent from ABS, Inc.
 - * For proper truss handling and bracing, refer to the "TPI" documents "BCSI-B1 through B4"
 - * Permanent and temporary bracing is the responsibility of the truss installer. The "Engineer of Record" for the project is responsible for the design of the permanent bracing, the diaphragm system, shear walls, and structural elements to resist lateral loads from wind and or seismic activity. The "EOR" is also responsible to call out the required strapping materials to sufficiently attach the trusses to the load bearing structure below, to verify truss design specifications (pitch, span, profiles, applied loading, wind application, etc.), and for the overall design and placement plan of the truss system.
 - * If any job site accidents occur involving trusses, the installer must immediately stop work on the project and notify a representative of ABS, Inc.. All trusses involved in an accident must be inspected by a licensed structural engineer to determine the cause of the accident. The builder assumes all liability if trusses involved in an accident are altered or moved in any way before an inspection is completed. All decisions regarding necessary repairs or replacement of trusses will be based on the recommendation of the report sub-mitted by the structural engineer.

- MULTI-PLY ATTACHMENT
- * For 4-ply or 5-ply attachment, refer to the Detail Packet Sheet: "STANDARD BOLT TO SCREW TRUSS CONNECTION DETAIL" "-T-4PLY OR 5PLY SCREW" *** CRITICAL ***
- ATTN: FRAMER
- For multi-ply girder attachments, refer to engineering for specific instructions for attaching plies. Each ply must be applied in layers per the nailing specifications. 2-ply trusses may be nailed from one face.
- * For 3-ply trusses, the first two plies are nailed together from one face, then third ply is attached to either face of first two plies.
 - * For 4 ply trusses, after assembling the first three plies, attach fourth ply to either face.
 - * For 5 ply trusses, after assembling the first four plies, attach fifth ply to either face.
- (Refer to engineering for additional bolts or screw rqrmnts and the "STANDARD BOLT TO SCREW TRUSS CONNECTION DETAIL" for substituting screws for bolts, located in the engineering detail pkg.
- NOTE: Bolts/Screws are intended to provide clamping force to aid in allowing the multi-ply assembly to act as a unit and are not included in the calculation of ply-to-ply load transfer.

- Hanger Notes
- * Refer to Simpson Strong-Tie website (www.strongtie.com/products/connectors), or the USP website (www.uspconnectors.com/us/products/connectors), for proper use and attachment of the specified hangers.



TYPICAL TRUSS END

LOADING CRITERIA				
	ROOF	FLOOR		
TCLL	20	0	Mean Hght	
TCDL	15	0	Wind Speed	160 m.p.h.
BCLL	0	0	Exposure	C
BCDL	10	0		
TTL LOAD	45	0		
DURATION	1.25	0		



BUILDER:
Pulte Homes Ft. Myers

MODEL:
MYSTIQUE FM2 EXTLAN ZEROCRNR

OPTIONS:
Garage Left

LOCATION:
93 LEGACY GROVES

DATE: 10/21/2025	SALES REP: AG	DESIGNER: ECB	JOB NUMBER: 851505
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REVISION: